

IN DEPTH INSPECTION REPORT

D031085

P.I.N. 3501.60

B.I.N. 1093571

TEAM LEADER	<u>Logan Bessel, P.E.</u>	<u>98781</u> NYSPE LICENSE #
ASSISTANT TEAM LEADER	<u>Devin Bush</u>	
ASSISTANT TEAM LEADER	<u>Brendan Cataldo</u>	
ASSISTANT TEAM LEADER	<u>Samantha Lusher</u>	
FEATURE CARRIED	<u>I-481 Southbound</u>	
FEATURE CROSSED	<u>CSX Transportation and AMTRAK</u>	
DATE FIELD WORK BEGAN	<u>April 25, 2022</u>	
DATE FIELD WORK COMPLETED	<u>May 11, 2022</u>	

TABLE OF CONTENTS

- IN-DEPTH CONDITION DOCUMENTATION
- IN-DEPTH PHOTO DOCUMENTATION

BIN 1093571**Purpose of Inspection**

In-depth inspection was performed in 2013 and 2014 as part of PIN 3501.60; I-81 Viaduct Replacement project. This report serves to update the documentation of the existing bridge elements which will be rehabilitated as part of Phase 1 of the I-81 Viaduct project.

Visual Inspection

The general observations of this bridge during this inspection include concrete cracking and spalling. The structural steel is showing signs of section loss. The bearings appear to be seized and no longer functioning as designed.

The field observations are shown on the plans. Below is a brief summary of conditions for each feature.

Superstructure

Deck – Deck is anticipated to be replaced in Phase 1

Steel – See Structural Steel Inspection Summary

Bearings – Bearings typically contained pack rust and did not appear to be functioning as intended. Pedestals contain cracking and spalling.

Substructure

Begin Abutment – Concrete backwall contained widespread map-cracking across the entire length and isolated areas of delaminated and spalled concrete near the deck joint. The wingwalls exhibit map-cracking over 100% of their exposed faces.

Pier 1 – Concrete cap beam contains areas of cracked and delaminated concrete. Some cracking is on the verge of becoming spalls. The concrete

columns also exhibit widespread delaminated concrete, cracking, and isolated areas of spalled concrete.

Pier 2 – The concrete cap beam contains delaminated and spalled concrete across the majority of its area. The columns also contain widespread areas of delaminated and cracked concrete. The crash wall is 100% delaminated and cracked.

Pier 3 – The concrete cap beam contains delaminated and spalled concrete. The concrete columns also contain widespread areas of delaminated and cracked concrete. The crash wall contains widespread areas of map-cracked, delaminated and spalled concrete with exposed rebar.

Pier 4 – This pier is located adjacent to a salt storage shed. Salt-laden runoff from the salt storage shed was puddled near the face of the crash wall during the inspection. The concrete cap beam contained widespread areas of delaminated concrete. Additionally, cracks are forming in the cap beam above column 2 on both faces of the cap beam. The columns also contained widespread areas of delaminated concrete. The crash wall contained widespread areas of map-cracked, delaminated and spalled concrete.

Pier 5 – The cap beam contains isolated areas of delaminated and cracked concrete. The concrete columns contain isolated areas of delaminated and cracked concrete.

Pier 6 – The cap beam contains isolated areas of delaminated and cracked concrete. The columns also contain isolated areas of delaminated, cracked and spalled concrete.

Pier 7 – The cap beam contains isolated areas of delaminated and cracked concrete. The columns also contain isolated areas of delaminated, cracked and spalled concrete.

Pier 8 – The cap beam contains isolated areas of delaminated and cracked concrete. The columns also contain isolated areas of delaminated, cracked and spalled concrete.

Pier 9 – The cap beam contains isolated areas of delaminated, cracked, and spalled concrete. The columns also contain isolated areas of delaminated, cracked and spalled concrete.

Pier 10 – The cap beam contains isolated areas of delaminated and cracked concrete. The columns also contain isolated areas of delaminated, and cracked concrete.

Pier 11 – The cap beam contains widespread areas of delaminated and cracked concrete. The columns also contain widespread areas of delaminated, and cracked concrete.

Pier 12 – The cap beam contains isolated areas of delaminated and cracked concrete. The columns also contain isolated areas of delaminated, cracked, and spalled concrete.

Pier 13 – The cap beam contains widespread areas of delaminated and cracked concrete. The columns also contain isolated areas of delaminated, cracked, and spalled concrete.

Pier 14 – The cap beam contains isolated areas of delaminated and cracked concrete. The columns also contain isolated areas of delaminated, cracked, and spalled concrete.

End Abutment – Concrete backwall contained widespread map-cracking and areas of delaminated and spalled concrete. The pedestals contained generally sound concrete.

BIN 1093571**Special Emphasis Inspection Required**

Non-Redundant/Fracture Critical Members – No

Pin and Hangers – No

Fatigue-Prone Welds – Yes

Non-Categorized Fatigue-Prone Details – No

Other (Specified in Text) – High Rocker Bearings, Steel Web Bearing Areas

Special Emphasis Details

Special emphasis details exist in the form of category D, E, and E' welds.

Additionally, high rocker bearings are present. As a result of deterioration, the steel webs within the bearing areas are also of special emphasis. These require 100% hands-on inspection.

Overall Steel Condition

The steel showed signs of deterioration. In general, the bottom flange of the fascia beams, girder ends of all beams were showing signs of section loss. Some cross frames were bent, buckled, or broken as a result of corrosion or receipt of unanticipated loading.

Paint

The paint system was generally failing over the entire structure. The paint failure ranges from flaking paint, to measurable section loss of steel members.

Section Loss

Section loss readings were taken at various points along the bridge. The readings are depicted on the framing plans of each span attached. In general, the areas of most significant loss were the lower six (6) inches of the web and the bottom flange. Additionally, the ends of the girders experienced section loss within the bearing area.

IN-DEPTH CONDITION DOCUMENTATION

D. NO.	STATE	FEDERAL AID PROJECT NO.	SHEET NO.	TOTAL SHEETS
	NEW YORK	1-690-3(28) 1-481-2(116)	183	309

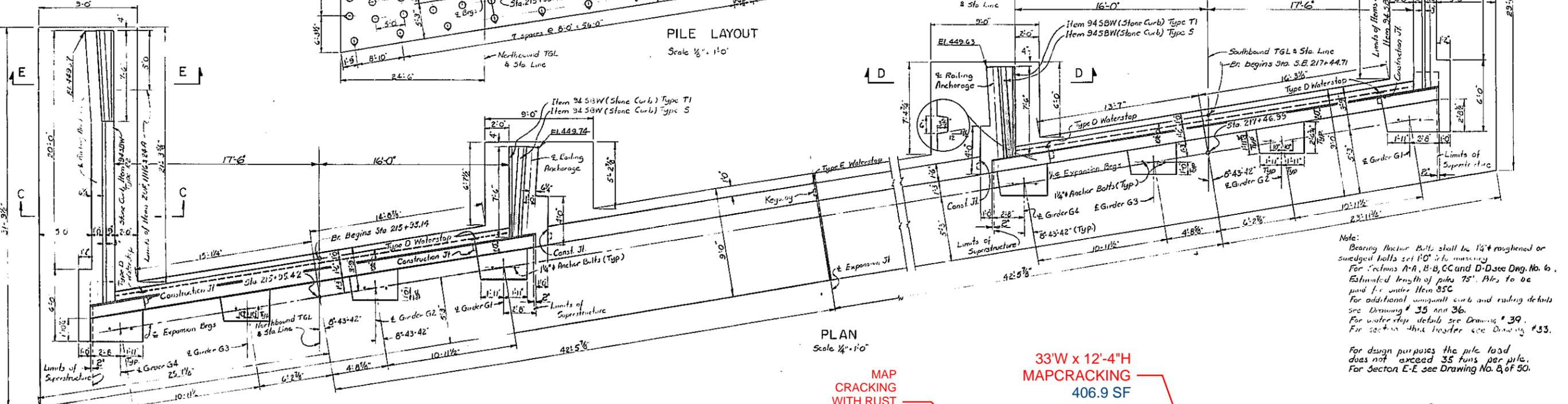
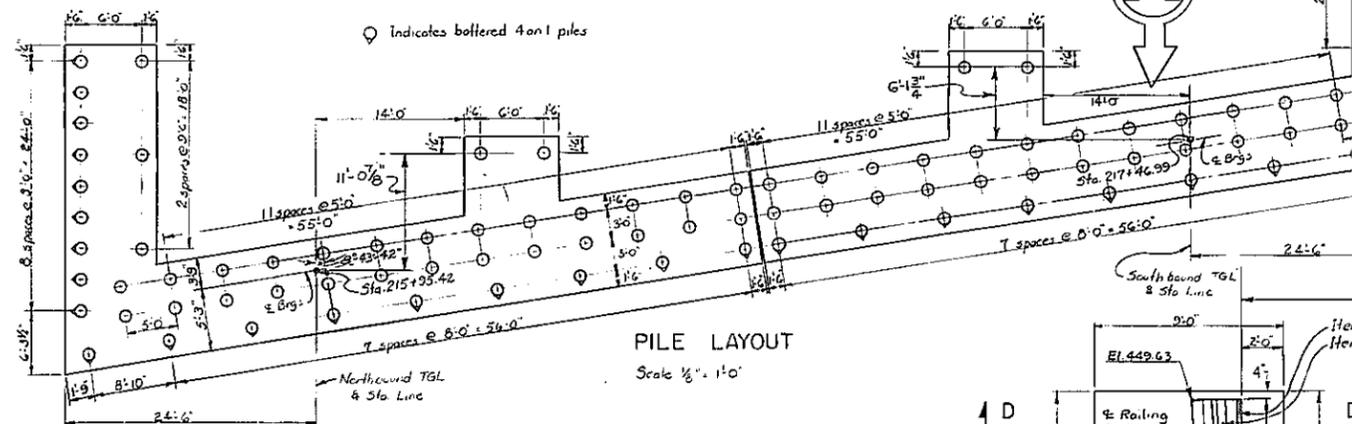
**INTERSTATE ROUTE CONNECTION 570
BUTTERNUT INTERCHANGE (PHASE 2)
ONONDAGA COUNTY**

26'W x 9'H (AVG)
MAPCRACKING
234.0 SF

LEGEND

-  Hollow Concrete (HC)
-  Spall (S)
-  Patch/Repair
-  Crack (C)
-  Hairline Crack (HL)
-  Core Location

**ELEVATION A-A
SOUTHWEST
WINGWALL**



Note:
Bearing Anchor Bolts shall be 1 1/2" roughened or swaged bolts set 1'0" into masonry.
For sections A-A, B-B, C-C and D-D see Dwg. No. 6.
Estimated length of piles 75'. Piles to be paid for under Item 85C.
For additional wingwall curb and railing details see Drawing # 35 and 36.
For water stop details see Drawing # 39.
For section thru header see Drawing # 33.

For design purposes the pile load does not exceed 35 tons per pile.
For section E-E see Drawing No. 8 of 50.

MAP CRACKING WITH RUST STAINING
3.0 SF

33'W x 12'-4"H
MAPCRACKING
406.9 SF

SPALL
1.0 SF

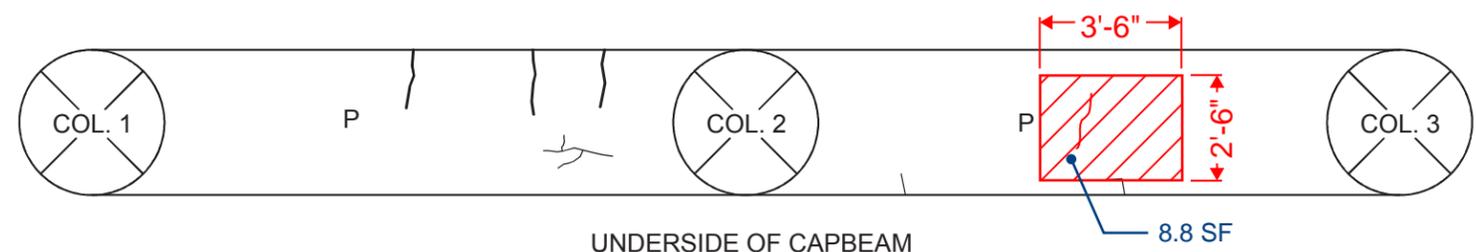
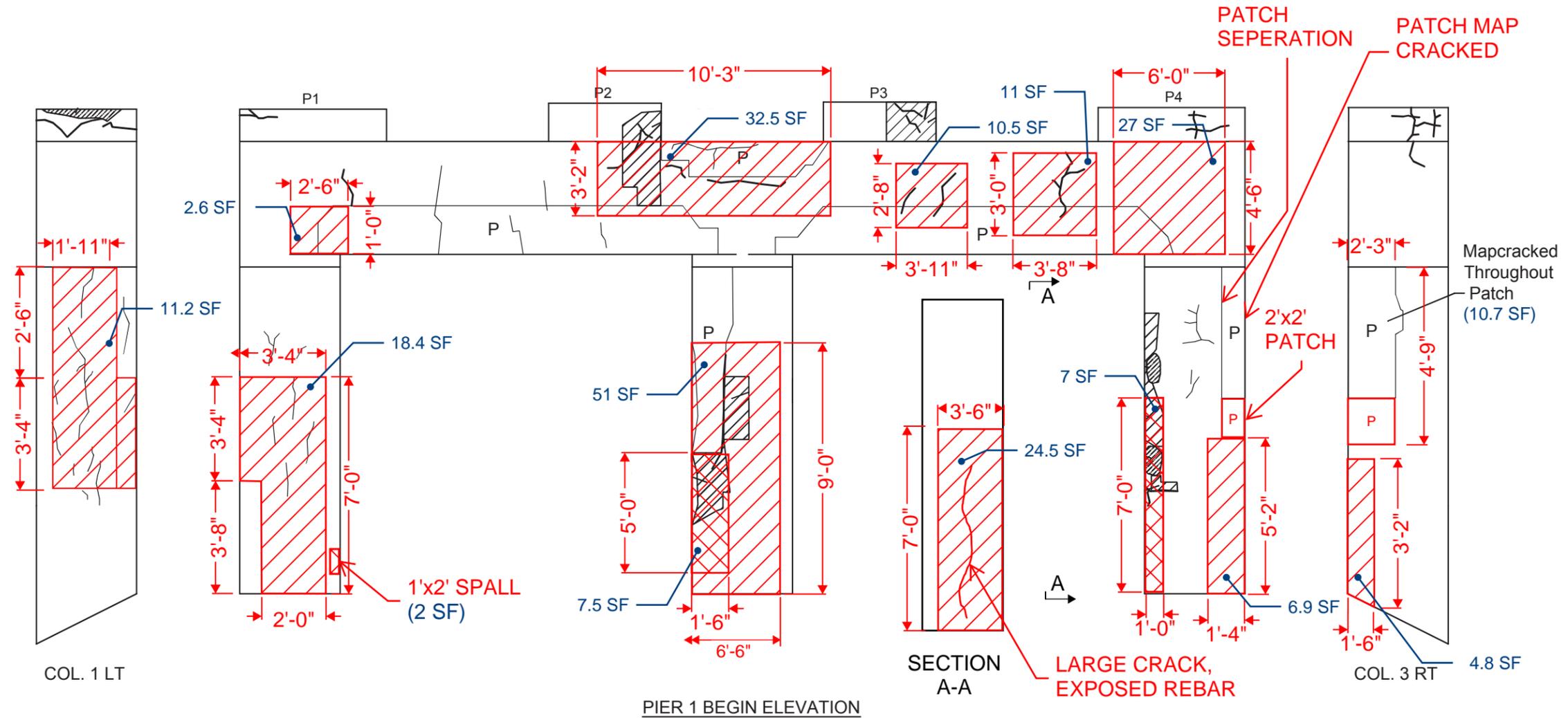
QUANTITIES:
SPALL AREAS: 1.0 SF
HOLLOW AREAS: 0.0 SF
MAP CRACKED AREAS: 234.0 SF + 3.0 SF + 406.9 SF = 643.9 SF
TOTAL LENGTH OF CRACKS: 0.0 LF

PROJECT ENGINEER: F. PUGH
IN CHARGE OF: F. PUGH
DESIGNED BY: D. SMITH
DESIGN CHECKED BY: F. PUGH
DETAILED BY: D. SMITH
DETAIL CHECKED BY: K. J. JAMES

**ELEVATION
Scale 1/4" = 1'-0"**

BRIDGE NO. 2
INTERSTATE ROUTE 481 OVER DEWITT YARDS
SOUTHBOUND BIN 1093571
SOUTH ABUTMENTS
DRAWING NO. 5 OF 47

NOT TO SCALE



- LEGEND**
- Hollow Concrete (HC)
 - Spall (S)
 - Patch/Repair
 - Crack (C)
 - Hairline Crack (HL)
 - Core Location

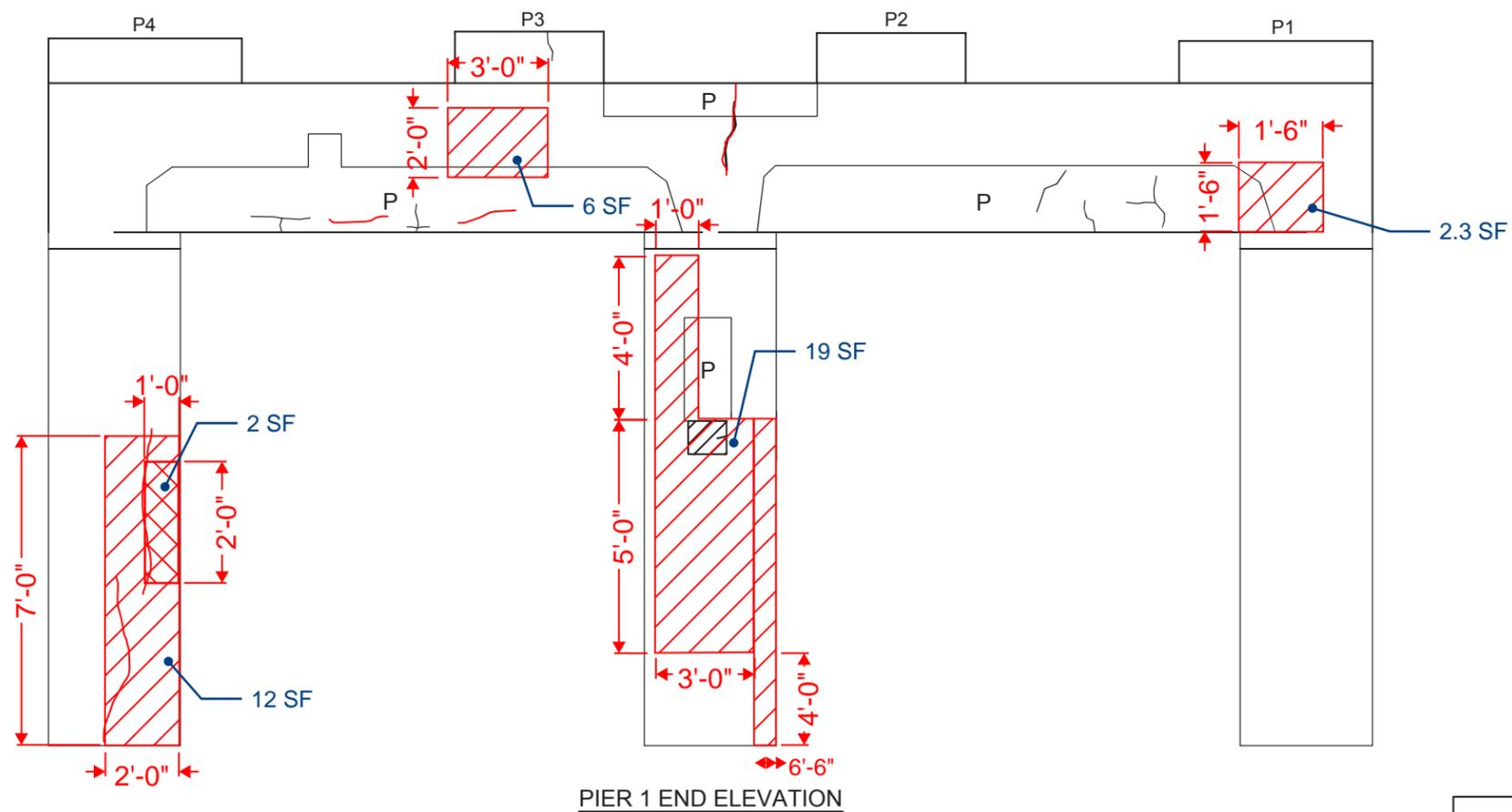
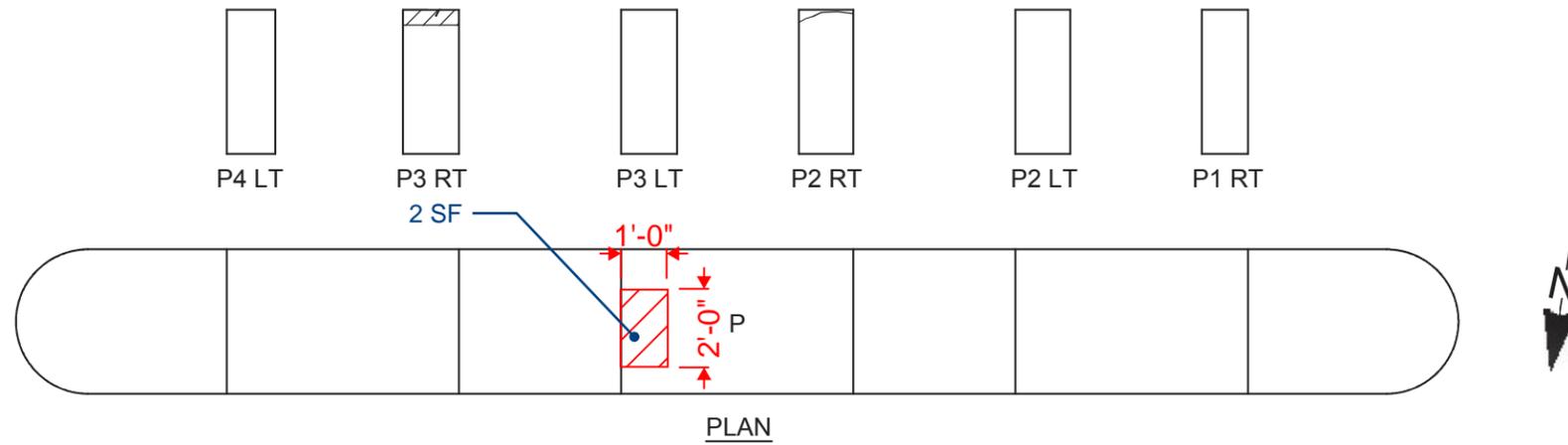
QUANTITIES:
 SPALL AREAS: 2 SF + 7.5 SF + 7 SF = **16.5 SF**
 HOLLOW AREAS: 11.2 SF + 18.4 SF + 2.6 SF + 32.5 SF + 51 SF + 24.5 SF + 10.5 SF + 11 SF + 27 SF + 6.9 SF + 4.8 SF + 8.8 SF = **209.2 SF**
 MAP CRACKED AREAS: **10.7 SF**
 TOTAL LENGTH OF CRACKS: **26 LF**

- LEGEND**
- Hollow Concrete (HC)
 - Spall (S)
 - Patch
 - Crack (C)
 - Hairline Crack (HL)

PIER 1 DETERIORATION SKETCH

NOT TO SCALE

* NOTE: P1 LT & P4 RT VIEWS LOCATED ON
 PIER 1 DETERIORATION SKETCH
 SHEET 1 OF 2



LEGEND

-  Hollow Concrete (HC)
-  Spall (S)
-  Patch/Repair
-  Crack (C)
-  Hairline Crack (HL)
-  Core Location

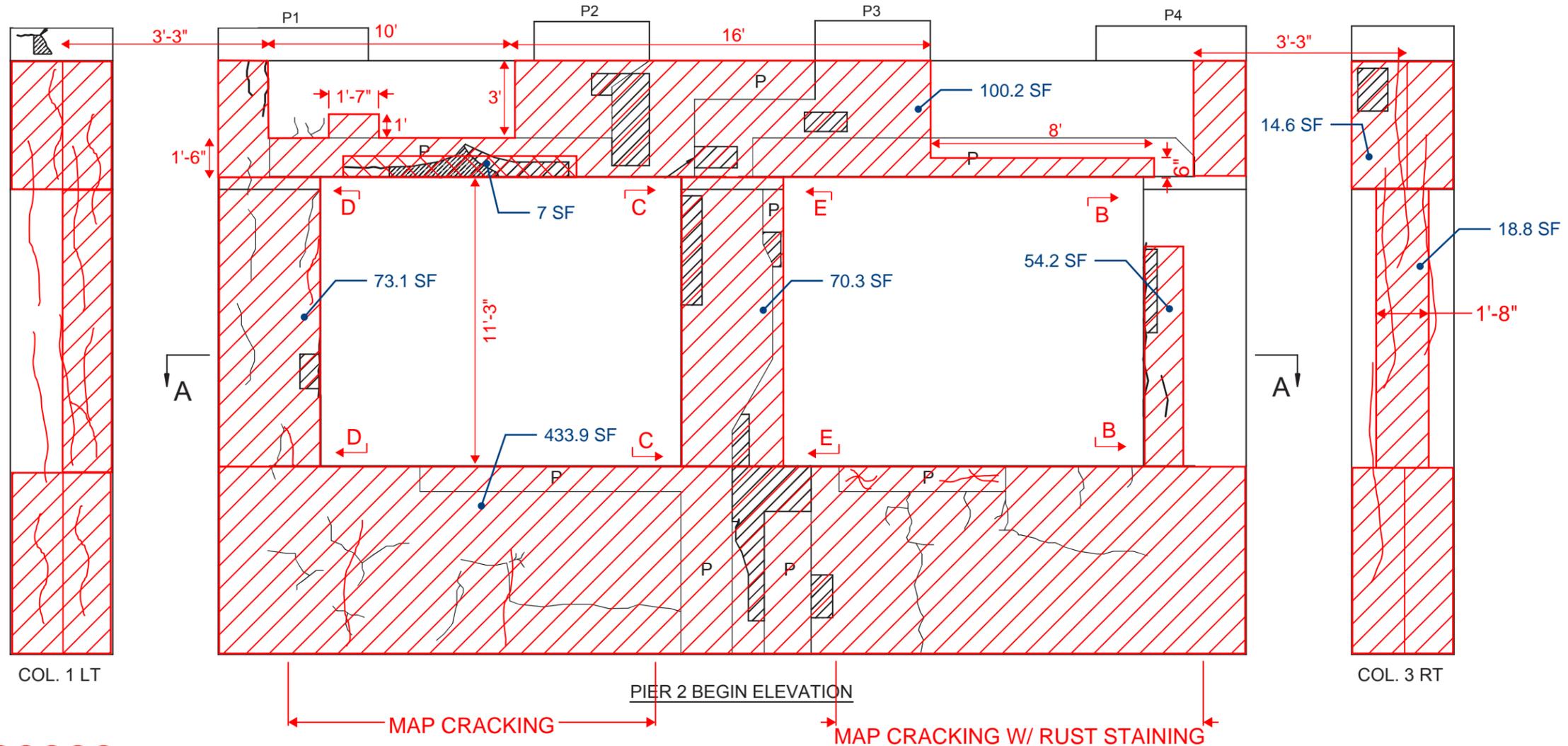
LEGEND

-  Hollow Concrete (HC)
-  Spall (S)
-  Patch
-  Crack (C)
-  Hairline Crack (HL)

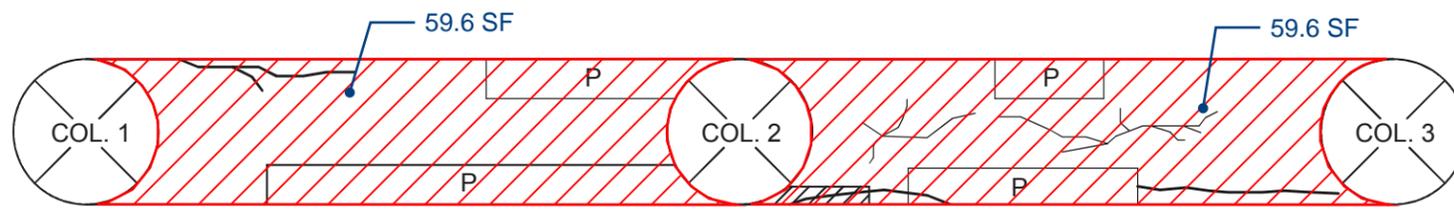
QUANTITIES:
 SPALL AREAS: 2 SF
 HOLLOW AREAS: 12 SF + 19 SF + 6 SF + 2 SF + 2.3 SF = 41.3 SF
 MAP CRACKED AREAS: 0
 TOTAL LENGTH OF CRACKS: 15 LF

PIER 2 DETERIORATION SKETCH

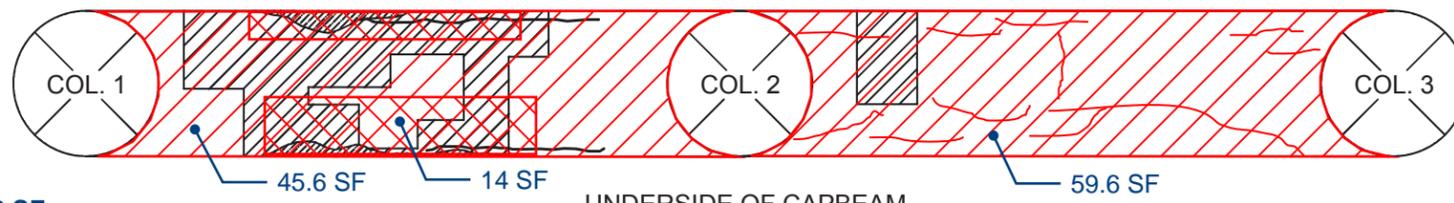
NOT TO SCALE



SEE NEXT PAGE FOR ELEVATIONS B-B, C-C, D-D, AND E-E



TOP OF STEM SOLID PIER SECTION A-A



UNDERSIDE OF CAPBEAM

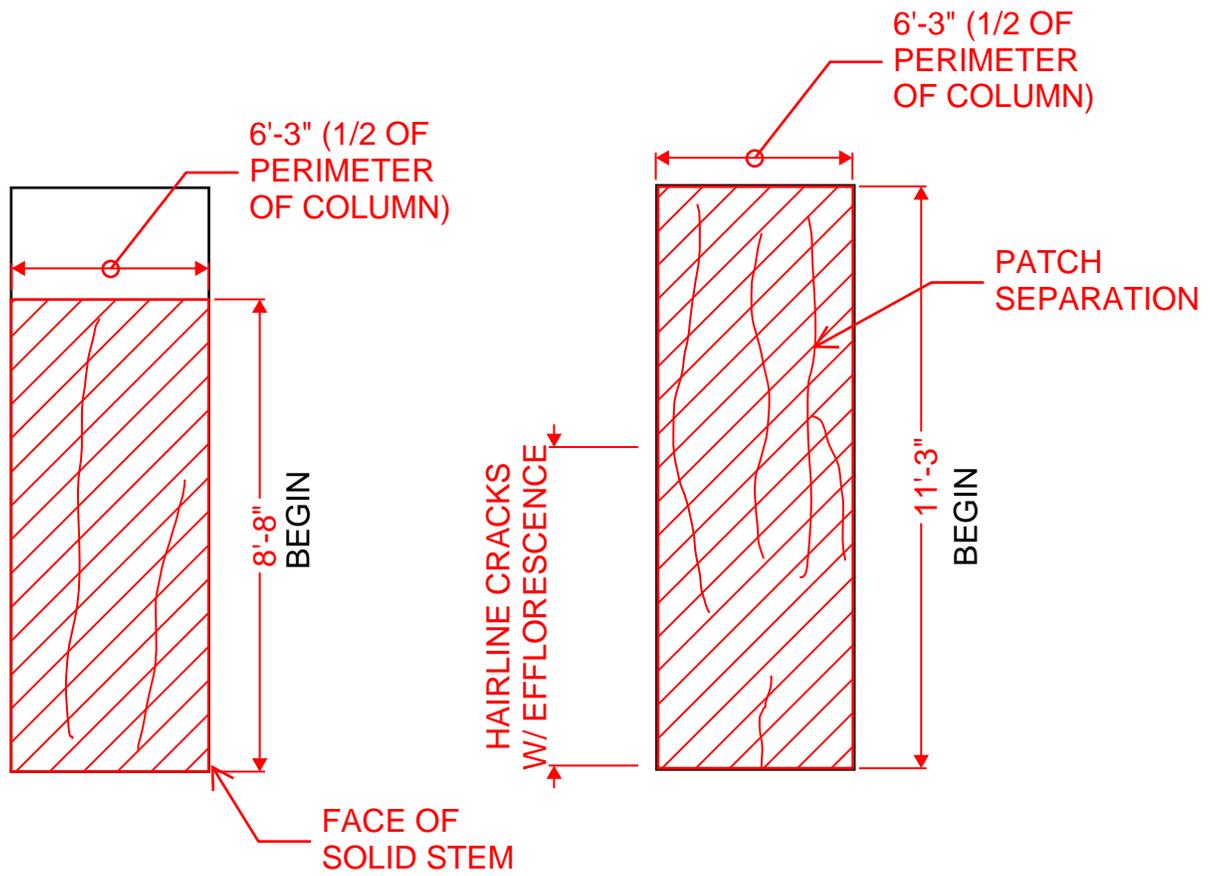
LEGEND

- Hollow Concrete (HC)
- Spall (S)
- Patch/Repair
- Crack (C)
- Hairline Crack (HL)

LEGEND

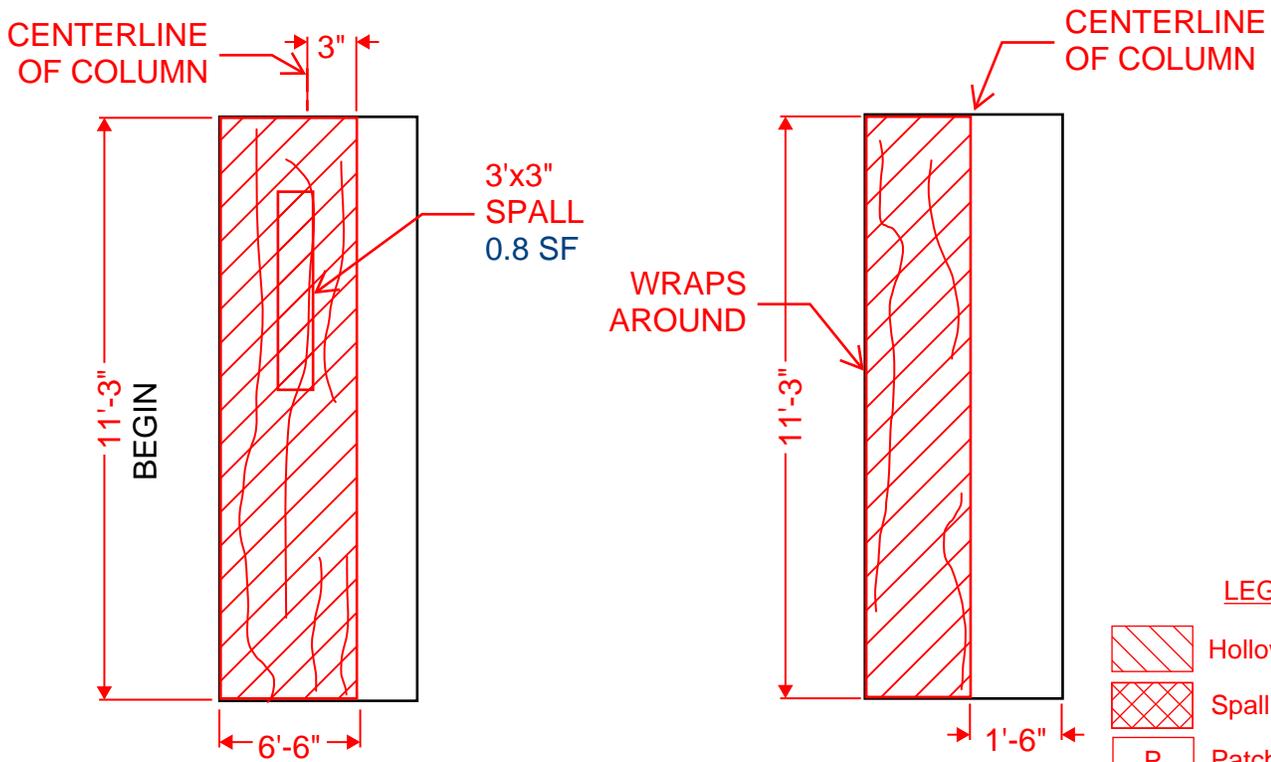
- Hollow Concrete (HC)
- Spall (S)
- Patch
- Crack (C)
- Hairline Crack (HL)

QUANTITIES:
 SPALL AREAS: 7 SF + 14 SF + 0.8 SF = **21.8 SF**
 HOLLOW AREAS: 73.1 SF + 70.3 SF + 54.2 SF + 18.8 SF + 100.2 SF + 14.6 SF + 433.9 SF + 59.6 SF + 59.6 SF + 45.6 SF + 59.6 SF = **989.5 SF**
 MAP CRACKED AREAS: **0 SF**
 TOTAL LENGTH OF CRACKS: **19 LF**



ELEVATION B-B

ELEVATION C-C



ELEVATION D-D

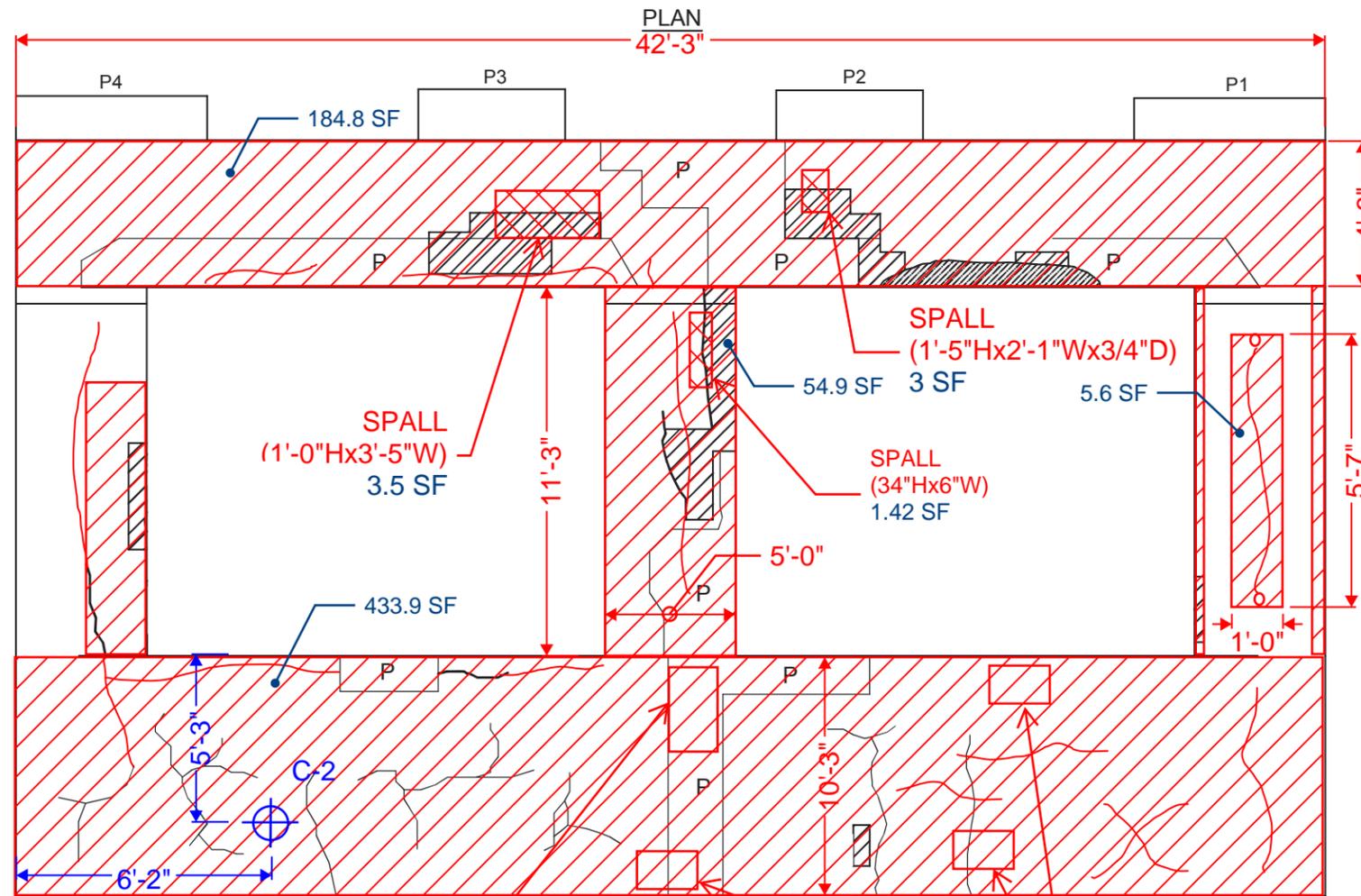
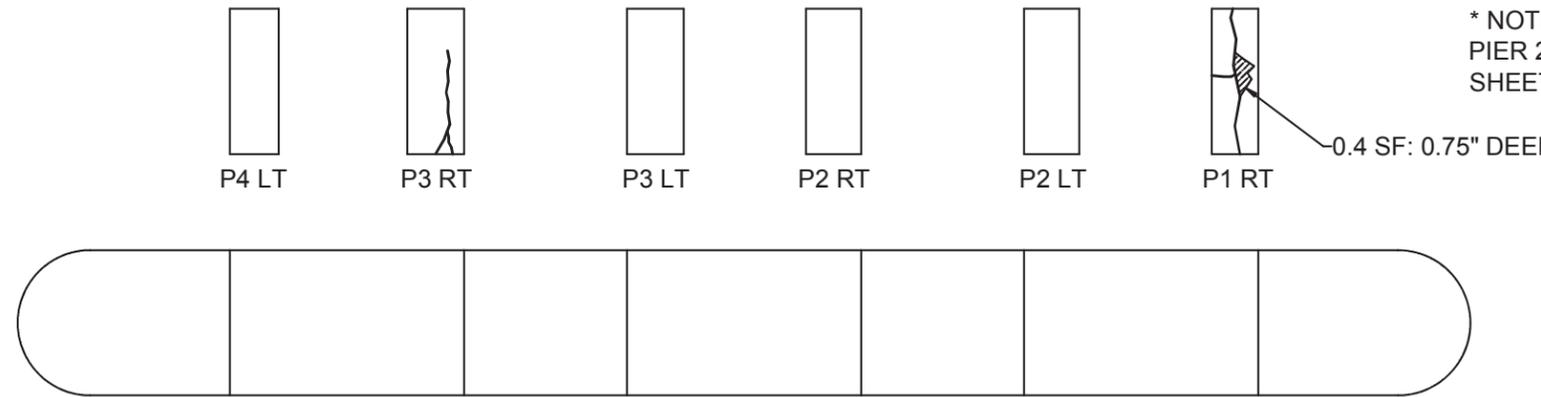
ELEVATION E-E

LEGEND

-  Hollow Concrete (HC)
-  Spall (S)
-  Patch/Repair
-  Crack (C)
-  Hairline Crack (HL)

PIER 2 DETERIORATION SKETCH

NOT TO SCALE



LEGEND

	Hollow Concrete (HC)
	Spall (S)
	Patch/Repair
	Crack (C)
	Hairline Crack (HL)
	Core Location

LEGEND

	Hollow Concrete (HC)
	Spall (S)
	Patch
	Crack (C)
	Hairline Crack (HL)

QUANTITIES:
 SPALL AREAS: 3.5 SF + 3 SF = **7.5 SF**
 HOLLOW AREAS: 184.8 SF + 54.9 SF + 5.6 SF + 433.9 SF = **679.2 SF**
 MAP CRACKED AREAS: **0 SF**
 TOTAL LENGTH OF CRACKS: **6 LF**

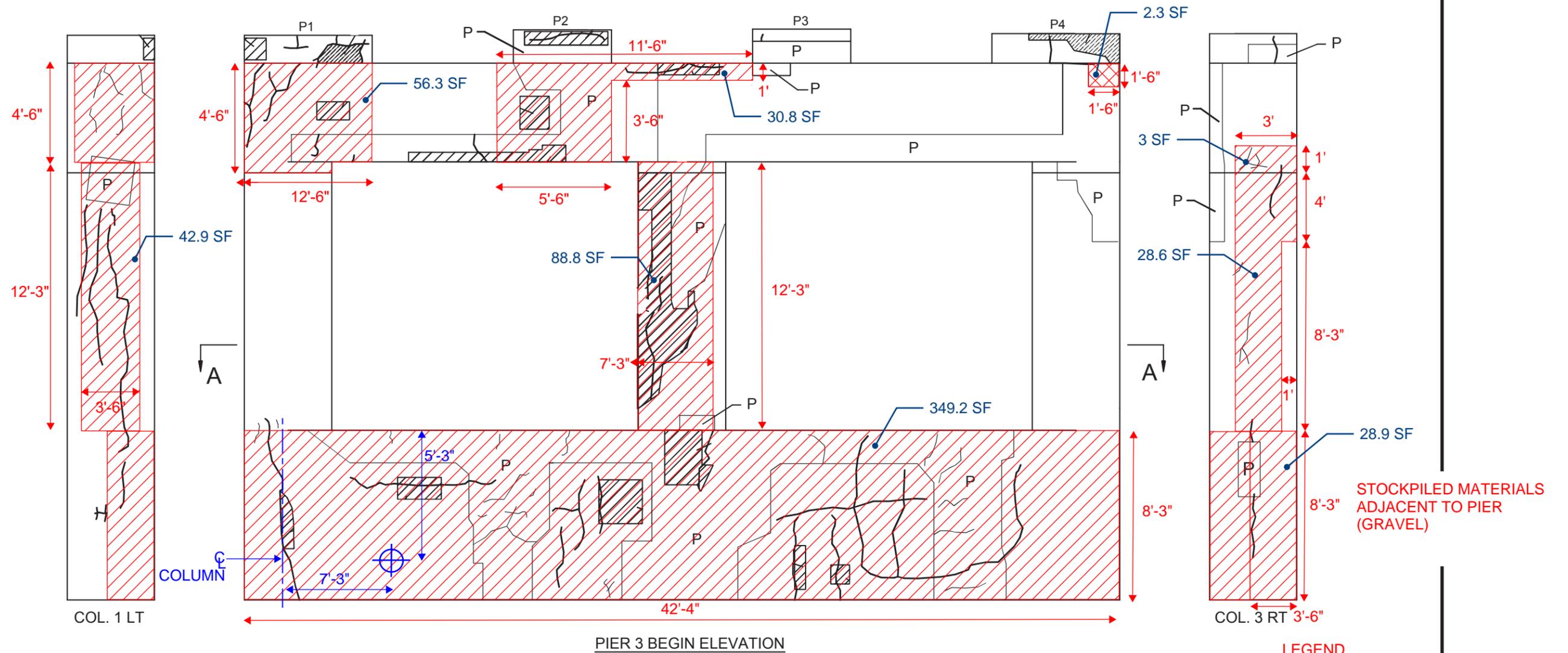
MAP CRACKING WITH EFFLORESCENCE

PIER 2 END ELEVATION

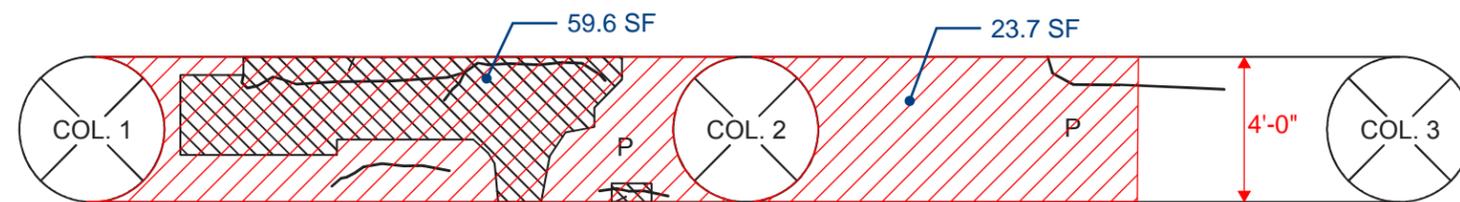
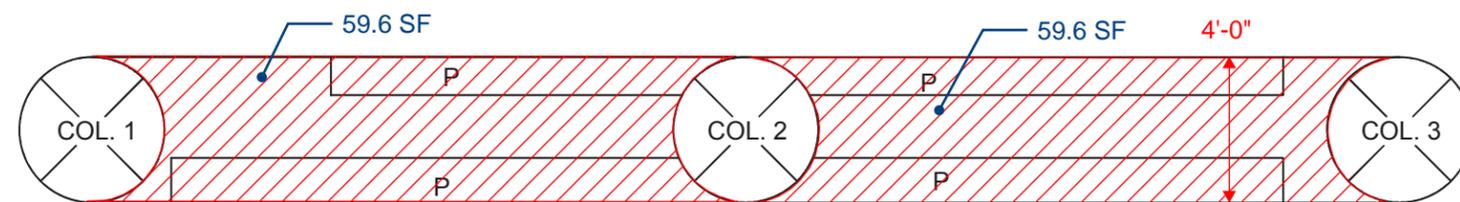
RUST STAINS

PIER 3 DETERIORATION SKETCH

NOT TO SCALE



STOCKPILED MATERIALS
ADJACENT TO PIER
(GRAVEL)



LEGEND

- Hollow Concrete (HC)
- Spall (S)
- Patch/Repair
- Crack (C)
- Hairline Crack (HL)
- Core Location

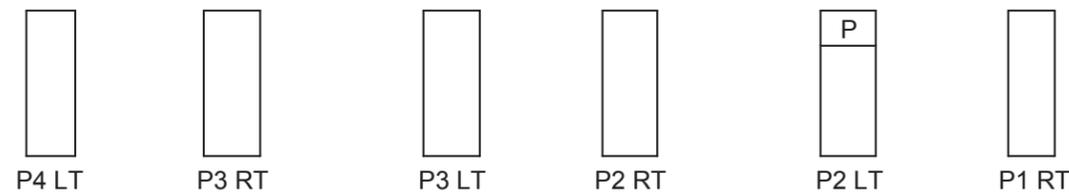
LEGEND

- Hollow Concrete (HC)
- Spall (S)
- Patch
- Crack (C)
- Hairline Crack (HL)

QUANTITIES:
 SPALL AREAS: 2.3 SF
 HOLLOW AREAS: 42.9 SF + 88.8 SF + 28.6 SF + 56.3 SF + 30.8 SF + 3 SF + 349.2 SF + 28.9 SF + 59.6 SF + 59.6 SF + 59.6 SF + 23.7 SF = 831.0 SF
 MAP CRACKED AREAS: 0 SF
 TOTAL LENGTH OF CRACKS: 5 LF

PIER 3 DETERIORATION SKETCH

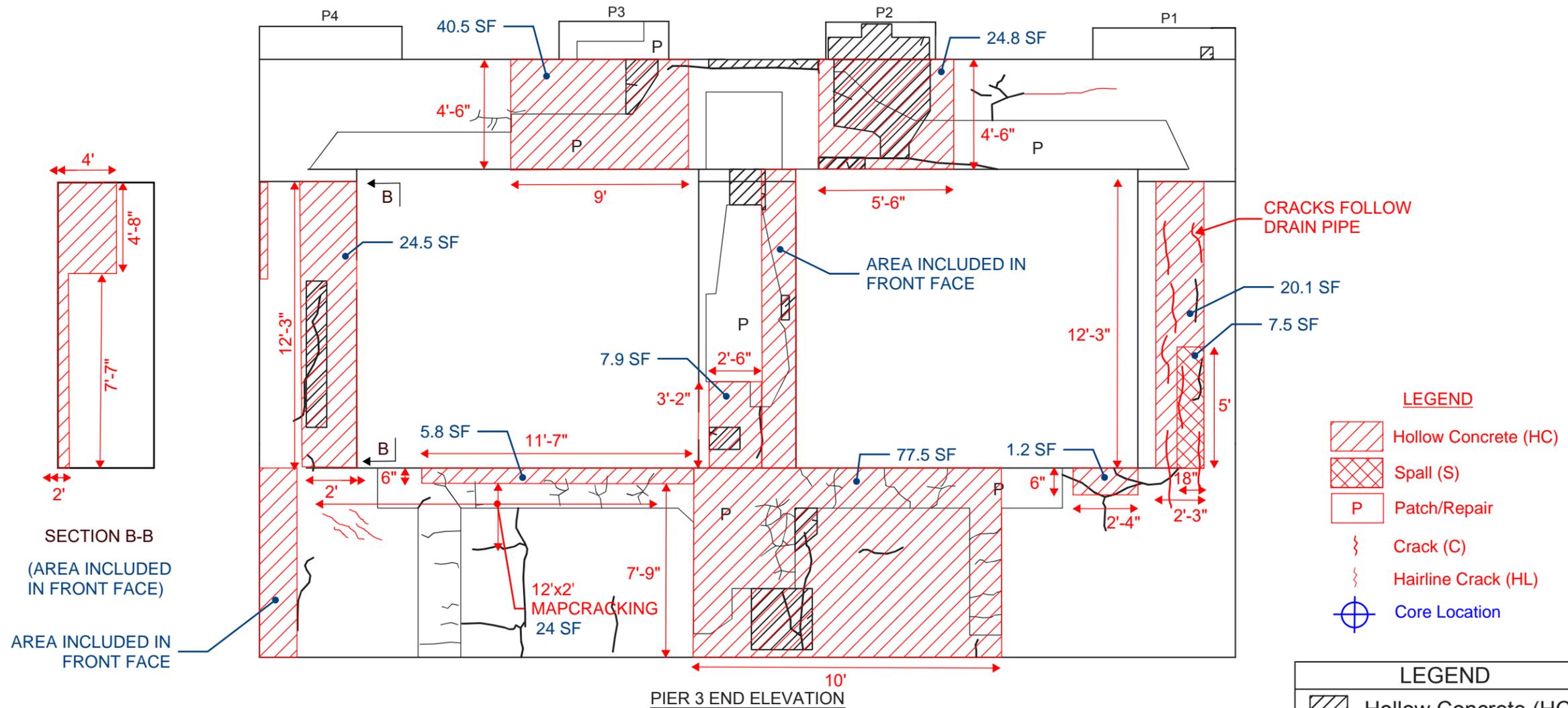
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* NOTE: P1 LT & P4 RT VIEWS LOCATED ON
PIER 3 DETERIORATION SKETCH
SHEET 1 OF 2



PLAN



QUANTITIES:

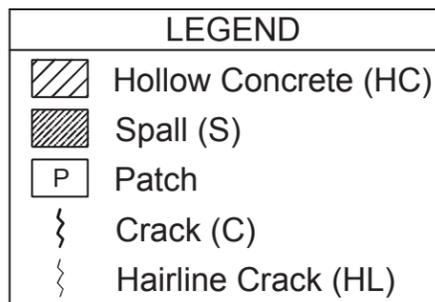
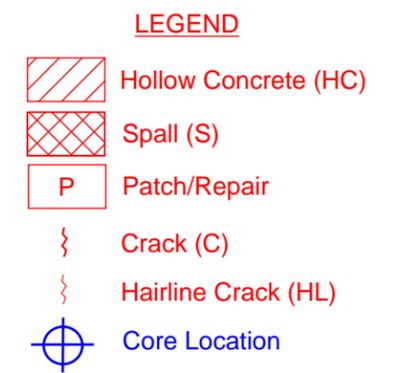
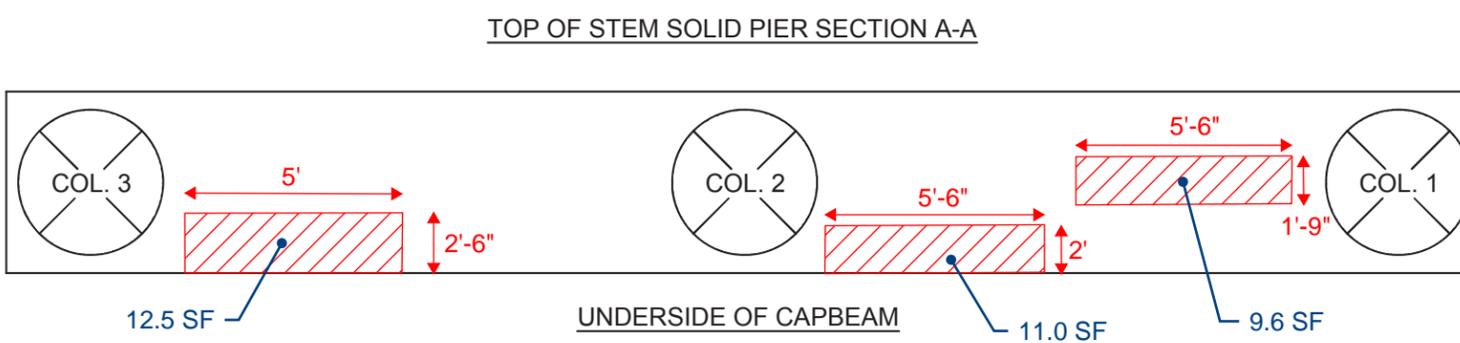
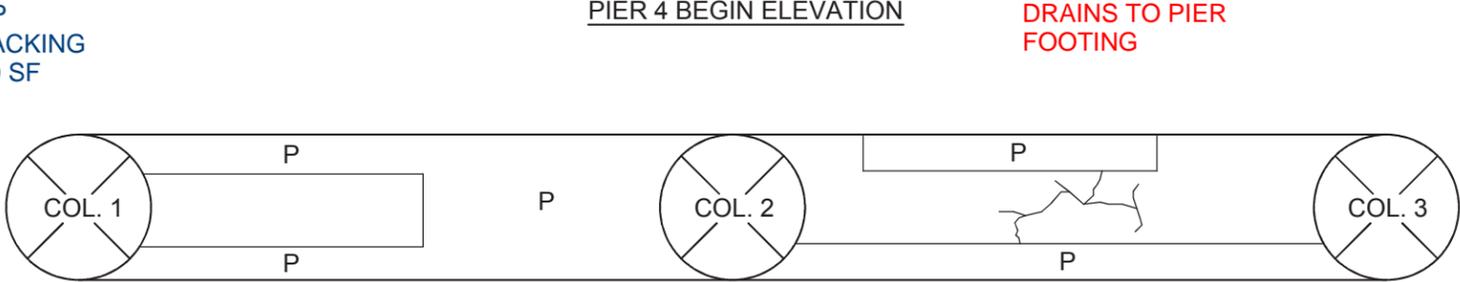
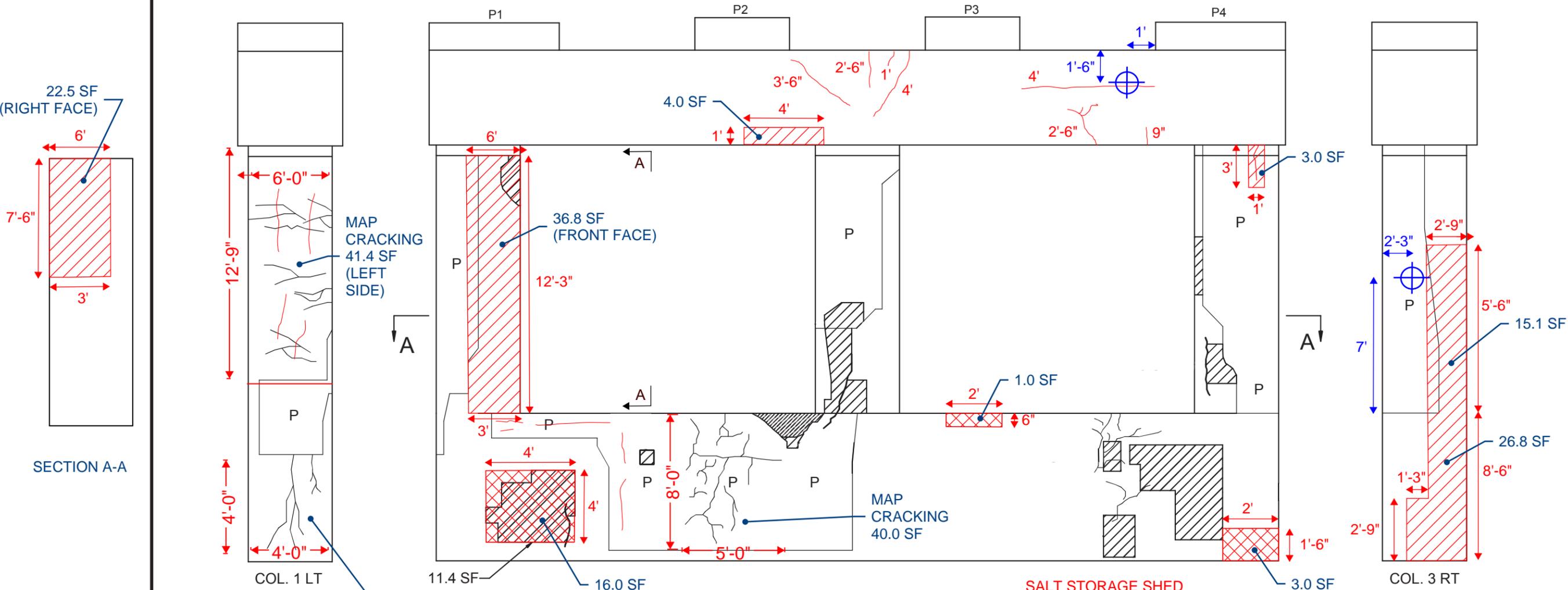
SPALL AREAS: 7.5 SF

HOLLOW AREAS: 40.5 SF + 24.8 SF + 24.51 SF + 5.8 SF + 7.9 SF + 77.5 SF + 1.2 SF + 20.1 SF = 225.7 SF

MAP CRACKED AREAS: 24 SF

TOTAL LENGTH OF CRACKS: 38 LF

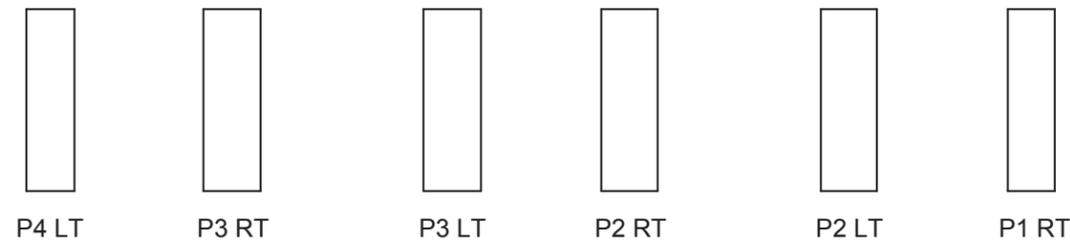
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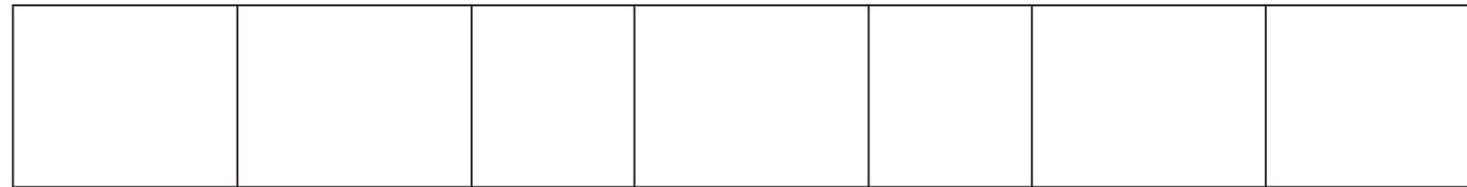
QUANTITIES:
 SPALL AREAS: 16.0 SF + 3.0 SF + 1.0 SF = 20 SF
 HOLLOW AREAS: 22.5 SF + 36.8 SF + 4.0 SF + 3.0 SF + 15.1 SF + 26.8 SF + 12.5 SF + 11.0 SF + 9.6 SF = 141.3 SF
 MAP CRACKED AREAS: 41.4 SF + 16.0 SF + 40.0 SF = 97.4 SF
 TOTAL LENGTH OF CRACKS: 13.3 LF

PIER 4 DETERIORATION SKETCH

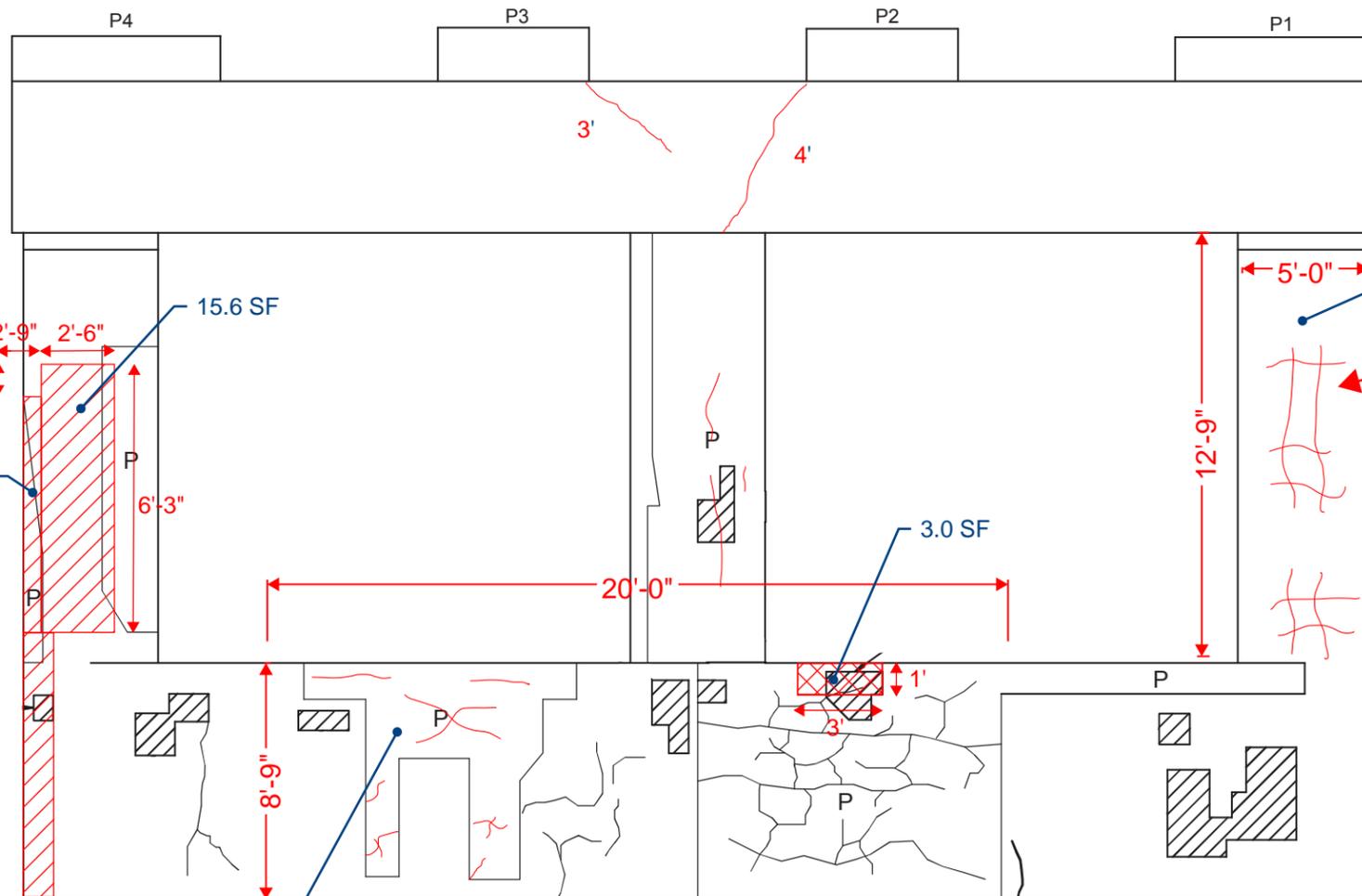
NOT TO SCALE



* NOTE: P1 LT & P4 RT VIEWS LOCATED ON
 PIER 4 DETERIORATION SKETCH
 SHEET 1 OF 2



PLAN



ACCOUNTED FOR
 ON SIDE FACE

15.6 SF

MAP CRACKING
 65.0 SF

COLUMN MAP CRACKED
 THROUGHOUT

3.0 SF

MAP CRACKING
 175.0 SF

LEGEND

- Hollow Concrete (HC)
- Spall (S)
- Patch/Repair
- Crack (C)
- Hairline Crack (HL)
- Core Location

LEGEND

- Hollow Concrete (HC)
- Spall (S)
- Patch
- Crack (C)
- Hairline Crack (HL)

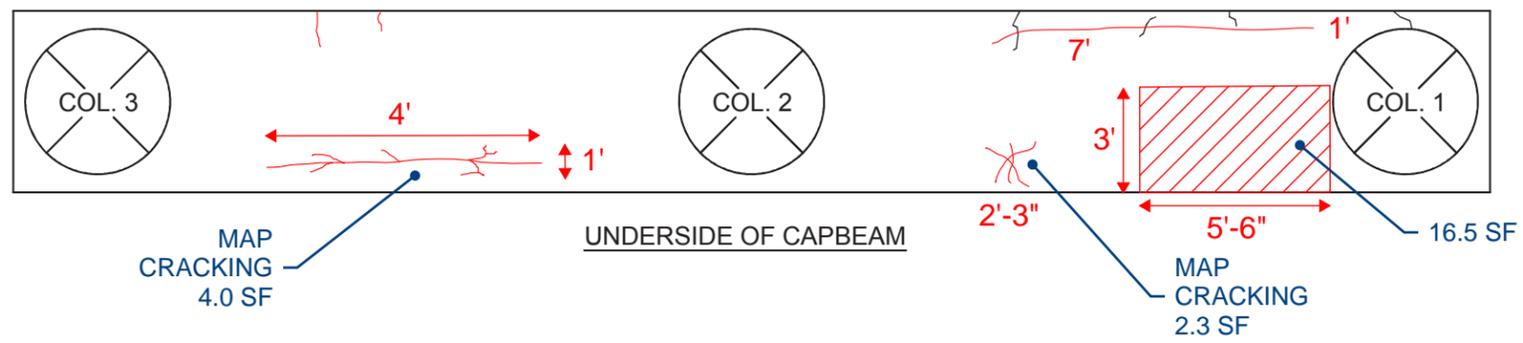
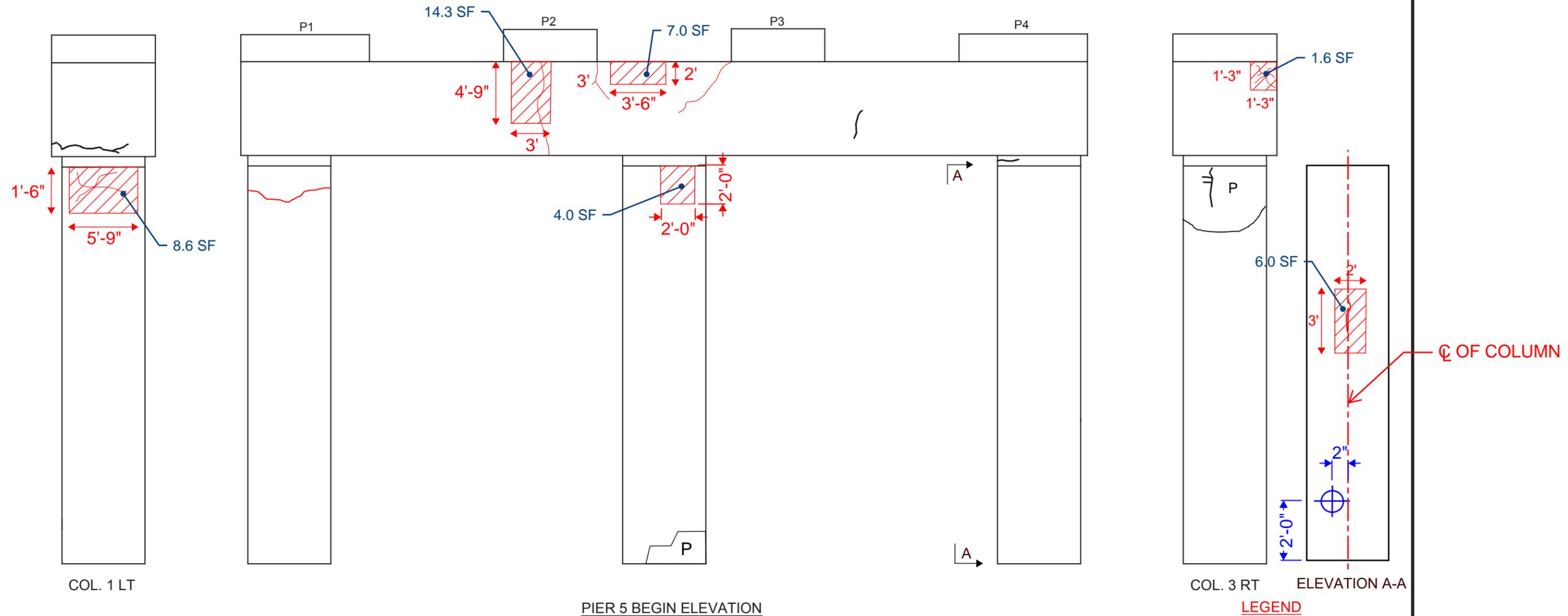
PIER 4 END ELEVATION

QUANTITIES:

SPALL AREAS: 3.0 SF
 HOLLOW AREAS: 15.6 SF
 MAP CRACKED AREAS: 175.0 SF + 41.4 SF = 216.4 SF
 TOTAL LENGTH OF CRACKS: 18.0 LF

PIER 5 DETERIORATION SKETCH

NOT TO SCALE



QUANTITIES:
 SPALL AREAS: 0.0 SF
 HOLLOW AREAS: 8.6 SF + 14.3 SF + 7.0 SF + 4.0 SF + 1.6 SF + 6.0 SF + 16.5 SF = 57.9 SF
 MAP CRACKED AREAS: 4.0 SF + 2.3 SF = 6.3 SF
 TOTAL LENGTH OF CRACKS: 25.0 LF

LEGEND

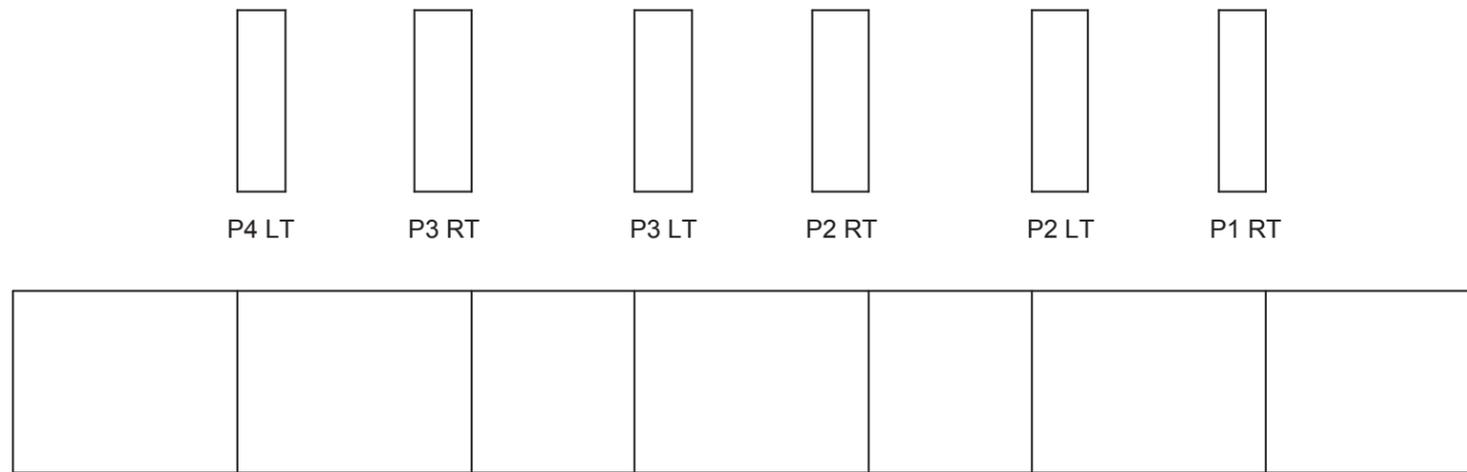
- Hollow Concrete (HC)
- Spall (S)
- Patch/Repair
- Crack (C)
- Hairline Crack (HL)
- Core Location

LEGEND

- Hollow Concrete (HC)
- Spall (S)
- Patch
- Crack (C)
- Hairline Crack (HL)

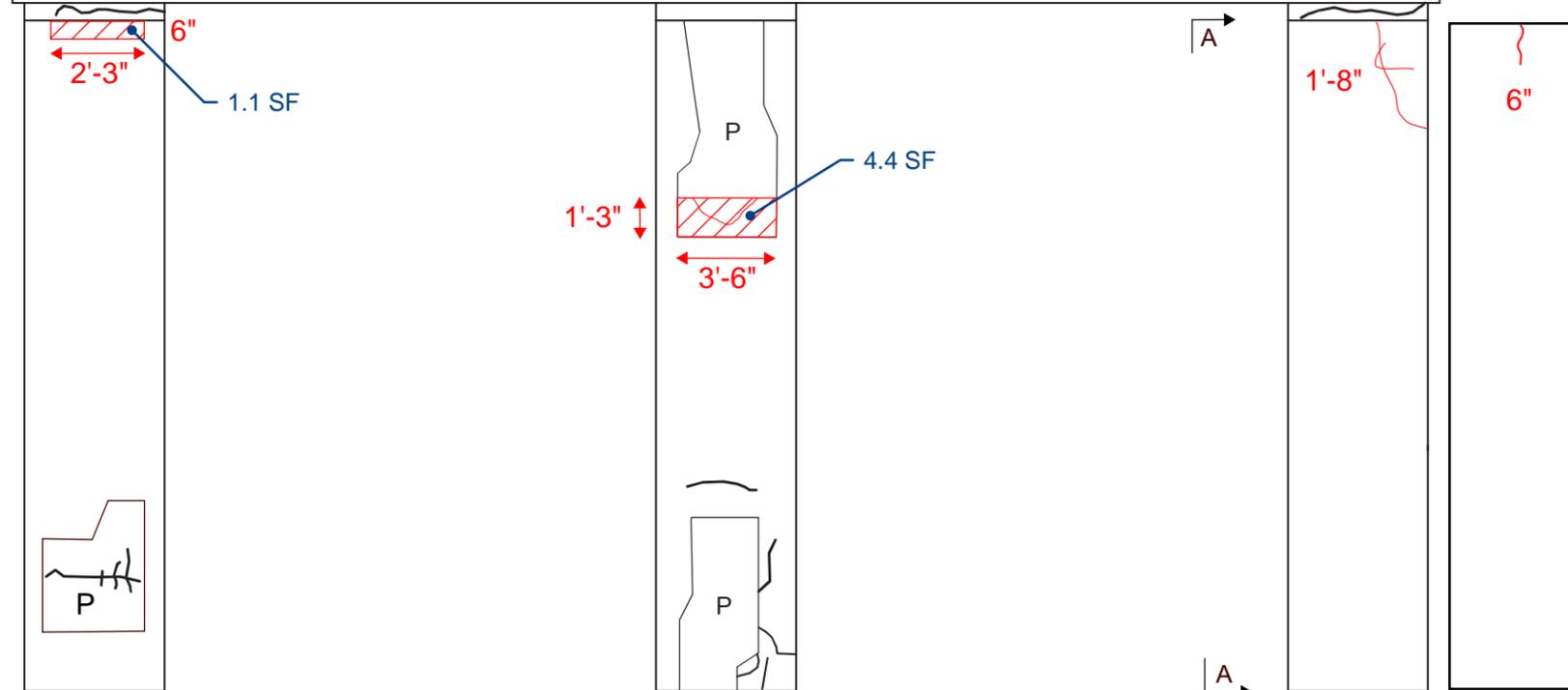
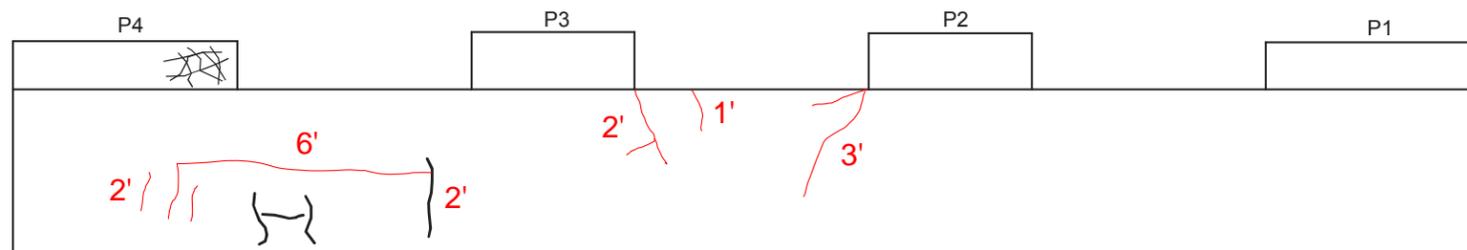
PIER 5 DETERIORATION SKETCH

NOT TO SCALE



* NOTE: P1 LT & P4 RT VIEWS LOCATED ON
PIER 5 DETERIORATION SKETCH
SHEET 1 OF 2

PLAN



PIER 5 END ELEVATION

SECTION A-A

LEGEND

-  Hollow Concrete (HC)
-  Spall (S)
-  Patch/Repair
-  Crack (C)
-  Hairline Crack (HL)
-  Core Location

LEGEND

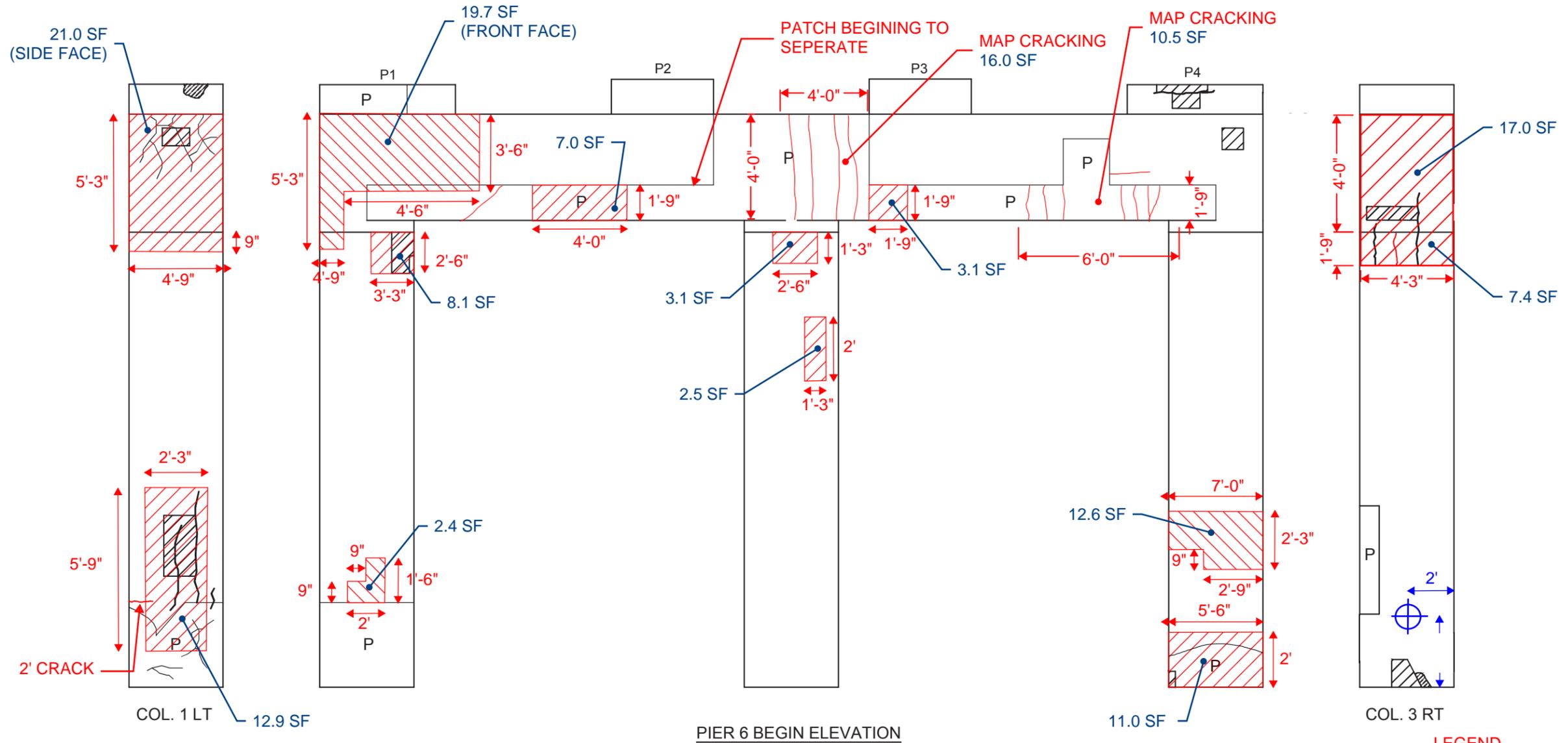
-  Hollow Concrete (HC)
-  Spall (S)
-  Patch
-  Crack (C)
-  Hairline Crack (HL)

QUANTITIES:

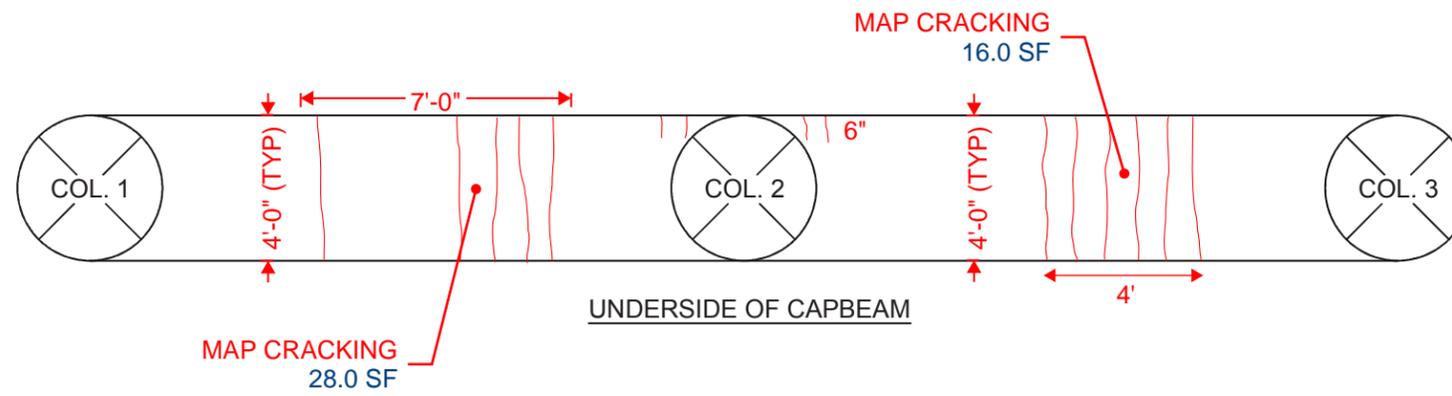
SPALL AREAS: 0.0 SF
HOLLOW AREAS: 1.1 SF + 4.4 SF = 5.5 SF
MAP CRACKED AREAS: 0.0 SF
TOTAL LENGTH OF CRACKS: 27.0 LF

PIER 6 DETERIORATION SKETCH

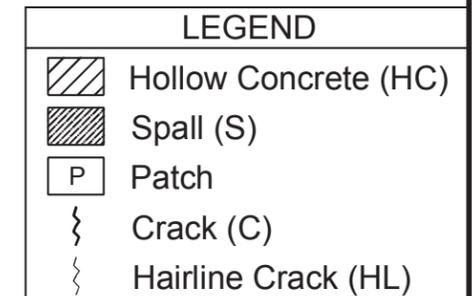
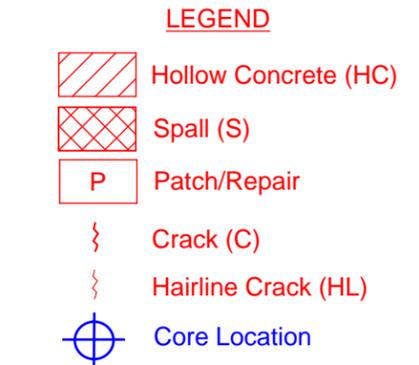
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PIER 6 BEGIN ELEVATION



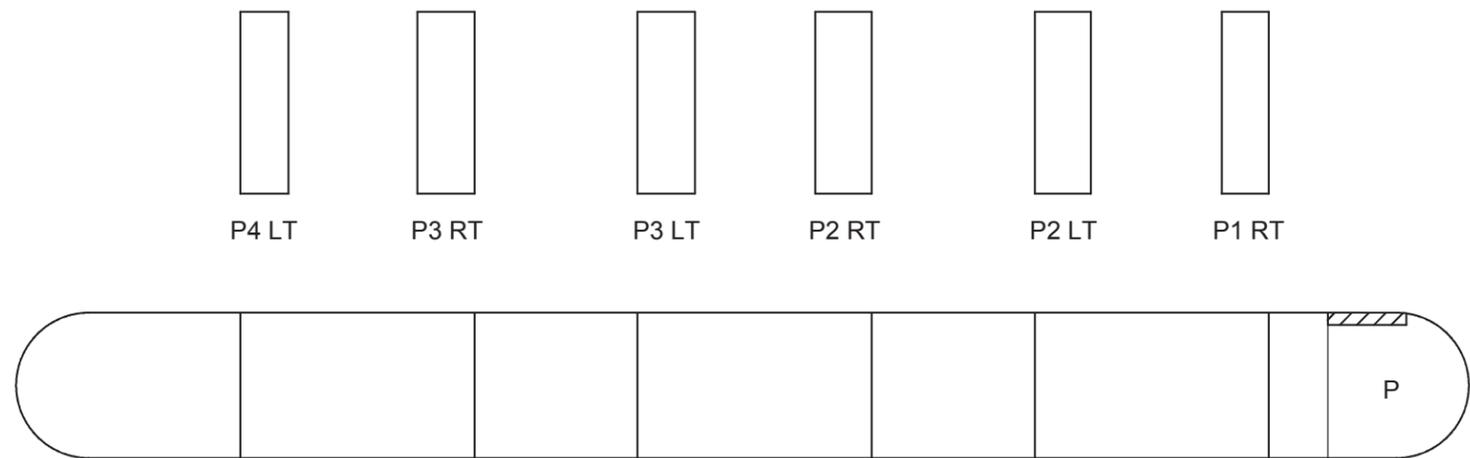
UNDERSIDE OF CAPBEAM



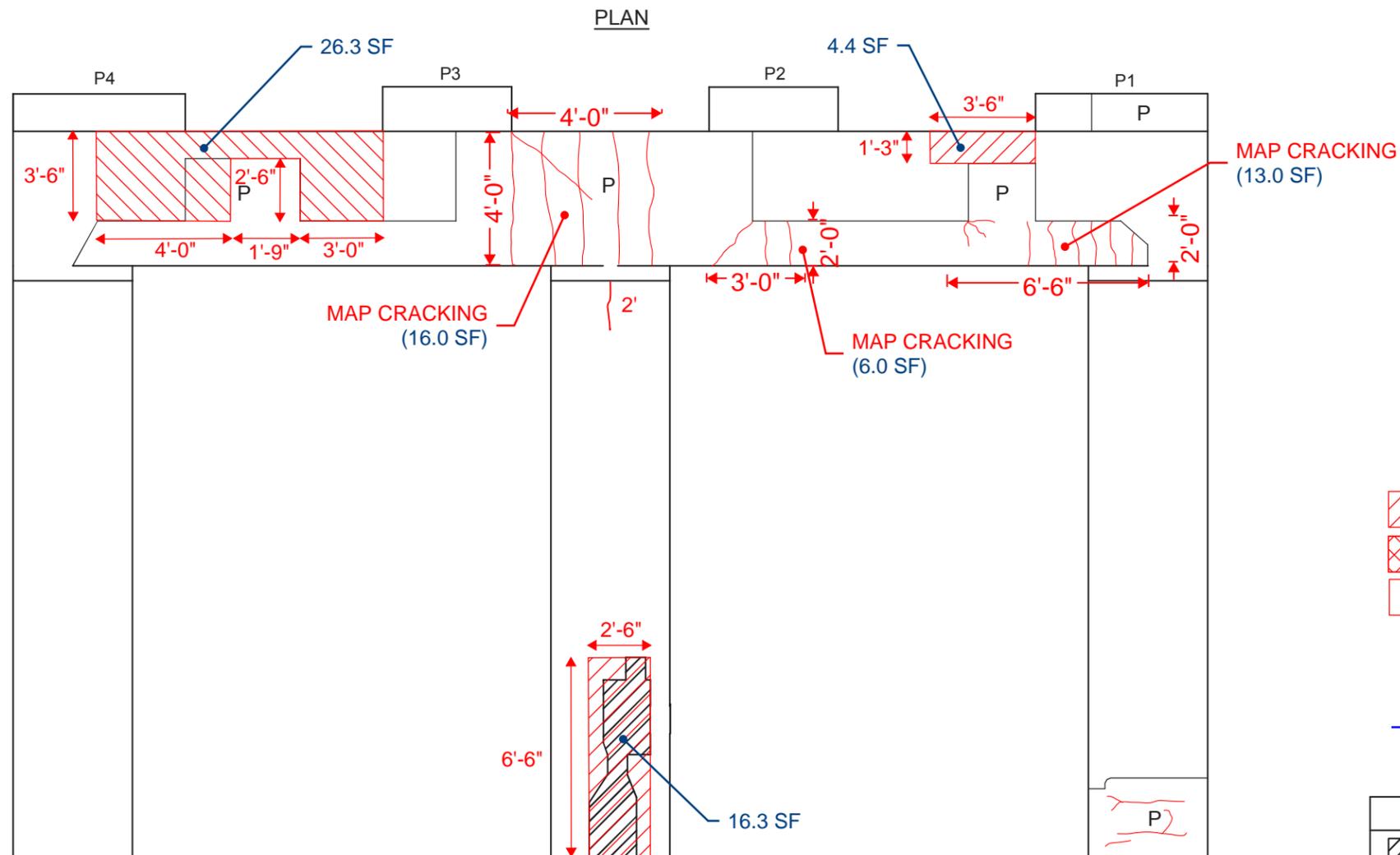
QUANTITIES:
 SPALL AREAS: 0.0 SF
 HOLLOW AREAS: 21.0 SF + 19.7 SF + 12.9 SF + 8.1 SF + 2.4 SF + 7.0 SF + 3.1 SF + 2.5 SF + 3.1 SF + 12.6 SF + 11.0 SF + 17.0 SF + 7.4 SF = 127.9 SF
 MAP CRACKED AREAS: 16.0 SF + 10.5 SF + 28.0 SF + 16.0 SF = 70.5 SF
 TOTAL LENGTH OF CRACKS: 5.0 LF

PIER 6 DETERIORATION SKETCH

NOT TO SCALE



* NOTE: P1 LT & P4 RT VIEWS LOCATED ON
PIER 6 DETERIORATION SKETCH
SHEET 1 OF 2



PIER 6 END ELEVATION

QUANTITIES:

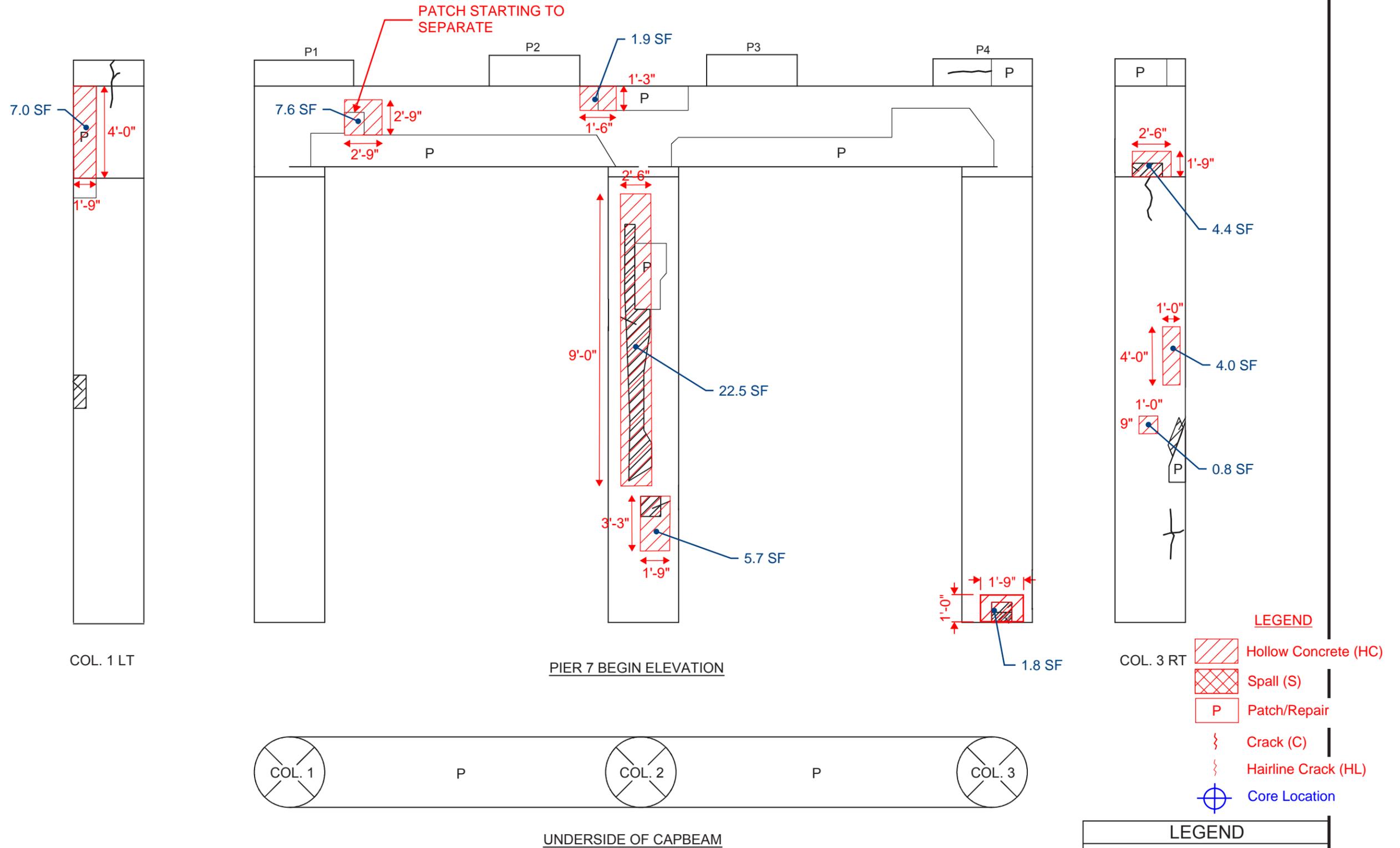
SPALL AREAS: 0.0 SF
 HOLLOW AREAS: 26.3 SF + 4.4 SF + 16.3 SF = 46.9 SF
 MAP CRACKED AREAS: 16.0 SF + 6.0 SF + 13.0 SF = 35.0 SF
 TOTAL LENGTH OF CRACKS: 9.0 LF

LEGEND	
	Hollow Concrete (HC)
	Spall (S)
	Patch/Repair
	Crack (C)
	Hairline Crack (HL)
	Core Location

LEGEND	
	Hollow Concrete (HC)
	Spall (S)
	Patch
	Crack (C)
	Hairline Crack (HL)

PIER 7 DETERIORATION SKETCH

NOT TO SCALE



QUANTITIES:

SPALL AREAS: 0.0 SF

HOLLOW AREAS: 7.0 SF + 7.6 SF + 5.7 SF + 22.5 SF + 1.9 SF + 1.8 SF + 0.8 SF + 4.0 SF + 4.4 SF = 55.5 SF

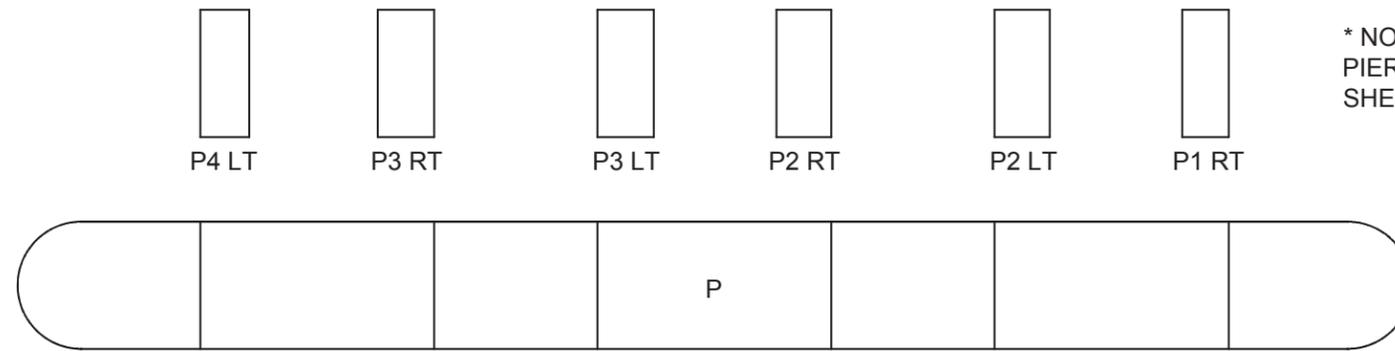
MAP CRACKED AREAS: 0.0 SF

TOTAL LENGTH OF CRACKS: 6.0 LF

LEGEND	
	Hollow Concrete (HC)
	Spall (S)
	Patch
	Crack (C)
	Hairline Crack (HL)

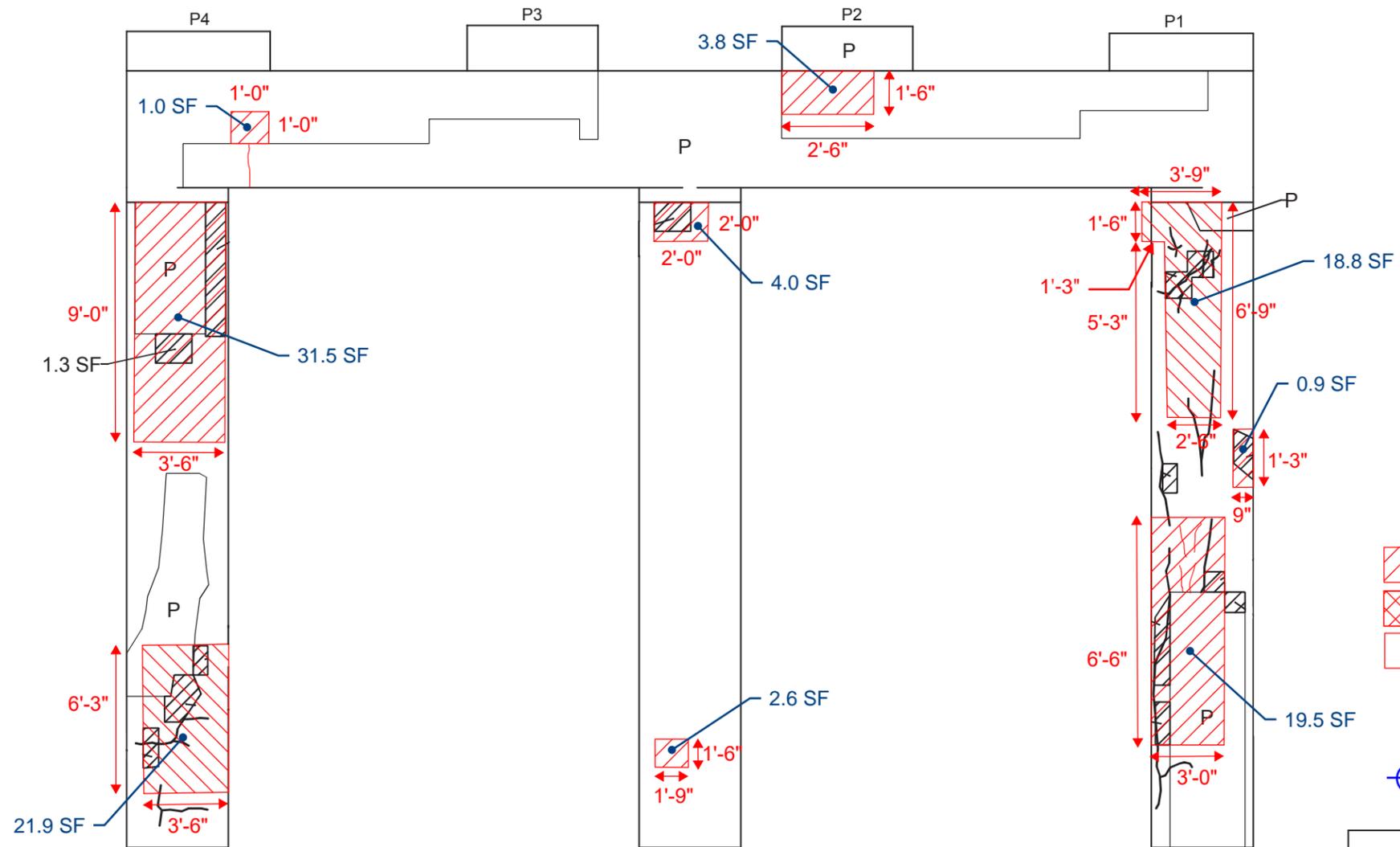
PIER 7 DETERIORATION SKETCH

NOT TO SCALE



* NOTE: P1 LT & P4 RT VIEWS LOCATED ON
PIER 7 DETERIORATION SKETCH
SHEET 1 OF 2

PLAN



PIER 7 END ELEVATION

LEGEND

- Hollow Concrete (HC)
- Spall (S)
- Patch/Repair
- Crack (C)
- Hairline Crack (HL)
- Core Location

LEGEND

- Hollow Concrete (HC)
- Spall (S)
- Patch
- Crack (C)
- Hairline Crack (HL)

QUANTITIES:

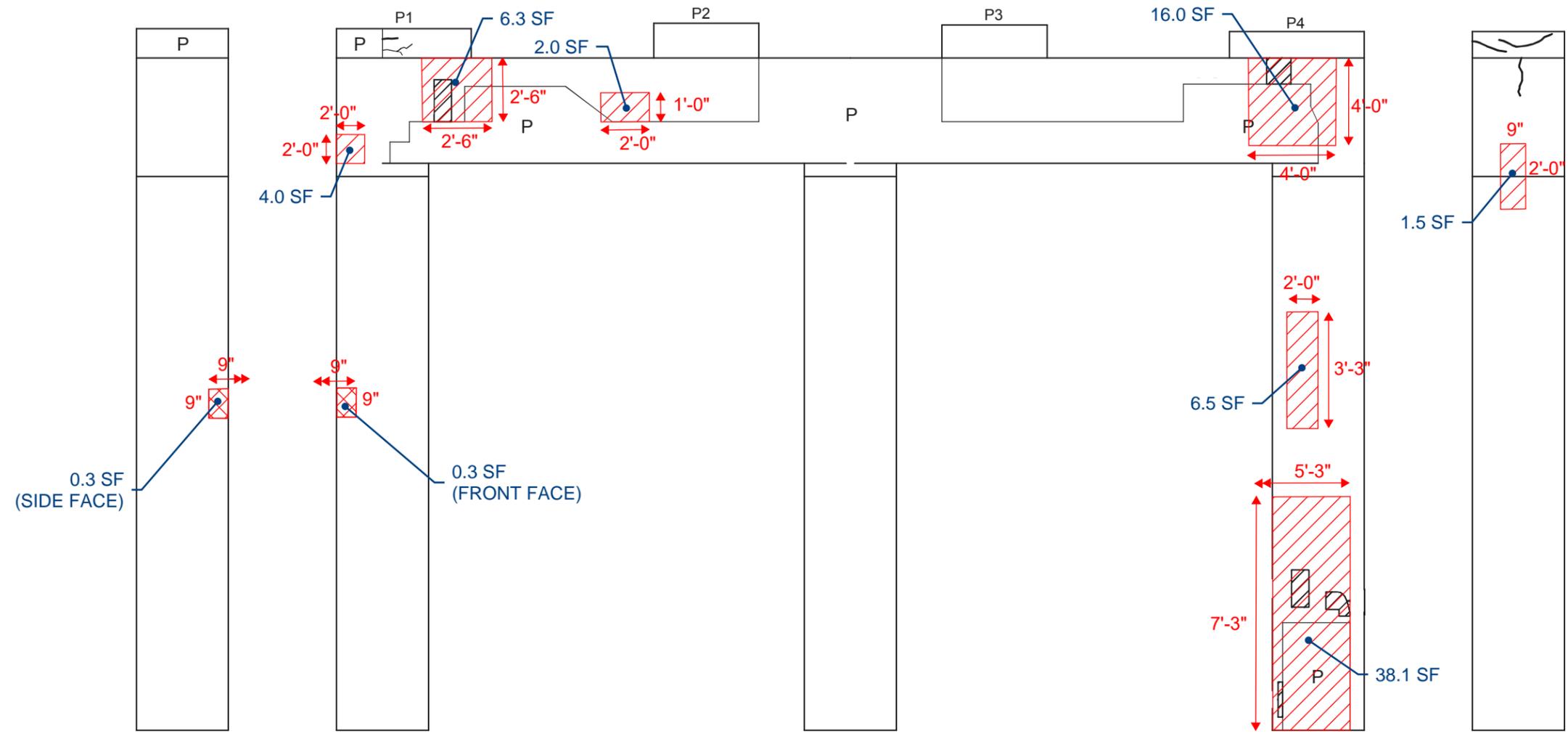
SPALL AREAS: 0.0 SF

HOLLOW AREAS: 21.9 SF + 31.5 SF + 1.0 SF + 2.6 SF + 4.0 SF + 3.8 SF + 19.5 SF + 0.9 SF + 18.8 SF = 103.9 SF

MAP CRACKED AREAS: 0.0 SF

TOTAL LENGTH OF CRACKS: 15.0 LF

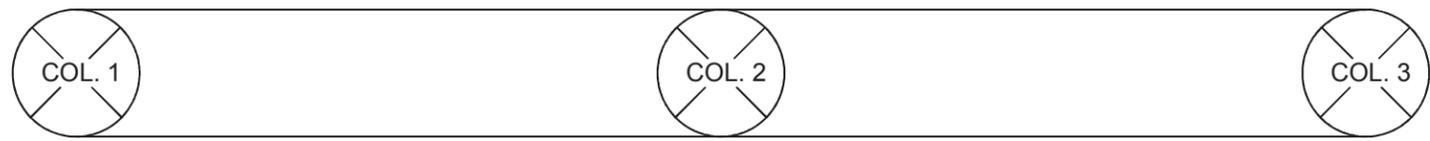
NOT TO SCALE



COL. 1 LT

PIER 8 BEGIN ELEVATION

COL. 3 RT



UNDERSIDE OF CAPBEAM

LEGEND

-  Hollow Concrete (HC)
-  Spall (S)
-  Patch/Repair
-  Crack (C)
-  Hairline Crack (HL)
-  Core Location

LEGEND

-  Hollow Concrete (HC)
-  Spall (S)
-  Patch
-  Crack (C)
-  Hairline Crack (HL)

QUANTITIES:

SPALL AREAS: 0.3 SF + 0.3 SF = 0.6 SF

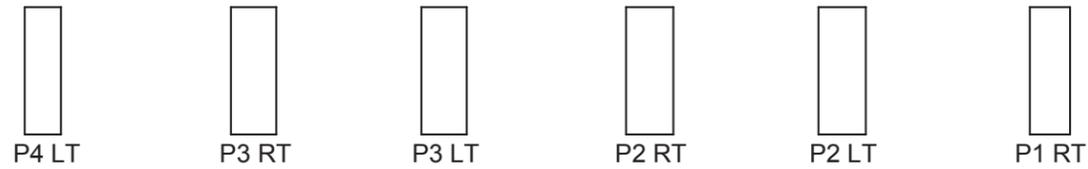
HOLLOW AREAS: 4.0 SF + 6.3 SF + 2.0 SF + 38.1 SF + 6.5 SF + 16.0 SF + 1.5 SF = 74.3 SF

MAP CRACKED AREAS: 0.0 SF

TOTAL LENGTH OF CRACKS: 2.0 LF

PIER 8 DETERIORATION SKETCH

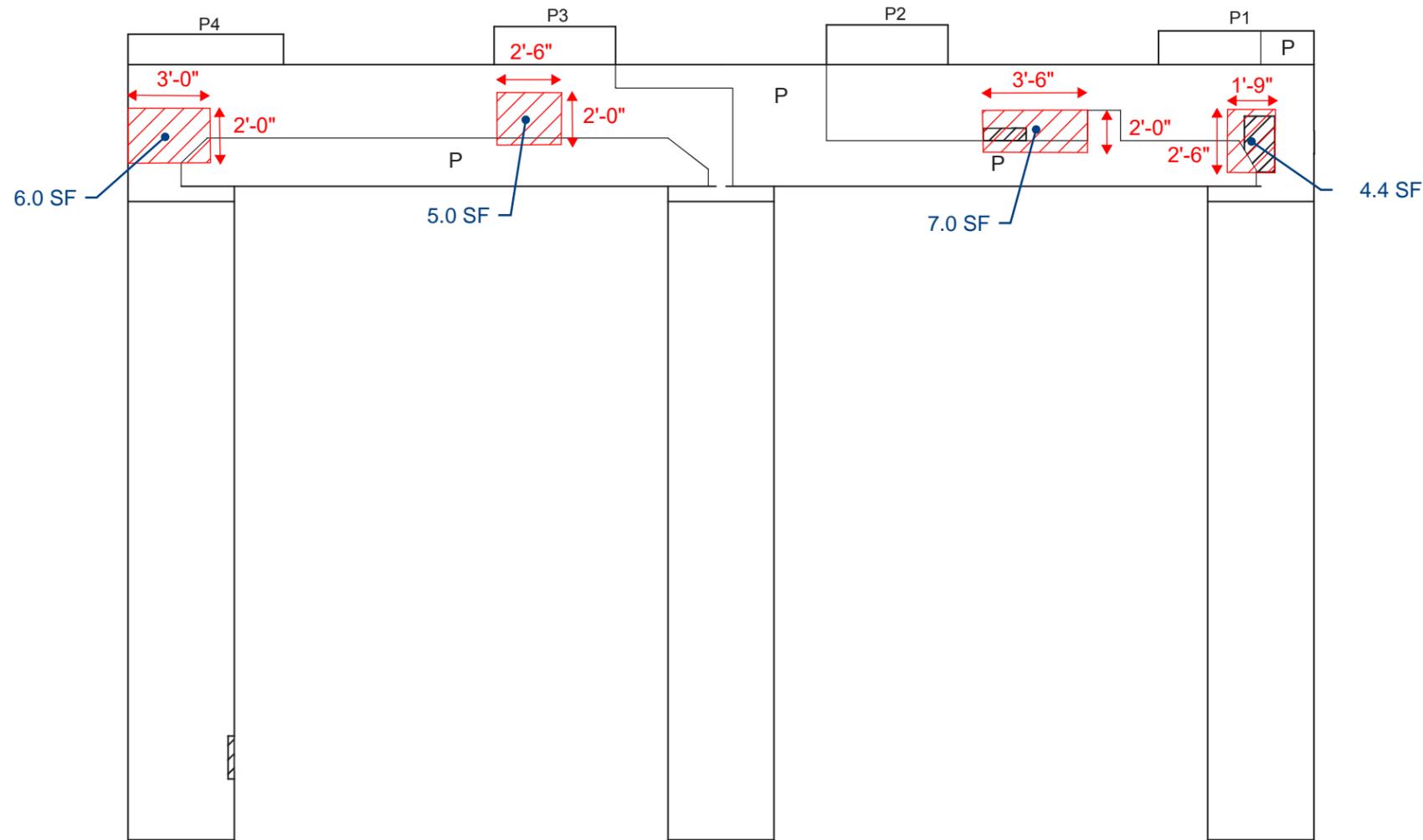
NOT TO SCALE



* NOTE: P1 LT & P4 RT VIEWS LOCATED ON
 PIER 8 DETERIORATION SKETCH
 SHEET 1 OF 2



PLAN



PIER 8 END ELEVATION

LEGEND

- Hollow Concrete (HC)
- Spall (S)
- Patch/Repair
- Crack (C)
- Hairline Crack (HL)
- Core Location

LEGEND

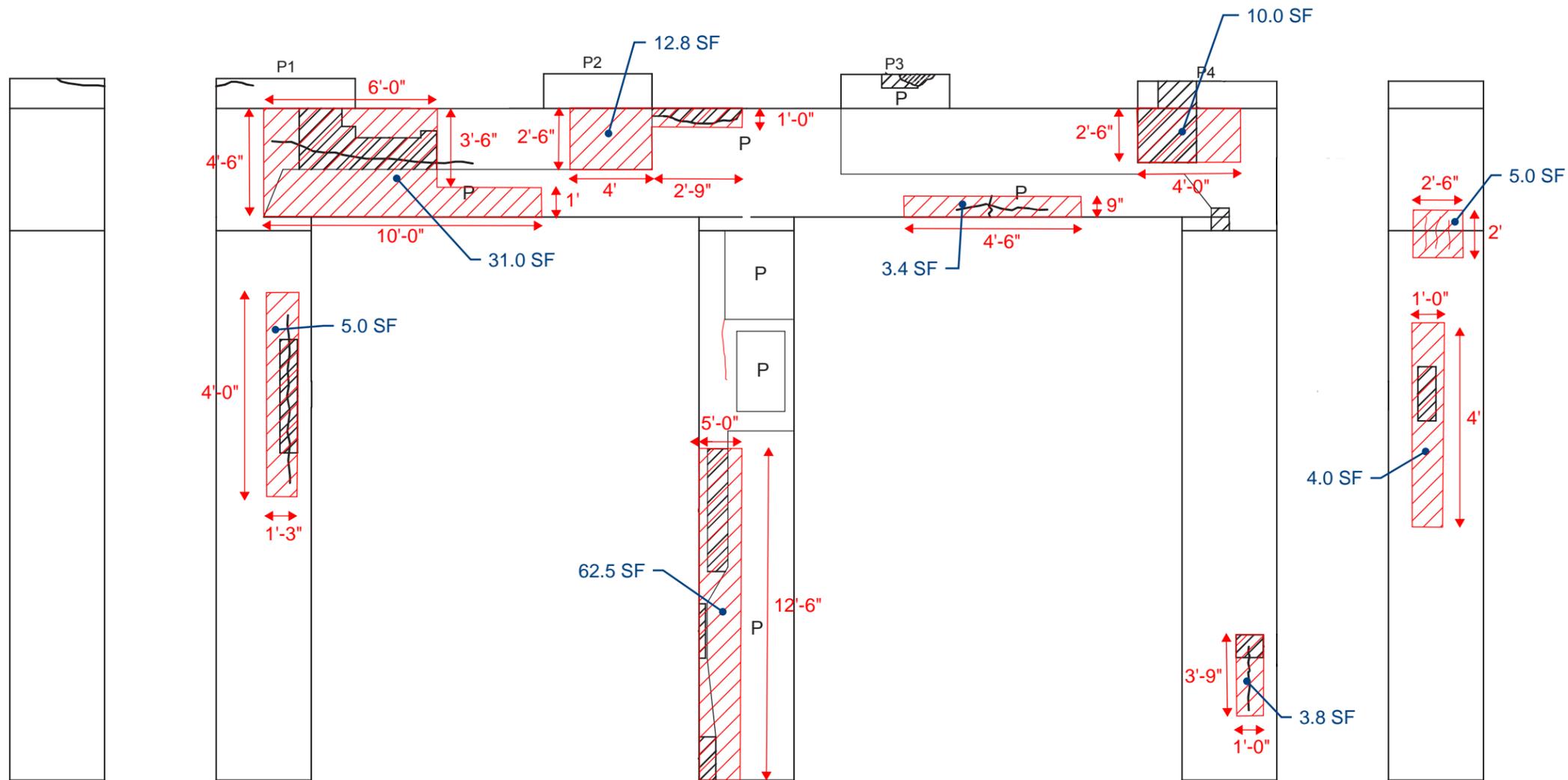
- Hollow Concrete (HC)
- Spall (S)
- Patch
- Crack (C)
- Hairline Crack (HL)

QUANTITIES:

SPALL AREAS: 0.0 SF
 HOLLOW AREAS: 6.0 SF + 5.0 SF + 7.0 SF + 4.4 SF = 22.4 SF
 MAP CRACKED AREAS: 0.0 SF
 TOTAL LENGTH OF CRACKS: 0.0 LF

PIER 9 DETERIORATION SKETCH

NOT TO SCALE



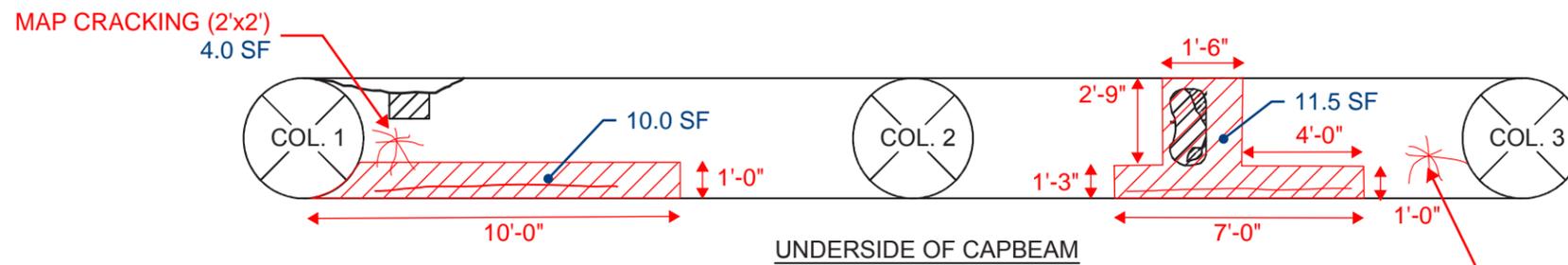
COL. 1 LT

PIER 9 BEGIN ELEVATION

COL. 3 RT

LEGEND

- Hollow Concrete (HC)
- Spall (S)
- Patch/Repair
- Crack (C)
- Hairline Crack (HL)
- Core Location



UNDERSIDE OF CAPBEAM

MAP CRACKING (2'x2')
4.0 SF

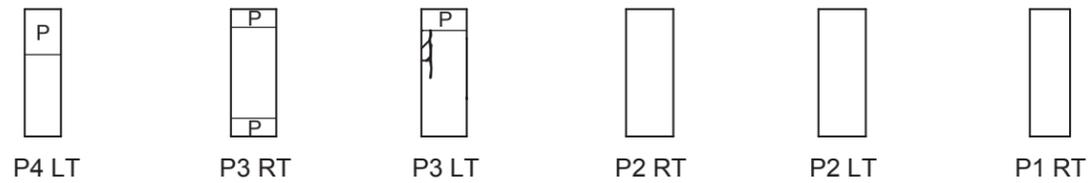
LEGEND

- Hollow Concrete (HC)
- Spall (S)
- Patch
- Crack (C)
- Hairline Crack (HL)

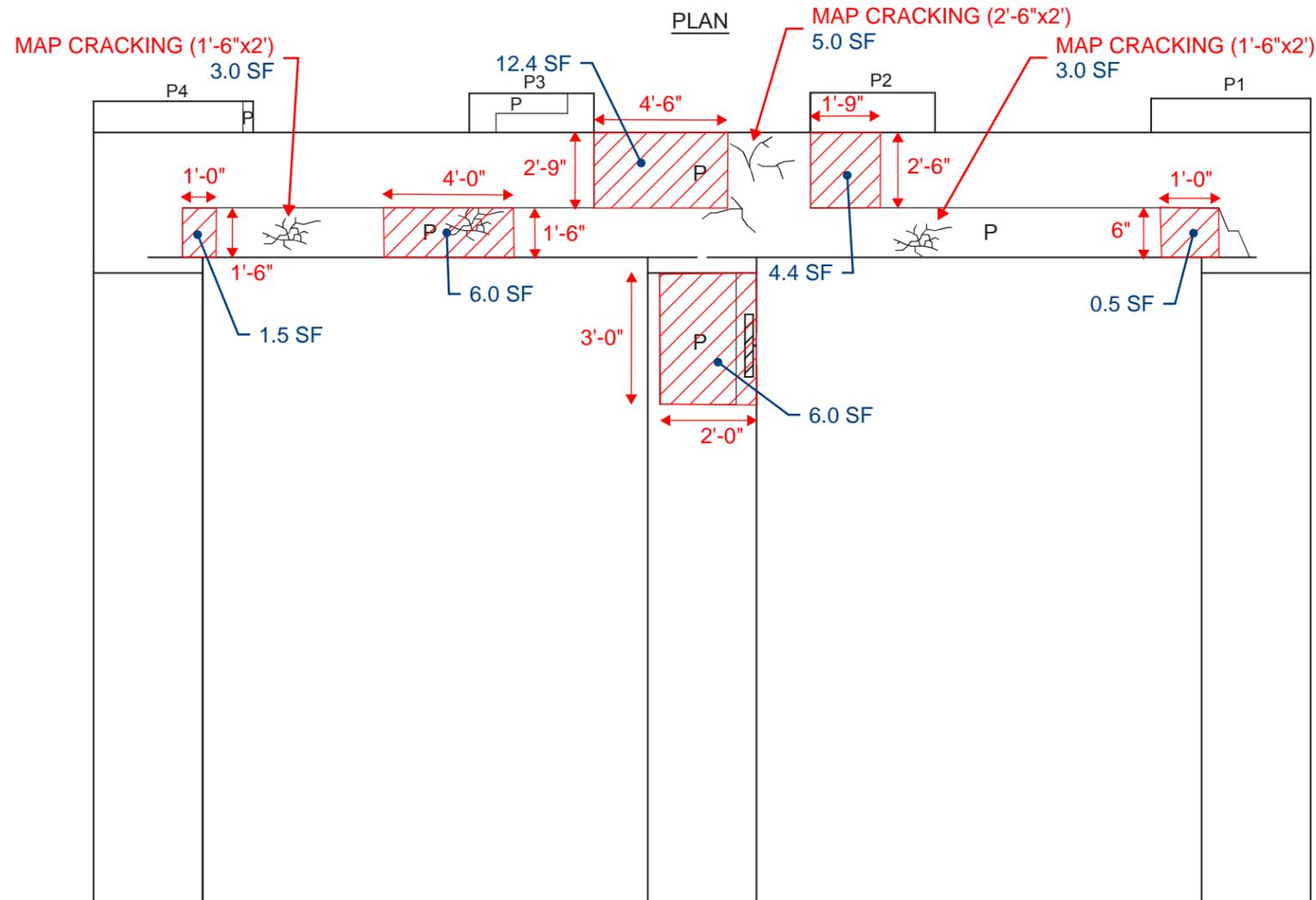
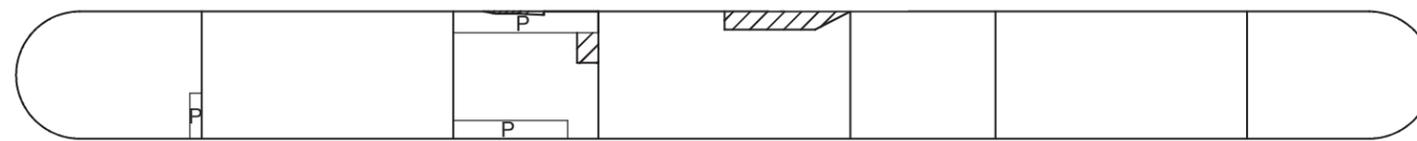
QUANTITIES:

SPALL AREAS: 0.0 SF
 HOLLOW AREAS: 5.0 SF + 31.0 SF + 12.8 SF + 62.5 SF + 3.4 SF + 10.0 SF + 3.8 SF + 4.0 SF
 + 5.0 SF + 10.0 SF + 11.5 SF = 158.9 SF
 MAP CRACKED AREAS: 4.0 SF + 4.0 SF = 8.0 SF
 TOTAL LENGTH OF CRACKS: 6.0 LF

NOT TO SCALE



* NOTE: P1 LT & P4 RT VIEWS LOCATED ON PIER 9 DETERIORATION SKETCH SHEET 1 OF 2



LEGEND

- Hollow Concrete (HC)
- Spall (S)
- Patch/Repair
- Crack (C)
- Hairline Crack (HL)
- Core Location

LEGEND

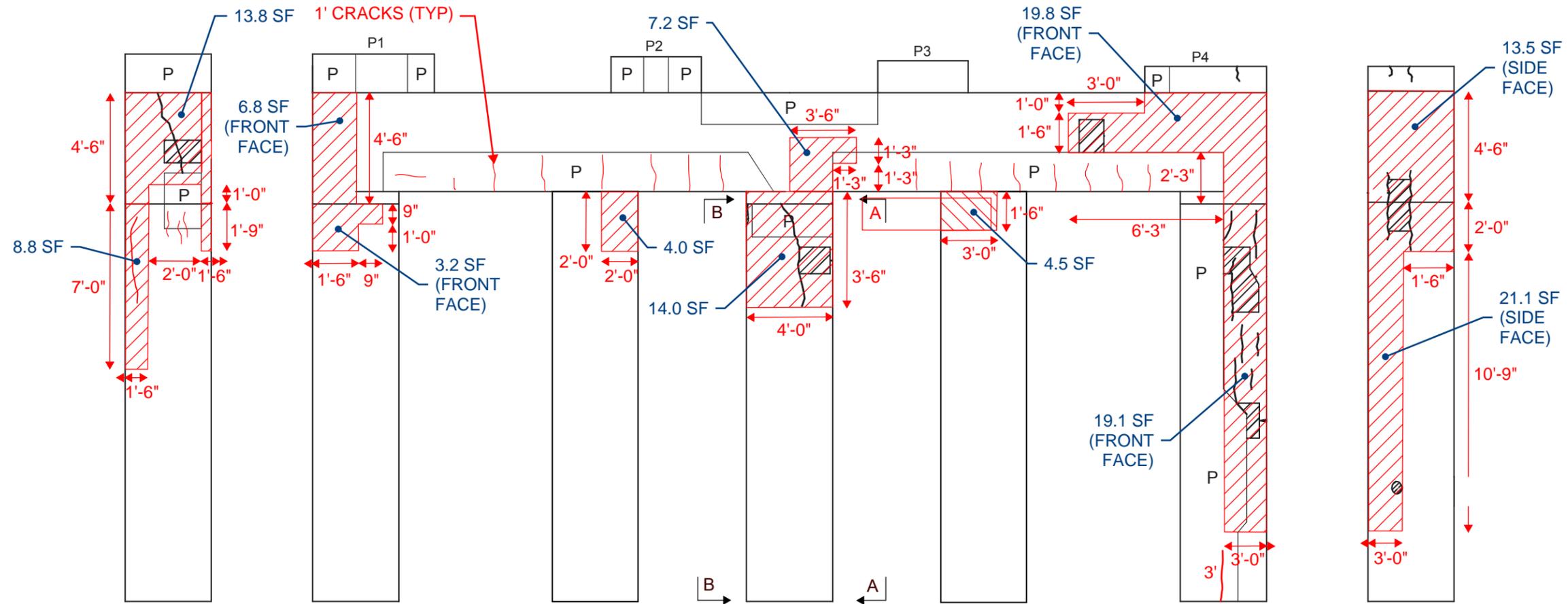
- Hollow Concrete (HC)
- Spall (S)
- Patch
- Crack (C)
- Hairline Crack (HL)

QUANTITIES:

SPALL AREAS: 0.0 SF
 HOLLOW AREAS: 1.5 SF + 6.0 SF + 12.4 SF + 6.0 SF + 4.4 SF + 0.5 SF = 30.8 SF
 MAP CRACKED AREAS: 3.0 SF + 3.0 SF + 5.0 SF = 11.0 SF
 TOTAL LENGTH OF CRACKS: 0.0 LF

PIER 9 END ELEVATION

NOT TO SCALE



COL. 1 LT

SECTION B-B

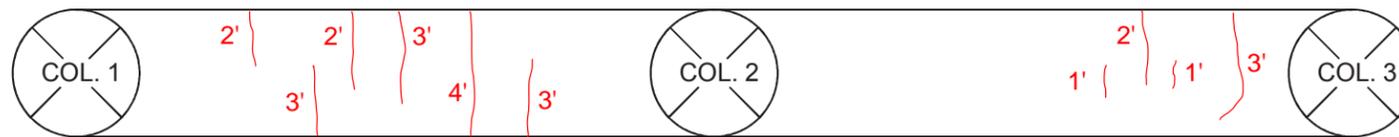
SECTION A-A

COL. 3 RT

PIER 10 BEGIN ELEVATION

LEGEND

- Hollow Concrete (HC)
- Spall (S)
- Patch/Repair
- Crack (C)
- Hairline Crack (HL)
- Core Location



UNDERSIDE OF CAPBEAM

LEGEND

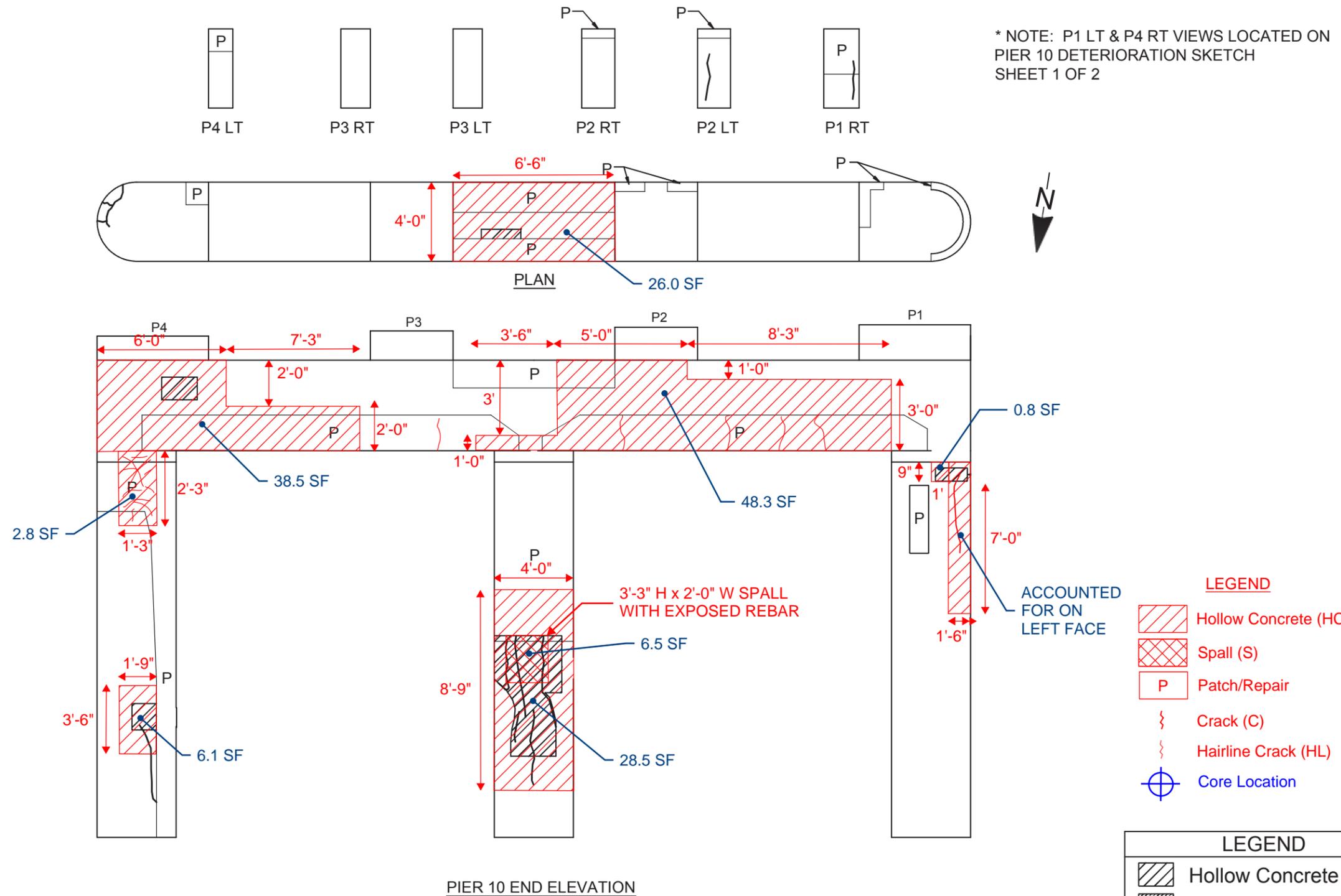
- Hollow Concrete (HC)
- Spall (S)
- Patch
- Crack (C)
- Hairline Crack (HL)

QUANTITIES:

SPALL AREAS: 0.0 SF
 HOLLOW AREAS: = 8.8 SF + 13.8 SF + 3.2 SF + 6.8 SF + 4.0 SF + 14.0 SF + 7.2 SF + 4.5 SF + 19.1 SF + 19.8 SF + 21.1 SF + 13.5 SF = 135.8 SF
 MAP CRACKED AREAS: 0.0 SF
 TOTAL LENGTH OF CRACKS: 45.0 LF

PIER 10 DETERIORATION SKETCH

NOT TO SCALE

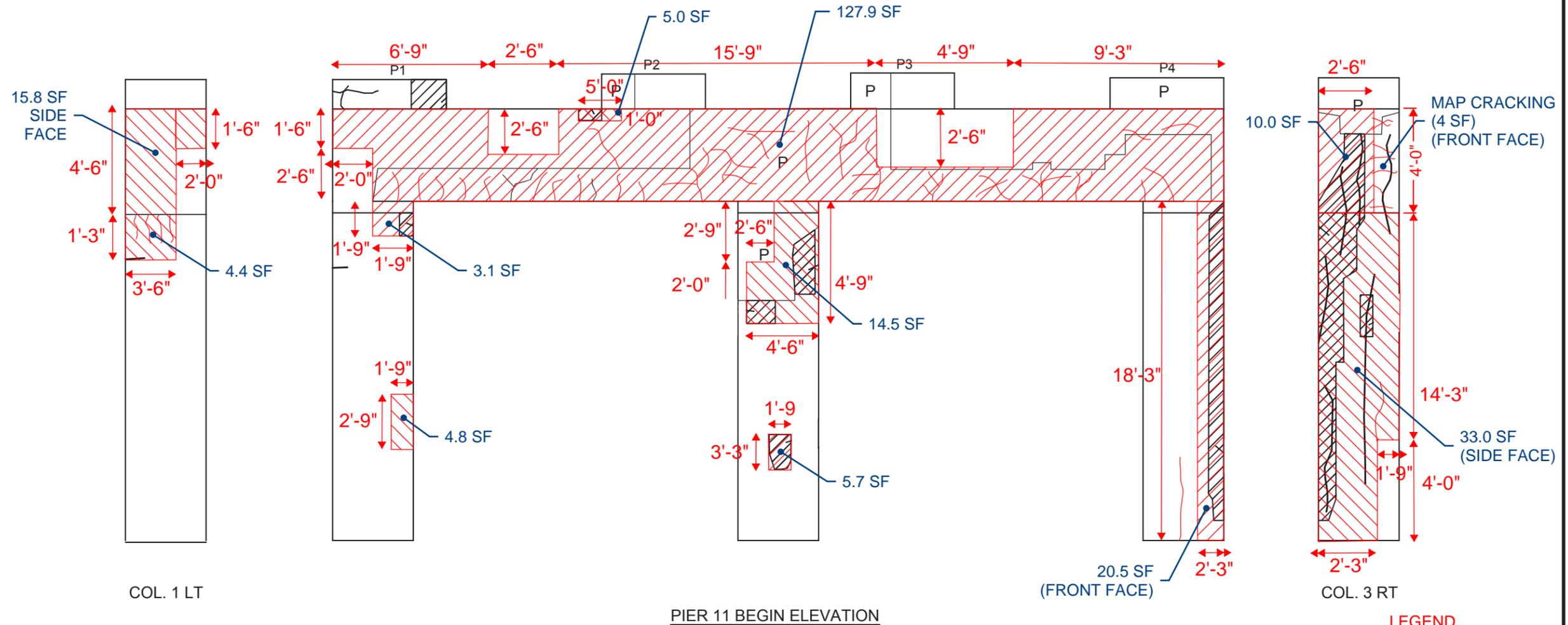


LEGEND	
	Hollow Concrete (HC)
	Spall (S)
	Patch/Repair
	Crack (C)
	Hairline Crack (HL)
	Core Location

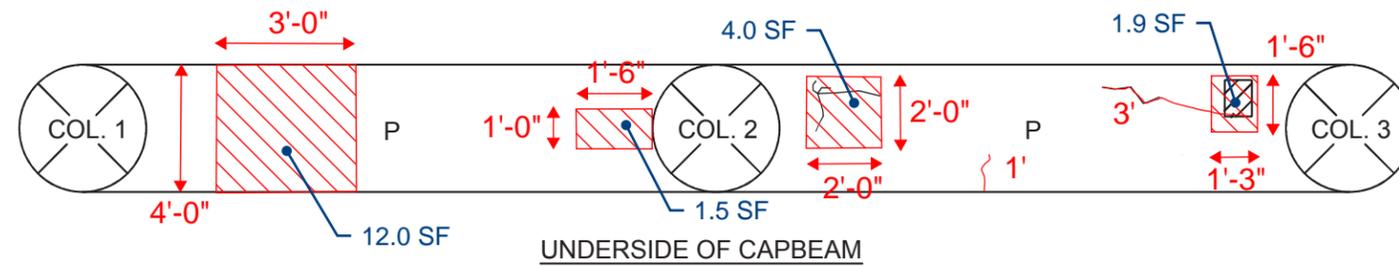
LEGEND	
	Hollow Concrete (HC)
	Spall (S)
	Patch
	Crack (C)
	Hairline Crack (HL)

PIER 11 DETERIORATION SKETCH

NOT TO SCALE



- LEGEND**
- Hollow Concrete (HC)
 - Spall (S)
 - Patch/Repair
 - Crack (C)
 - Hairline Crack (HL)
 - Core Location



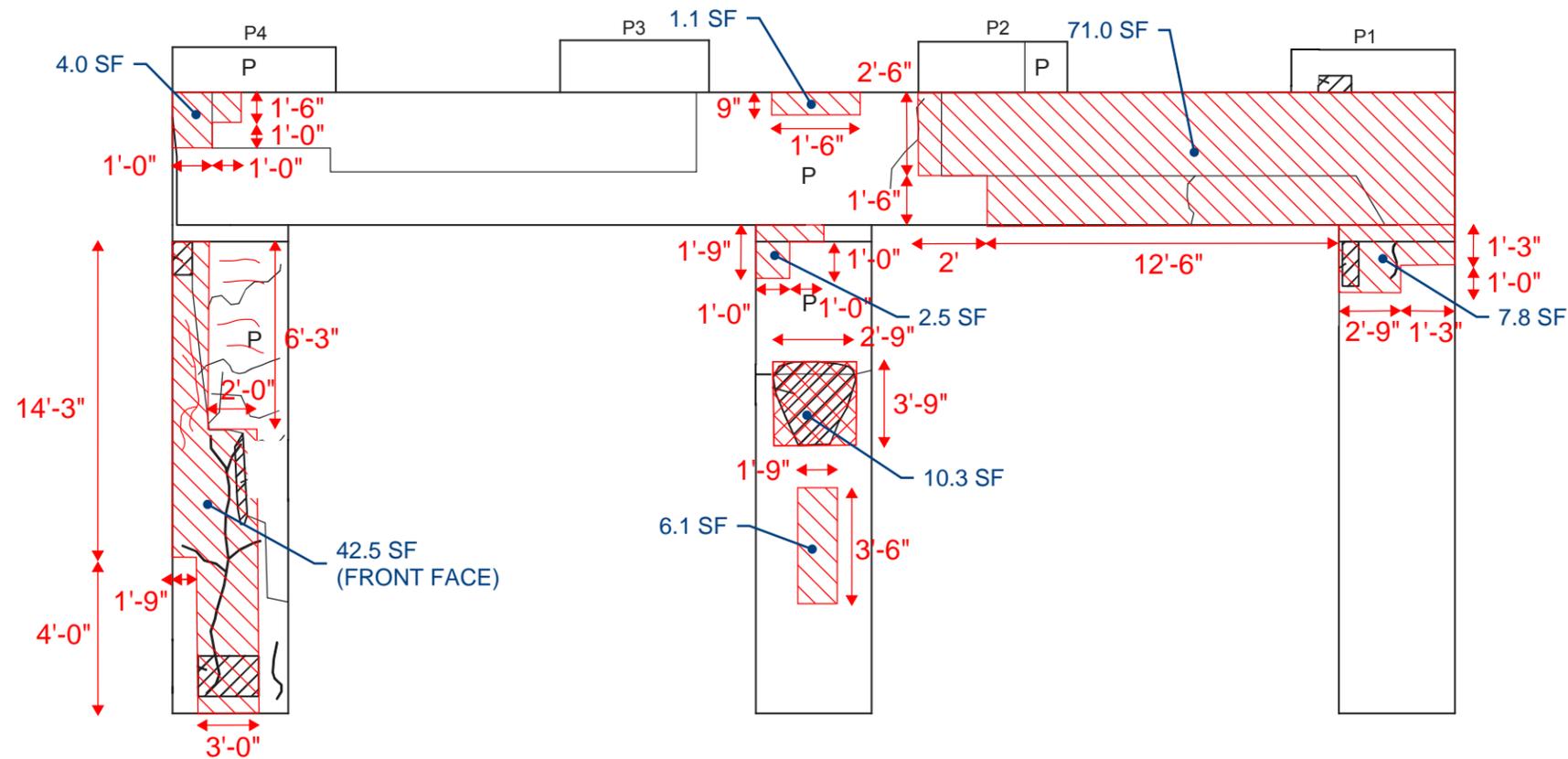
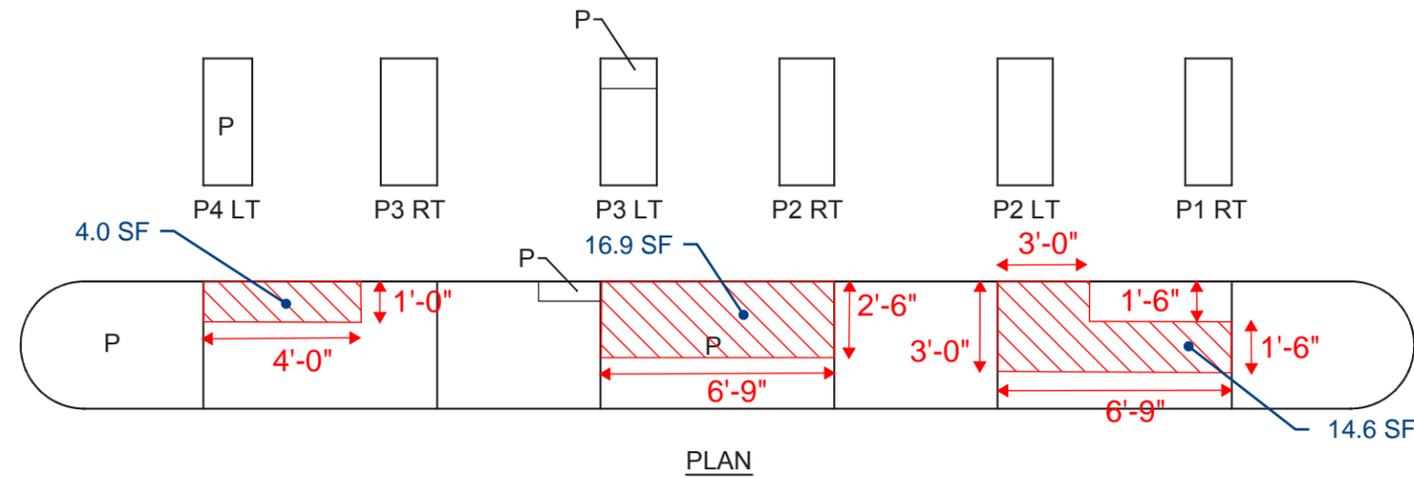
- LEGEND**
- Hollow Concrete (HC)
 - Spall (S)
 - Patch
 - Crack (C)
 - Hairline Crack (HL)

QUANTITIES:
 SPALL AREAS: 5.0 SF
 HOLLOW AREAS: 4.4 SF + 15.8 SF + 4.8 SF + 3.1 SF + 5.7 SF + 14.5 SF + 20.5 SF + 33.0 SF
 + 127.9 SF + 10.0 SF + 12.0 SF + 1.5 SF + 4.0 SF + 1.9 SF = 259.1 SF
 MAP CRACKED AREAS: 4.0 SF
 TOTAL LENGTH OF CRACKS: 5.0 LF

PIER 11 DETERIORATION SKETCH

NOT TO SCALE

* NOTE: P1 LT & P4 RT VIEWS LOCATED ON
 PIER 11 DETERIORATION SKETCH
 SHEET 1 OF 2



LEGEND

	Hollow Concrete (HC)
	Spall (S)
	Patch/Repair
	Crack (C)
	Hairline Crack (HL)
	Core Location

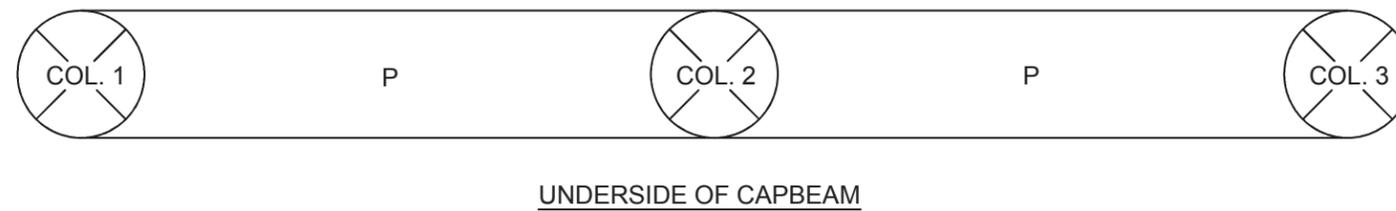
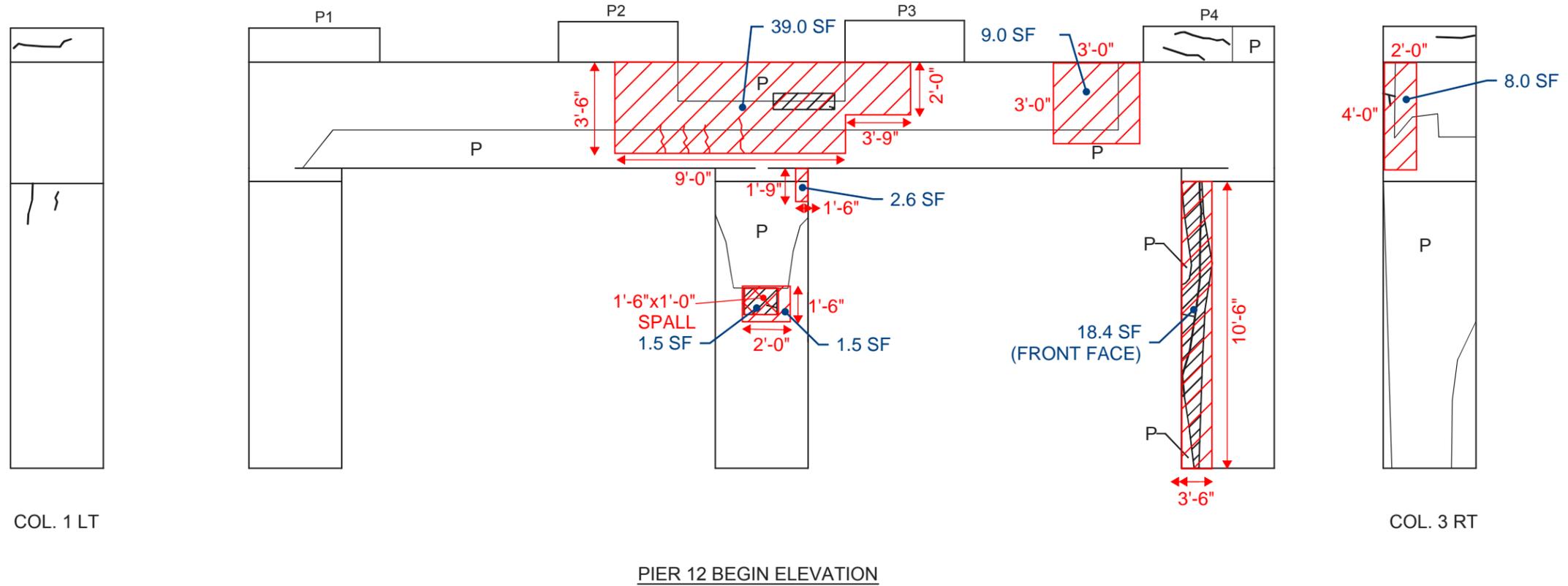
LEGEND

	Hollow Concrete (HC)
	Spall (S)
	Patch
	Crack (C)
	Hairline Crack (HL)

QUANTITIES:
 SPALL AREAS: 10.3 SF
 HOLLOW AREAS: 42.5 SF + 4.0 SF + 6.1 SF + 2.5 SF + 7.8 SF + 1.1 SF + 71.0 SF + 4.0 SF +
 16.9 SF + 14.6 SF = 170.5 SF
 MAP CRACKED AREAS: 0.0 SF
 TOTAL LENGTH OF CRACKS: 18.0 LF

PIER 12 DETERIORATION SKETCH

NOT TO SCALE



LEGEND

- Hollow Concrete (HC)
- Spall (S)
- Patch/Repair
- Crack (C)
- Hairline Crack (HL)
- Core Location

LEGEND

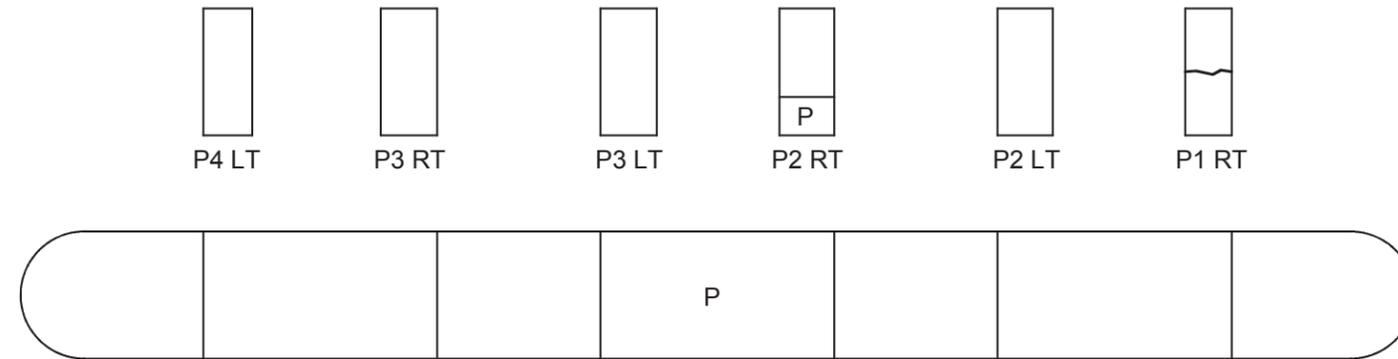
- Hollow Concrete (HC)
- Spall (S)
- Patch
- Crack (C)
- Hairline Crack (HL)

QUANTITIES:

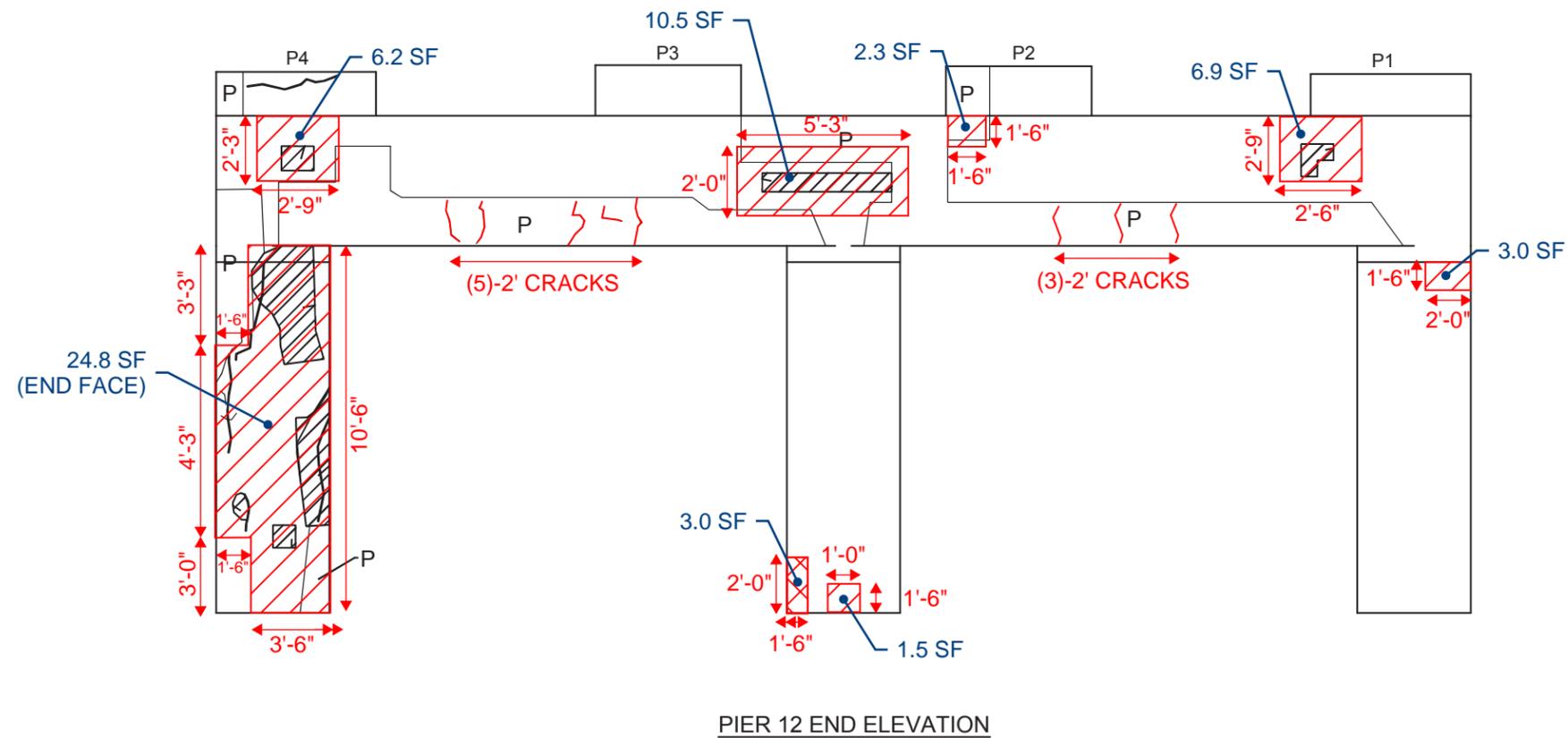
SPALL AREAS: 1.5 SF
 HOLLOW AREAS: 1.5 SF + 2.6 SF + 18.4 SF + 39.0 SF - 9.0 SF + 8.0 SF = 78.5 SF
 MAP CRACKED AREAS: 0.0 SF
 TOTAL LENGTH OF CRACKS: 5.0 LF

NOT TO SCALE

* NOTE: P1 LT & P4 RT VIEWS LOCATED ON
 PIER 12 DETERIORATION SKETCH
 SHEET 1 OF 2



PLAN



PIER 12 END ELEVATION

LEGEND

- Hollow Concrete (HC)
- Spall (S)
- Patch/Repair
- Crack (C)
- Hairline Crack (HL)
- Core Location

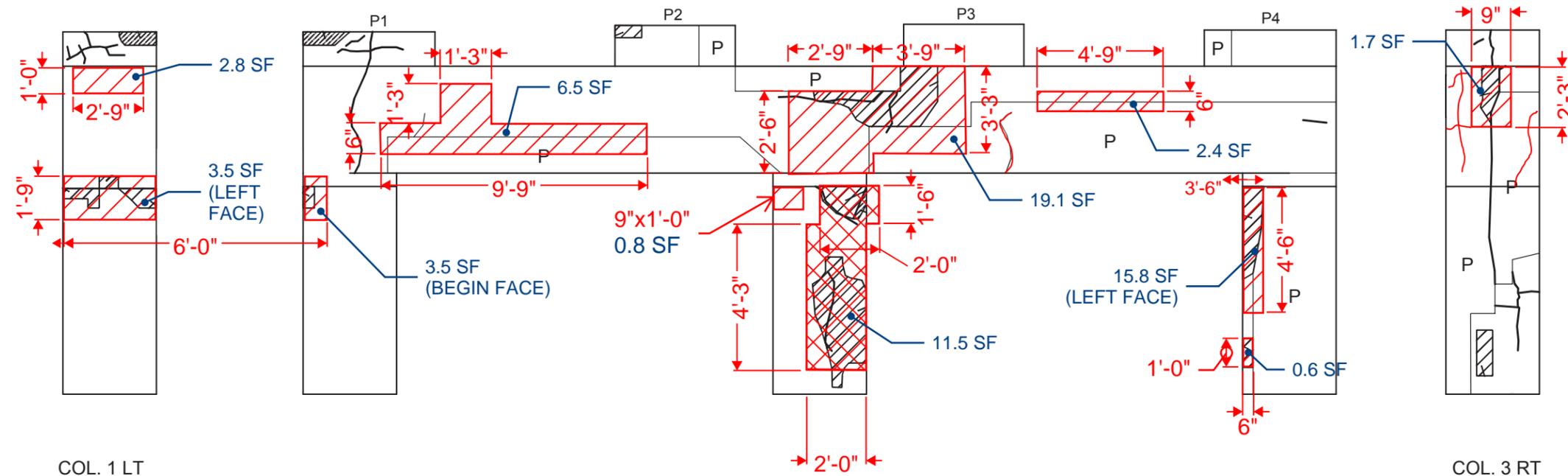
LEGEND

- Hollow Concrete (HC)
- Spall (S)
- Patch
- Crack (C)
- Hairline Crack (HL)

QUANTITIES:

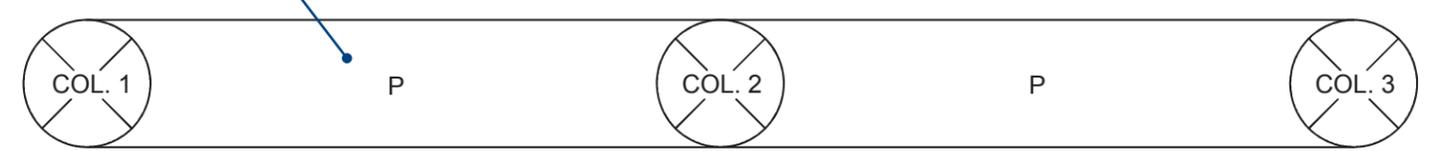
SPALL AREAS: 3.0 SF
 HOLLOW AREAS: 24.8 SF + 3.0 SF + 1.5 SF + 6.2 SF + 10.5 SF + 2.3 SF + 6.9 SF = 55.1 SF
 MAP CRACKED AREAS: 0.0 SF
 TOTAL LENGTH OF CRACKS: 16.0 LF

NOT TO SCALE



APPROXIMATELY 25 LF OF FINE CRACKING (NOT INCLUDED IN REPAIR AREAS)

5 LF OF FINE CRACKING



UNDERSIDE OF CAPBEAM

LEGEND

- Hollow Concrete (HC)
- Spall (S)
- Patch/Repair
- Crack (C)
- Hairline Crack (HL)
- Core Location

LEGEND

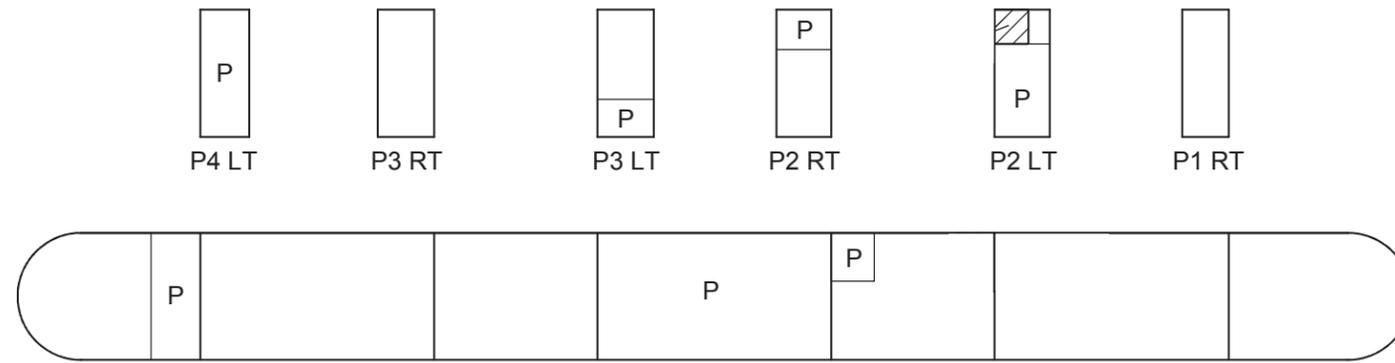
- Hollow Concrete (HC)
- Spall (S)
- Patch
- Crack (C)
- Hairline Crack (HL)

QUANTITIES:

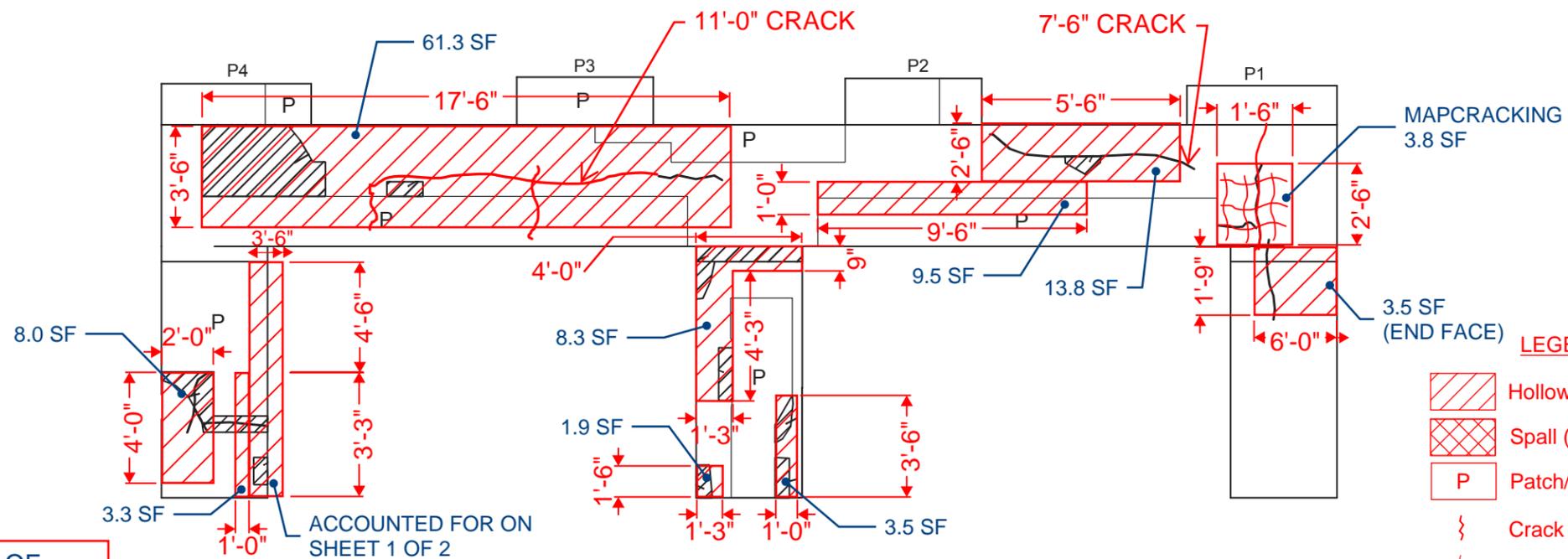
SPALL AREAS: 11.5 SF
 HOLLOW AREAS: 2.8 SF + 3.5 SF + 3.5 SF + 6.5 SF + 19.1 SF + 0.8 SF + 2.4 SF + 15.8 SF + 0.6 SF + 1.7 SF = 56.7 SF
 MAP CRACKED AREAS: 0 SF
 TOTAL LENGTH OF CRACKS: 30.0 LF

NOT TO SCALE

* NOTE: P1 LT & P4 RT VIEWS LOCATED ON
 PIER 13 DETERIORATION SKETCH
 SHEET 1 OF 2



PLAN



PIER 13 END ELEVATION

LEGEND

- Hollow Concrete (HC)
- Spall (S)
- Patch/Repair
- Crack (C)
- Hairline Crack (HL)
- Core Location

LEGEND

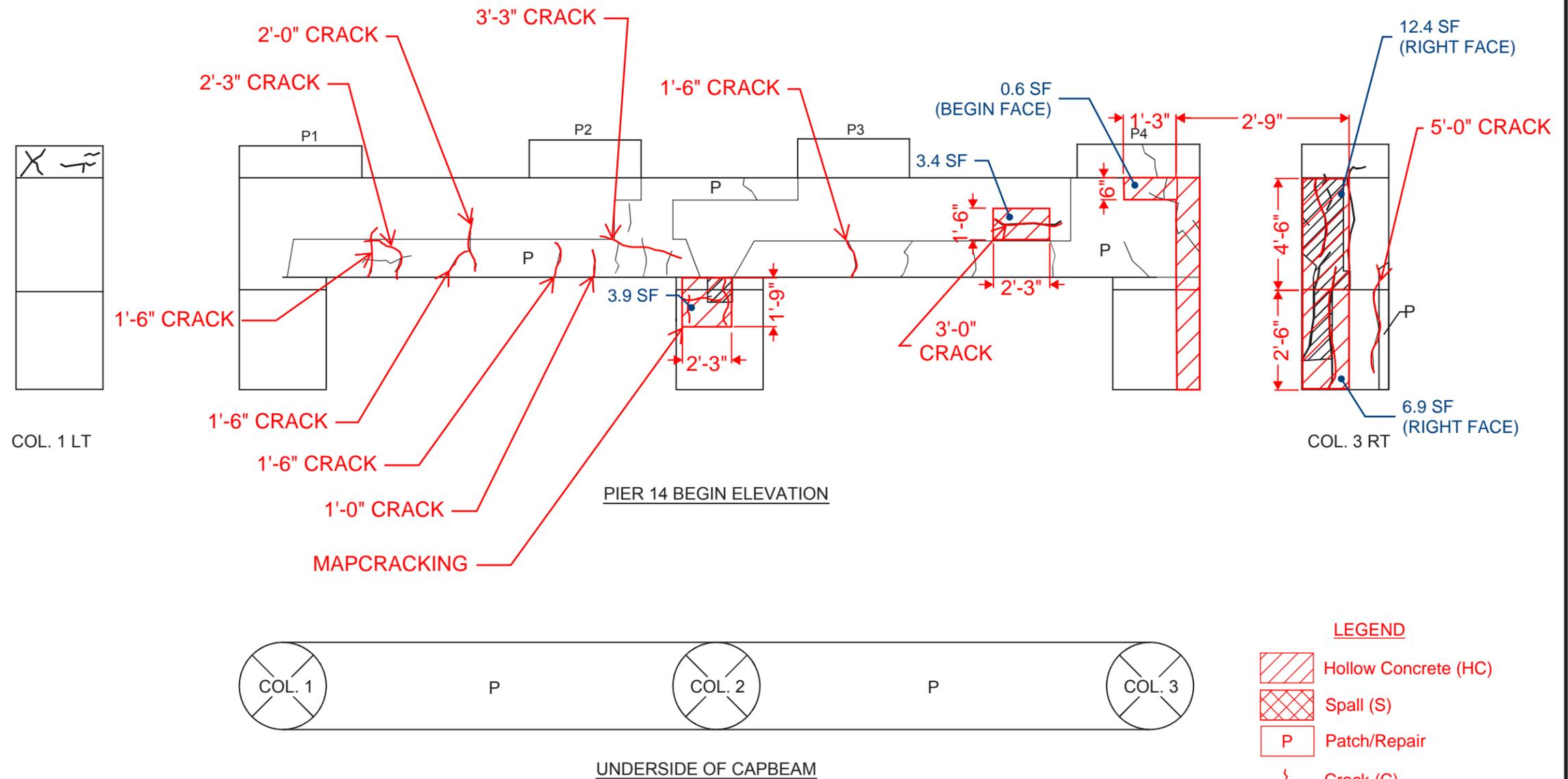
- Hollow Concrete (HC)
- Spall (S)
- Patch
- Crack (C)
- Hairline Crack (HL)

APPROXIMATELY 15 LF OF
 FINE CRACKING (NOT
 INCLUDED IN REPAIR AREAS)

QUANTITIES:

SPALL AREAS: **0.0 SF**
 HOLLOW AREAS: 8.0 SF + 3.3 SF + 61.3 SF + 8.3 SF + 1.9 SF + 3.5 SF + 9.5 SF + 13.8 SF + 3.5 SF = **113.1 SF**
 MAP CRACKED AREAS: **3.8 SF**
 TOTAL LENGTH OF CRACKS: **15.0 LF**

NOT TO SCALE



LEGEND

- Hollow Concrete (HC)
- Spall (S)
- Patch/Repair
- Crack (C)
- Hairline Crack (HL)
- Core Location

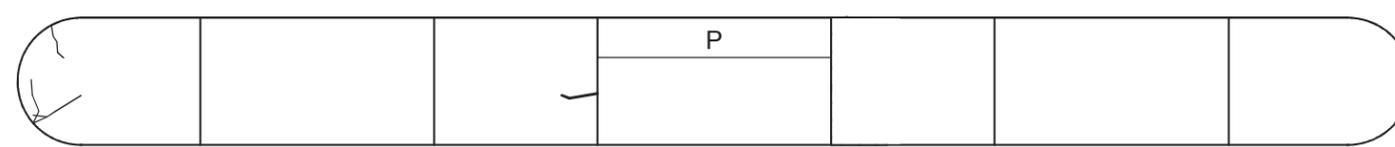
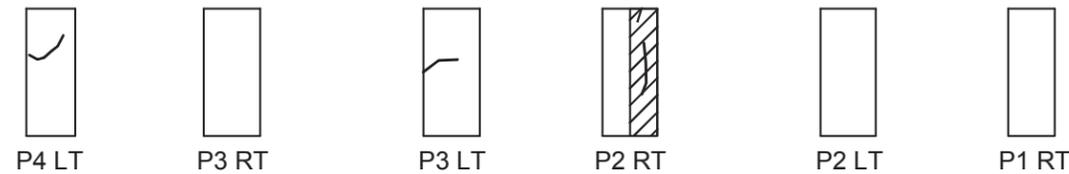
LEGEND

- Hollow Concrete (HC)
- Spall (S)
- Patch
- Crack (C)
- Hairline Crack (HL)

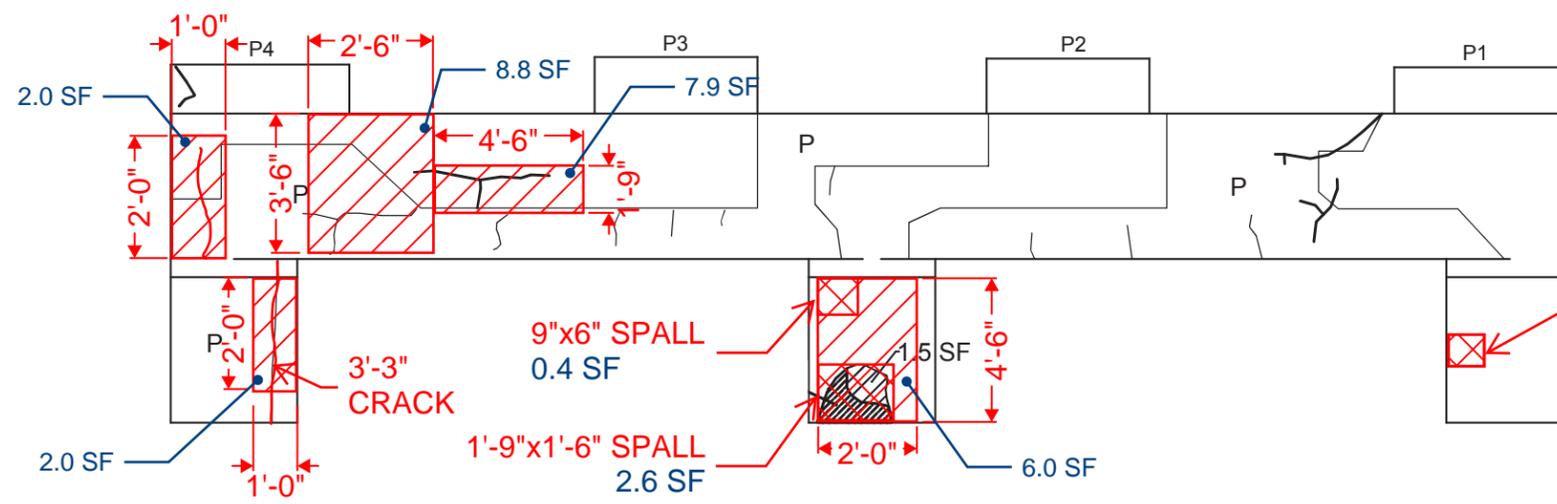
QUANTITIES:
 SPALL AREAS: **0.0 SF**
 HOLLOW AREAS: 3.9 SF + 3.4 SF + 0.6 SF + 12.4 SF + 6.9 SF = **27.2 SF**
 MAP CRACKED AREAS: **0 SF**
 TOTAL LENGTH OF CRACKS: **60 LF**

NOT TO SCALE

* NOTE: P1 LT & P4 RT VIEWS LOCATED ON
 PIER 14 DETERIORATION SKETCH
 SHEET 1 OF 2



PLAN



PIER 14 END ELEVATION

LEGEND

- Hollow Concrete (HC)
- Spall (S)
- Patch/Repair
- Crack (C)
- Hairline Crack (HL)
- Core Location

LEGEND

- Hollow Concrete (HC)
- Spall (S)
- Patch
- Crack (C)
- Hairline Crack (HL)

APPROXIMATELY 45'
 OF END FACE CRACKS

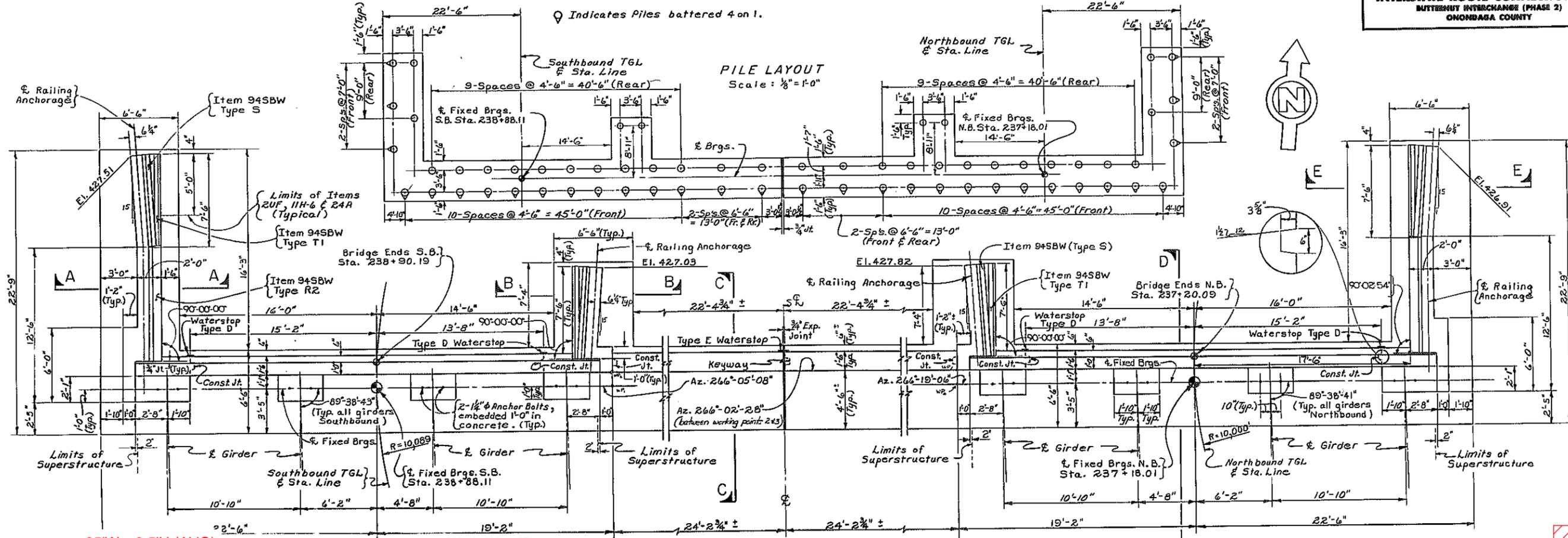
QUANTITIES:

SPALL AREAS: 0.4 SF + 2.6 SF + 0.3 = **3.3 SF**
 HOLLOW AREAS: 2.0 SF + 2.0 SF + 8.8 SF + 7.9 SF + 6.0 SF = **26.7 SF**
 MAP CRACKED AREAS: **0 SF**
 TOTAL LENGTH OF CRACKS: **60 LF**

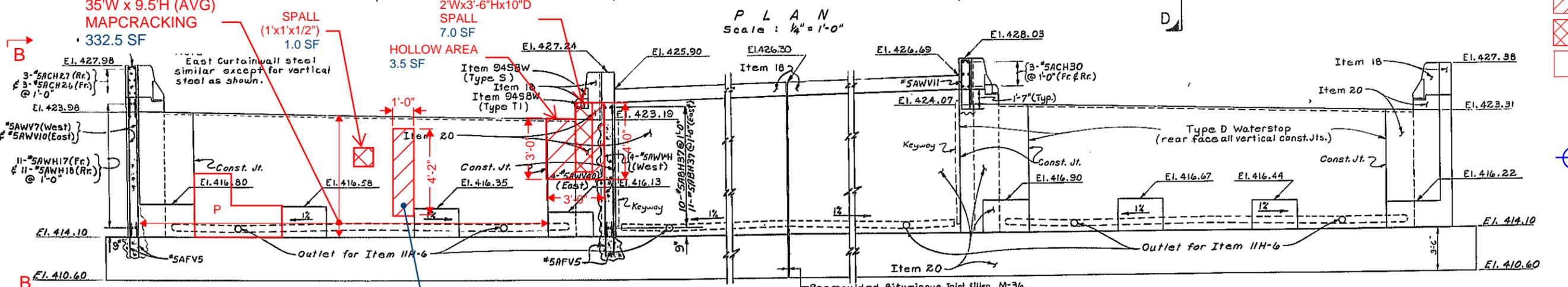
FED. RD. REG. NO.	STATE	FEDERAL AID PROJECT NO.	SHEET NO.	TOTAL SHEETS
1	NEW YORK	I-690-3(28) I-481-2(116)	185	309

INTERSTATE ROUTE CONNECTION 570
 BUTTERNUT INTERCHANGE (PHASE 2)
 ONONDAGA COUNTY

Piles to be Cast-in-Place Concrete Piles, Item 85C
 Estimated length of pile = 65 feet
 For design purposes the pile load does not exceed 35 tons per pile.



PLAN
 Scale: 1/4" = 1'-0"



ELEVATION
 Scale: 1/4" = 1'-0"

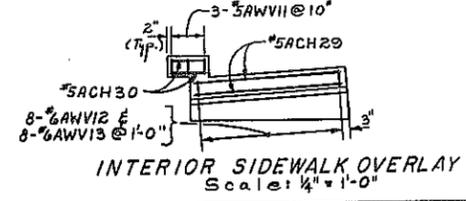
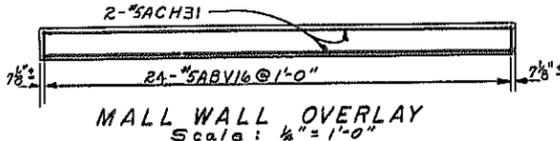
Note: Piles not shown in Elevation. Sink perforations in Item 11H-6 extension thru wall.

Note: Northwest overlays shown, Northeast similar by opposite hand.

SEE NEXT PAGE FOR ELEVATION B-B

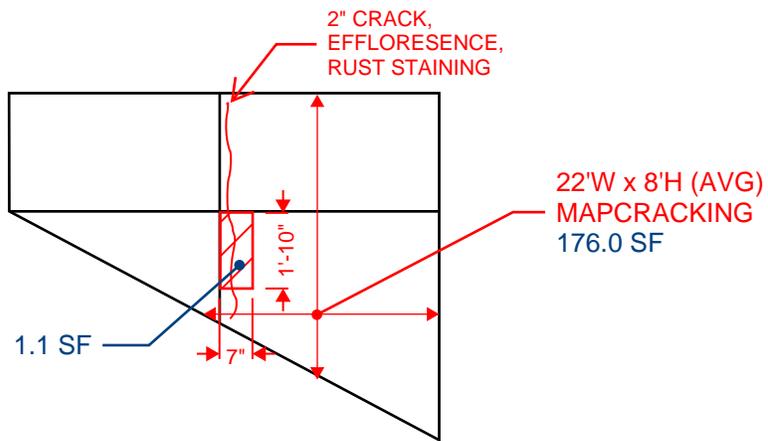
QUANTITIES:
 SPALL AREAS: 1.0 SF + 7.0 SF = 8.0 SF
 HOLLOW AREAS: 4.2 SF + 3.5 SF = 7.7 SF
 MAP CRACKED AREAS: 332.5 SF
 TOTAL LENGTH OF CRACKS: 0.0 LF

PROJECT ENGINEER: R. Parker
 IN CHARGE OF: F. Eckel
 DESIGNED BY: H. W. E. Row
 DESIGN CHECKED BY: J. B. Sherman
 DETAILED BY: J. C. Thompson
 DETAIL CHECKED BY: D. H. Smith



- LEGEND**
- Hollow Concrete (HC)
 - Spall (S)
 - Patch/Repair
 - Crack (C)
 - Hairline Crack (HL)
 - Core Location

BRIDGE NO. 2
 INTERSTATE ROUTE 481 OVER DEWITT YARDS
SOUTHBOUND BIN 1093571
 NORTH ABUTMENTS
 DRAWING NO. 7 OF 50



ELEVATION B-B
NORTHWEST
WINGWALL

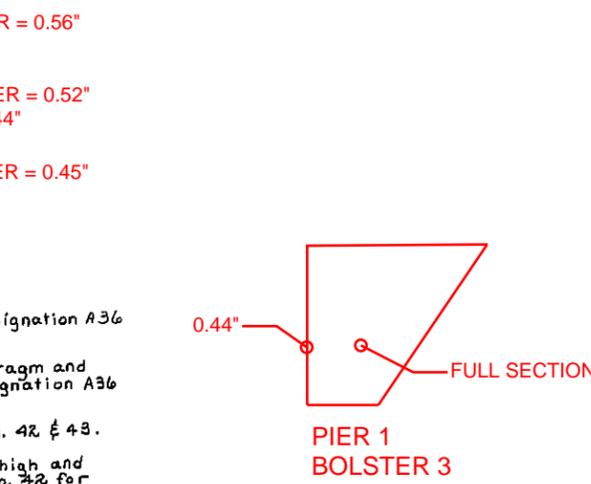
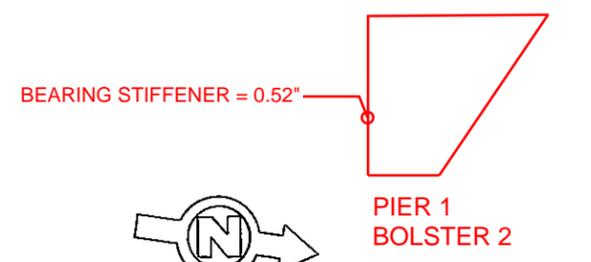
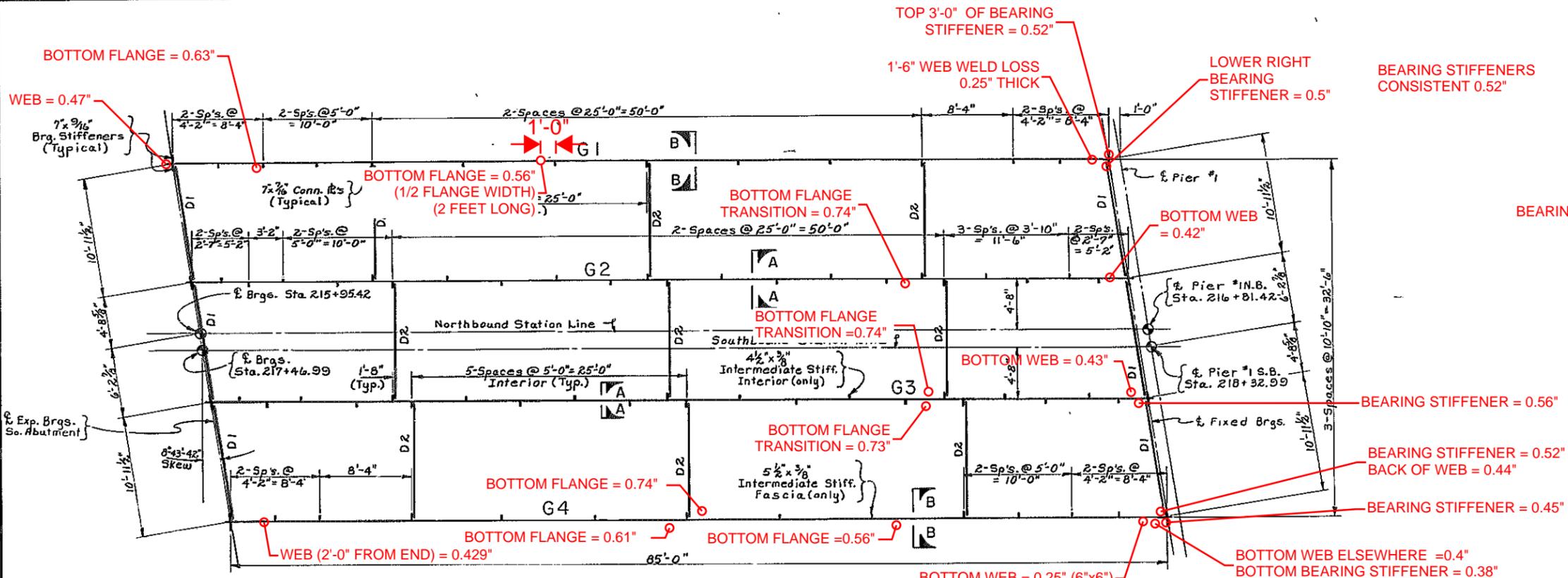
QUANTITIES:
 SPALL AREAS: **0.0 SF**
 HOLLOW AREAS: **1.1 SF**
 MAP CRACKED AREAS: **176.0 SF**
 TOTAL LENGTH OF CRACKS: **4.0 LF**

LEGEND

-  Hollow Concrete (HC)
-  Spall (S)
-  Patch/Repair
-  Crack (C)
-  Hairline Crack (HL)
-  Core Location

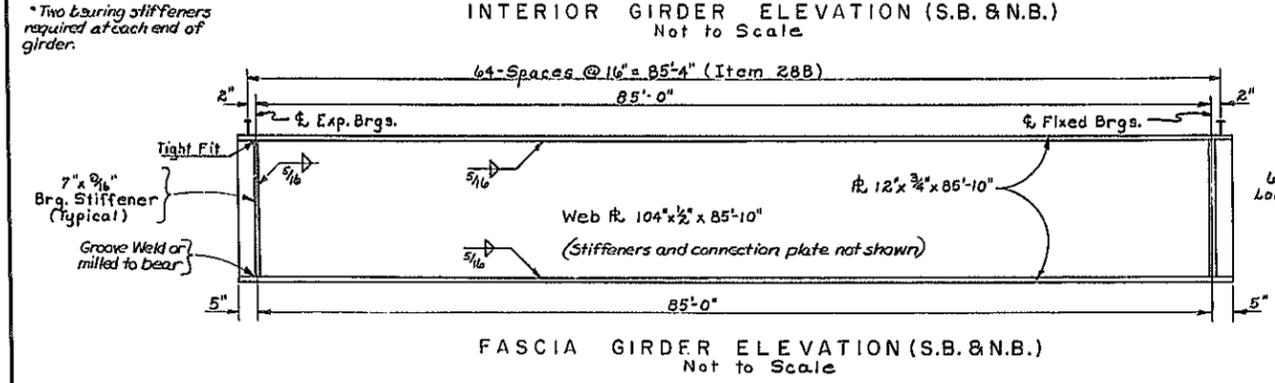
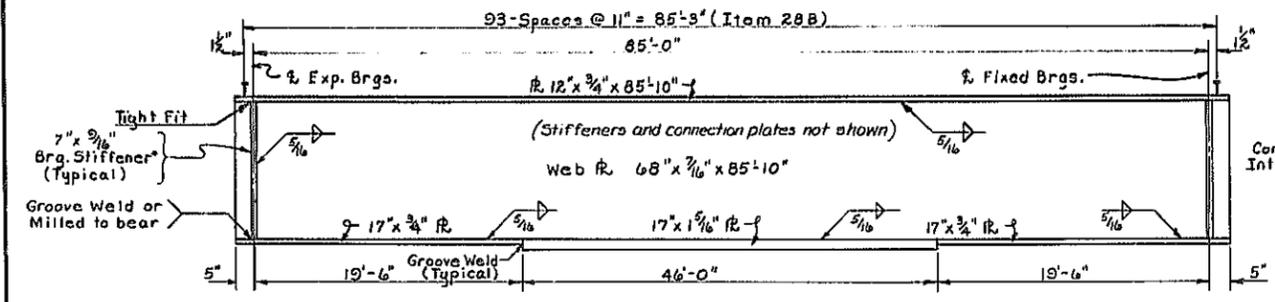
FED. RD. REG. NO.	STATE	FEDERAL AID PROJECT NO.	SHEET NO.	TOTAL SHEETS
1	NEW YORK	1-690-3(2B) 1-481-2(116)	196	309

INTERSTATE ROUTE CONNECTION 570
NUTCRACK INTERCHANGE (PHASE 2)
ONEIDA COUNTY

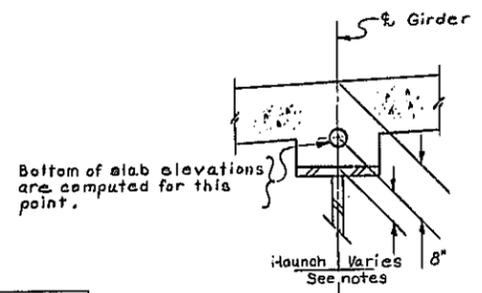


Notes:

- Web and flanges shall be ASTM designation A36 steel.
- Stiffeners, Connection Plates, Diaphragm and Gasket Plates shall be ASTM designation A36 steel.
- For Bearing Details see Drawing Nos. 42 & 43.
- Stud Shear Connectors shall be 6" high and placed in pairs. See Drawing No. 42 for additional details.
- For Height of Haunch see Table of Haunch Heights Drawing No. 34.



GIRDER	SPAN	THEO. BOTTOM OF SLAB ELEVATIONS			DEFLECTIONS (FT.)			CAMBER		
		% Exp. Brgs.	% Span	% Fix. Brgs.	STEEL	SLAB	S.D.L.	TOTAL	(ft.)	(in.)
G1 N.B.	1	446.55	447.12	447.65	.01	.03	.01	.05	.02	.07 1/8
G2 N.B.	1	446.80	447.40	447.90	.02	.08	.02	.12	.02	.14 1 1/16
G3 N.B.	1	446.79	447.36	447.89	.02	.08	.02	.12	.02	.14 1 1/16
G4 N.B.	1	446.59	447.16	447.65	.01	.03	.01	.05	.02	.07 1/8
G1 S.B.	1	446.34	446.92	447.46	.01	.03	.01	.05	.02	.07 1/8
G2 S.B.	1	446.59	447.17	447.71	.02	.08	.02	.12	.02	.14 1 1/16
G3 S.B.	1	446.64	447.22	447.76	.02	.08	.02	.12	.02	.14 1 1/16
G4 S.B.	1	446.44	447.02	447.55	.01	.03	.01	.05	.02	.07 1/8



PROJECT ENGINEER R. Parker
 IN CHARGE OF E. E. K. E. J.
 DESIGNED BY N. TOPPES
 DESIGN CHECKED BY R. THIMBLE
 DETAILED BY J. C. THOMPSON
 DETAIL CHECKED BY H. A. THOMPSON

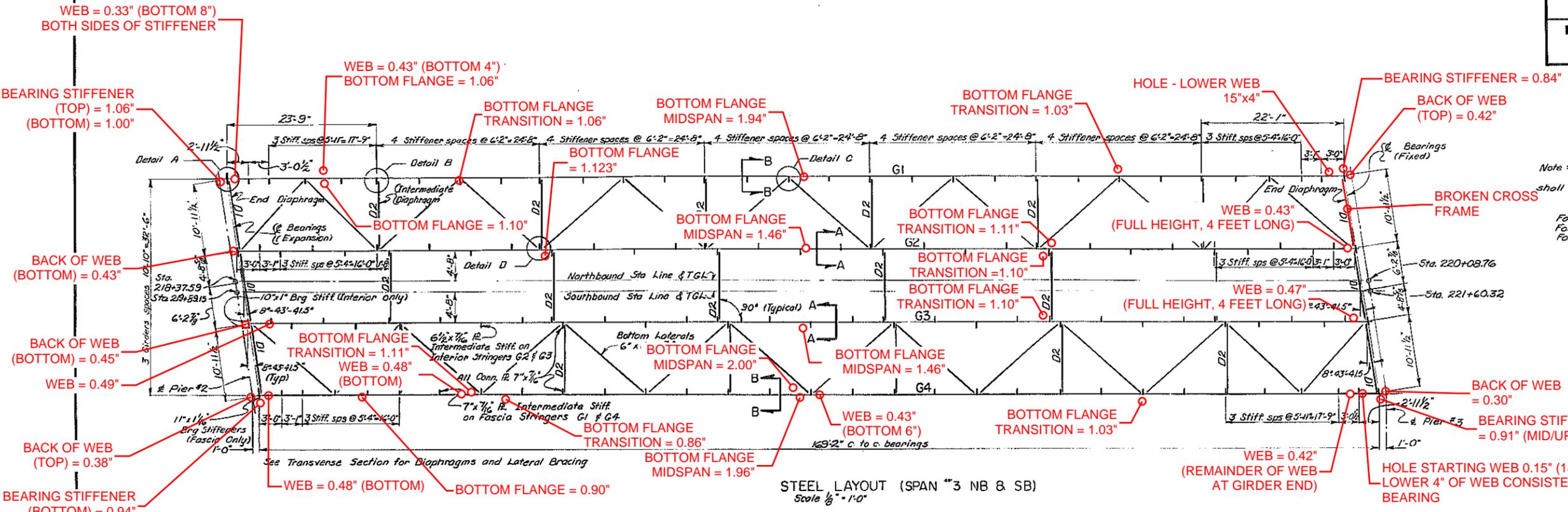
V.C.C. = Vertical Curve Correction
 S.D.L. = Superimposed Dead Load, includes weight of railing.

BRIDGE NO. 2
 INTERSTATE ROUTE 481 OVER DEWITT YARDS
 SPAN 1 SOUTHBOUND
 BIN 1093571

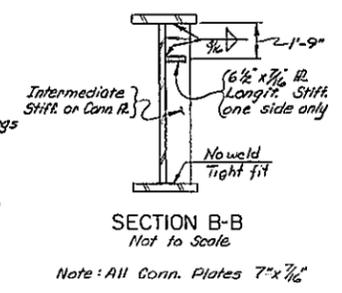
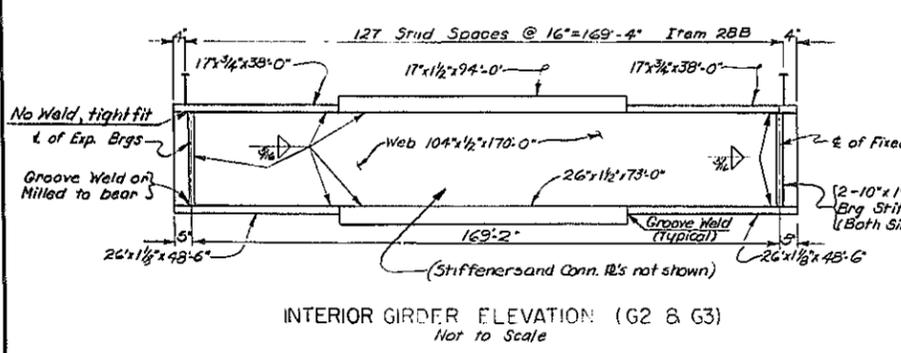
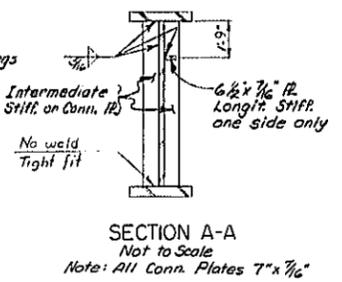
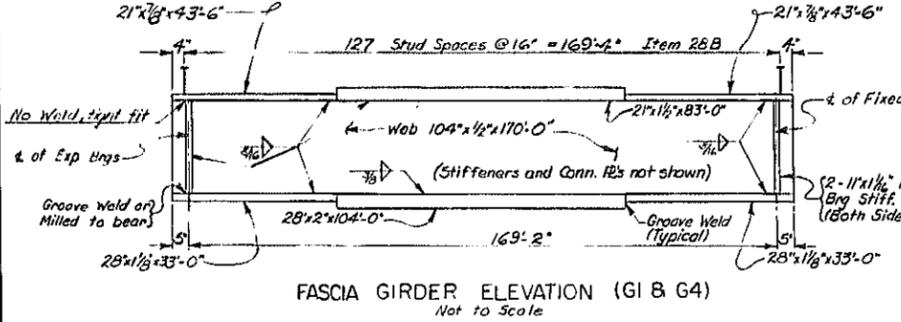
DRAWING NO. 18 OF 50

FED. RD. REG. NO.	STATE	FEDERAL AID PROJECT NO.	SHEET NO.	TOTAL SHEETS
1	NEW YORK	1-690-3(28) 1-481-2(116)	199	309

INTERSTATE ROUTE CONNECTION 570
BUTTERNUT INTERCHANGE (PHASE 2)
ONONDAGA COUNTY



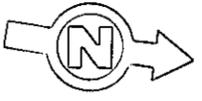
Note: Stud Shear Connectors shall be 6" high.
For Bearings Details see Drawing # 42 and 43
For Joint Details see Drawing # 31
For Trough Details see Drawing # 39



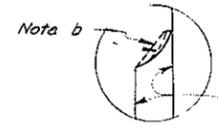
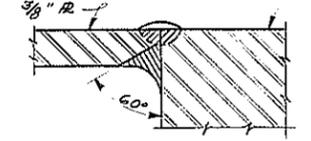
Height of Haunch varies (see Table of Haunch Heights Drawing No. 34)
Bottom of Slab elevations shown in table are computed for this

GIRDER	NORTHBOUND			SOUTHBOUND			D.L. DEFLECTION			CAMBER	
	THEO. BOT. OF SLAB EL. (F.T.)	THEO. BOT. OF SLAB EL. (F.T.)	THEO. BOT. OF SLAB EL. (F.T.)	STEEL (F.T.)	SLAB (F.T.)	S.D.L. (F.T.)	TOTAL (F.T.)	V.C.C. (F.T.)	TOTAL (F.T.)	(I.I.D.)	
SPAN 3	€ SO. BRG	€ SPAN	€ NO. BRG	€ SO. BRG	€ SPAN	€ NO. BRG					
G1	449.24	449.85	450.29	449.10	449.74	450.20	.11	.15	.11	.37	
G2	449.48	450.09	450.52	449.34	449.97	450.44	.12	.29	.04	.45	
G3	449.46	450.07	450.50	449.39	450.02	450.48	.12	.29	.04	.45	
G4	449.25	449.85	450.28	449.18	449.80	450.26	.11	.15	.11	.37	

S.D.L. = superimposed dead load, includes weight of parapet and railing.
V.C.C. = vertical curve correction



Top of Bottom Flange when bottom flange is in tension
Bottom of Top Flange when top flange is in tension



Note b: All references to the connection plate in notes a, b, c and d refer to this plate

Note:
a. The connection plate shall be prepared and welded as a single bevel groove weld as shown in Detail #1. It shall then be Air Carbon-Arc gauged from the second side into sound weld metal and then welded as detailed. All welding shall be in the flat or 'downhand' position.
b. The plate may be of any shape that will provide after welding, cutting, and finish grinding a smooth transition from the flange edge of a minimum radius of 12".
c. Both the connection plate and flange are to be the same type of steel.
d. Field welding to the connection plate will not be permitted.

LATERAL BRACING DETAILS
Not to Scale

For additional details of bottom lateral bracing see drawing No. 22

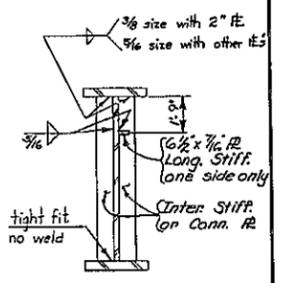
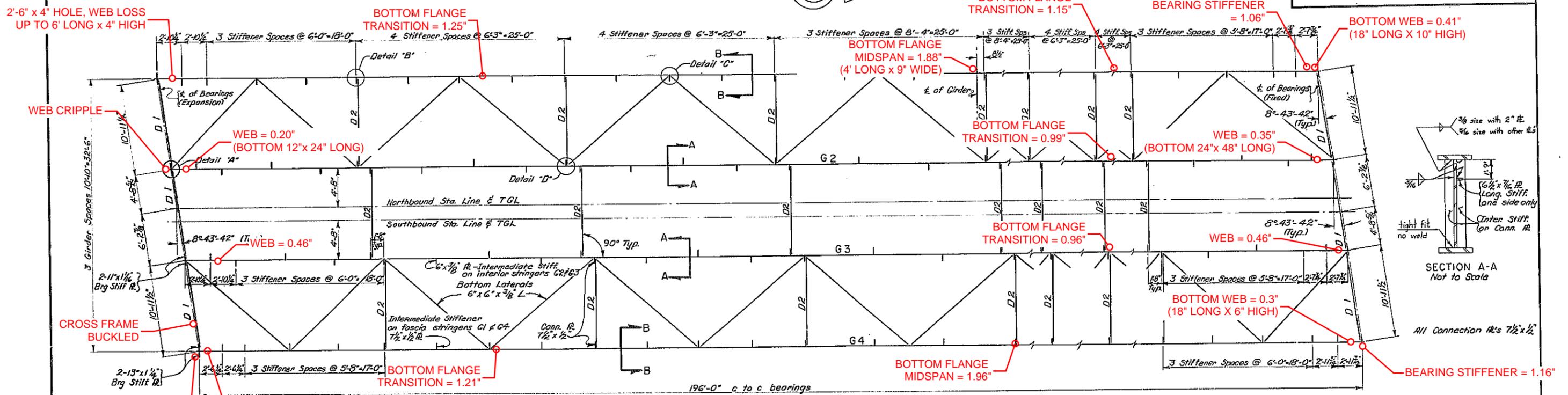
The webs, flanges and gusset plates for bottom lateral bracing for the Span 3 girders shall be A.S.T.M. Designation A441 steel. Stiffeners, diaphragms, bearings, and bottom lateral bracing (except gusset plates) shall be A.S.T.M. A36 steel.

PROJECT ENGINEER: R. PARKER
IN CHARGE OF: F.W. ECKEL
DESIGNED BY: S. BOWEN
DESIGN CHECKED BY: H. THOMPSON
DETAILED BY: J. QUINN
DETAIL CHECKED BY: D.H. SMITH

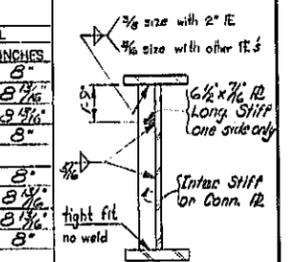
BRIDGE NO. 2
INTERSTATE ROUTE 481 OVER DEWITT YARDS
BIN 1093571 SOUTHBOUND
SPAN 3
DRAWING NO. 21 OF 50

FED. RD. REG. NO.	STATE	FEDERAL AID PROJECT NO.	SHEET NO.	TOTAL SHEETS
1	NEW YORK	I-090-3(28) 1-481-2(116)	200	309

INTERSTATE ROUTE CONNECTION 570
BUTTERNUT INTERCHANGE (PHASE 2)
ONDONAGA COUNTY



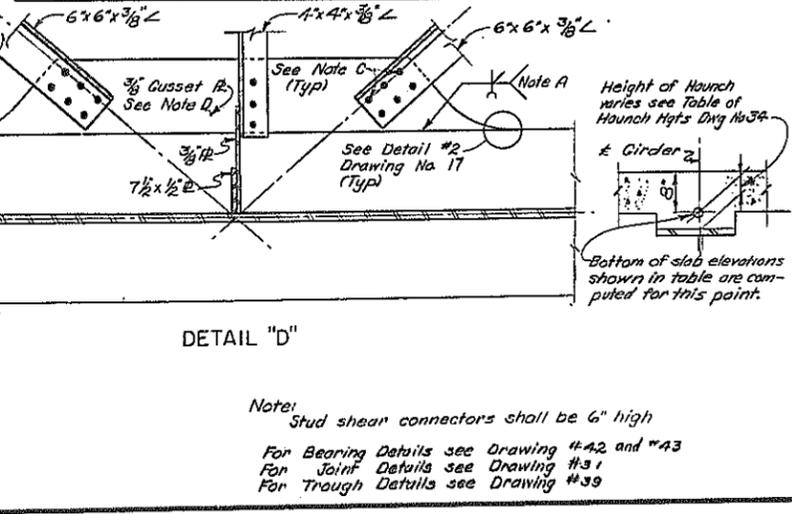
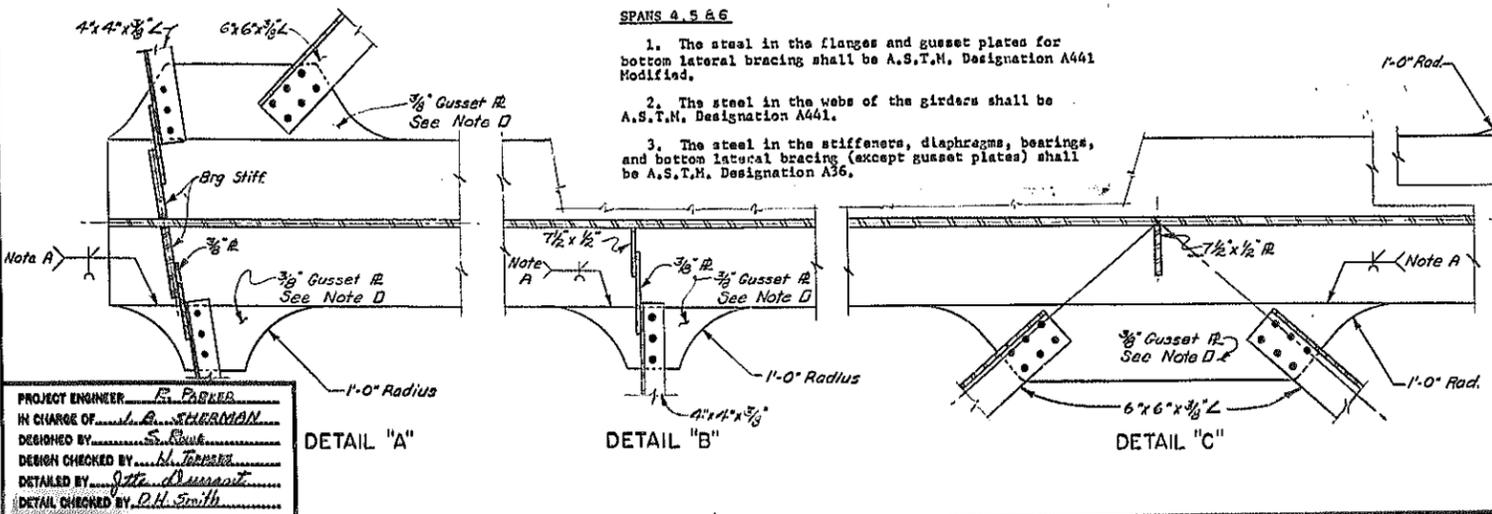
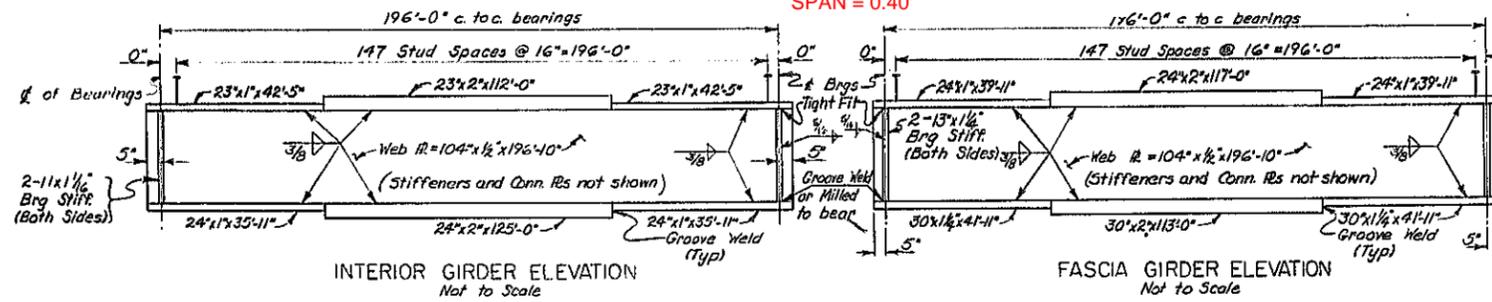
SECTION A-A
Not to Scale
All Connection R's 7 1/2 x 1/2



SECTION B-B
Not to Scale

STEEL LAYOUT - SPANS 4, 5 & 6 (NB & SB)
Scale: 3/8" = 1'-0"

GIRDER	SOUTHBOUND				NORTHBOUND				DL DEFLECTION (FT)				CAMBER		
	THEO. BOT. OF SLAB EL.	± SO. BRG	± NO. BRG	± SPAN	THEO. BOT. OF SLAB EL.	± SO. BRG	± NO. BRG	± SPAN	STEEL	SLAB	SDL	TOTAL DL	VCC (FT)	TOTAL FEET	INCHES
SPAN 4															
G1	450.21	450.54	450.65	450.30	450.59	450.66	450.66	450.66	0.172	0.212	0.168	0.552	0.113	0.665	8"
G2	450.45	450.77	450.86	450.53	450.82	450.89	450.89	450.89	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"
G3	450.48	450.80	450.89	450.51	450.80	450.85	450.85	450.85	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"
G4	450.27	450.38	450.67	450.29	450.57	450.63	450.63	450.63	0.172	0.212	0.168	0.552	0.113	0.665	8"
SPAN 5															
G1	450.63	450.50	450.14	450.66	450.50	450.10	450.10	450.10	0.172	0.212	0.168	0.552	0.113	0.665	8"
G2	450.86	450.72	450.36	450.89	450.72	450.32	450.32	450.32	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"
G3	450.89	450.75	450.38	450.85	450.68	450.28	450.28	450.28	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"
G4	450.66	450.52	450.15	450.63	450.45	450.05	450.05	450.05	0.172	0.212	0.168	0.552	0.113	0.665	8"
SPAN 6															
G1	450.13	449.54	448.72	450.09	449.47	448.62	448.62	448.62	0.172	0.212	0.168	0.552	0.113	0.665	8"
G2	450.35	449.75	448.93	450.31	449.68	448.83	448.83	448.83	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"
G3	450.37	449.77	448.94	450.27	449.64	448.78	448.78	448.78	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"
G4	450.14	449.53	448.70	450.04	449.40	448.54	448.54	448.54	0.172	0.212	0.168	0.552	0.113	0.665	8"



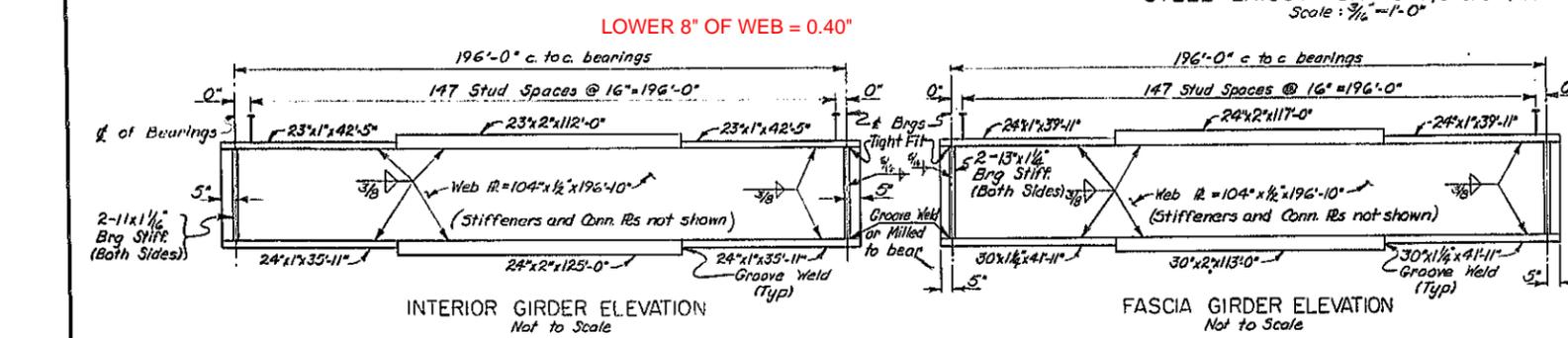
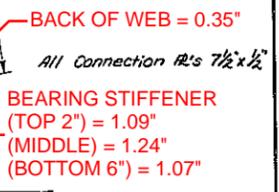
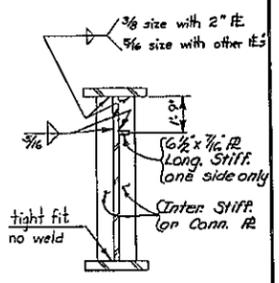
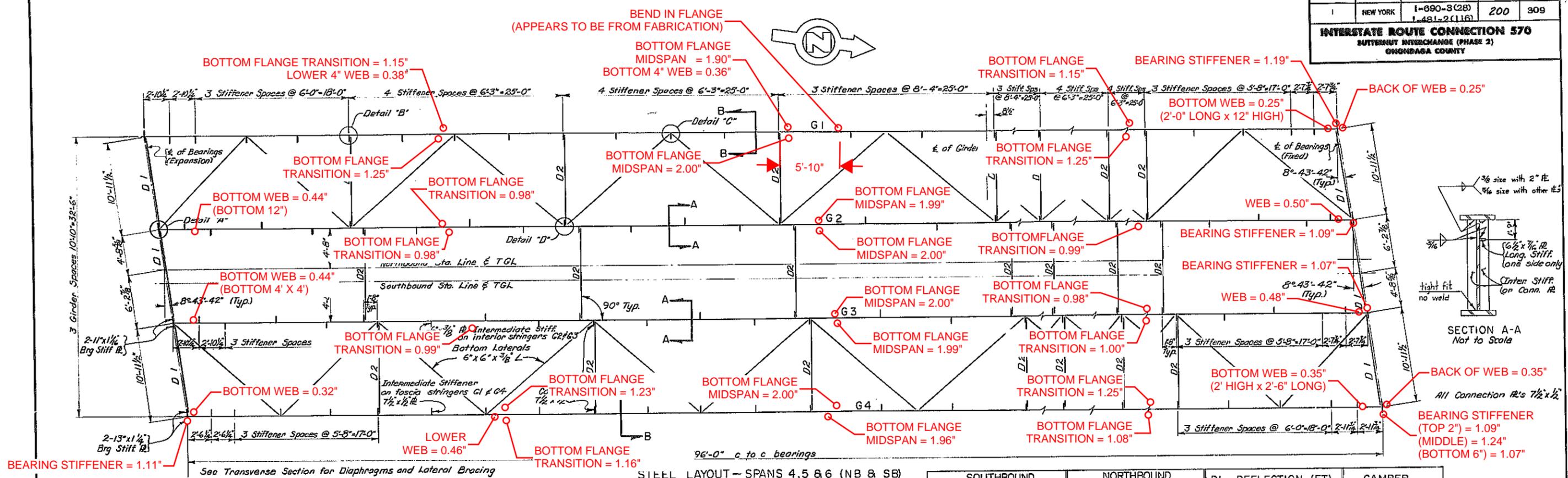
Notes:
 A. The connection plate shall be prepared and welded as a single bevel groove weld as shown in Detail A. It shall then be Min Carbon-Arc gouged from the second side into sound weld metal and then welded as detailed. All welding shall be in the flat or "downhand" position.
 B. The plate may be of any shape that will provide after welding, cutting, and finish grinding a smooth transition from the flange edge of a minimum radius of 12".
 C. Lateral bracing shall be attached to gusset plates with 3/8 inch high strength bolts. Field welding will not be permitted.
 D. The gusset plate shall be the same type of steel as the flange to which it is welded.

PROJECT ENGINEER: R. PARKER
 IN CHARGE OF: S. SHERMAN
 DESIGNED BY: S. PARKER
 DESIGN CHECKED BY: M. TARRANT
 DETAILED BY: J. D. DUNN
 DETAIL CHECKED BY: D.H. SMITH

BRIDGE NO. 2
 INTERSTATE ROUTE 481 OVER DEWITT YARDS
 SPANS 4
SOUTHBOUND BIN 1093571
 DRAWING NO. 2.2 OF 50

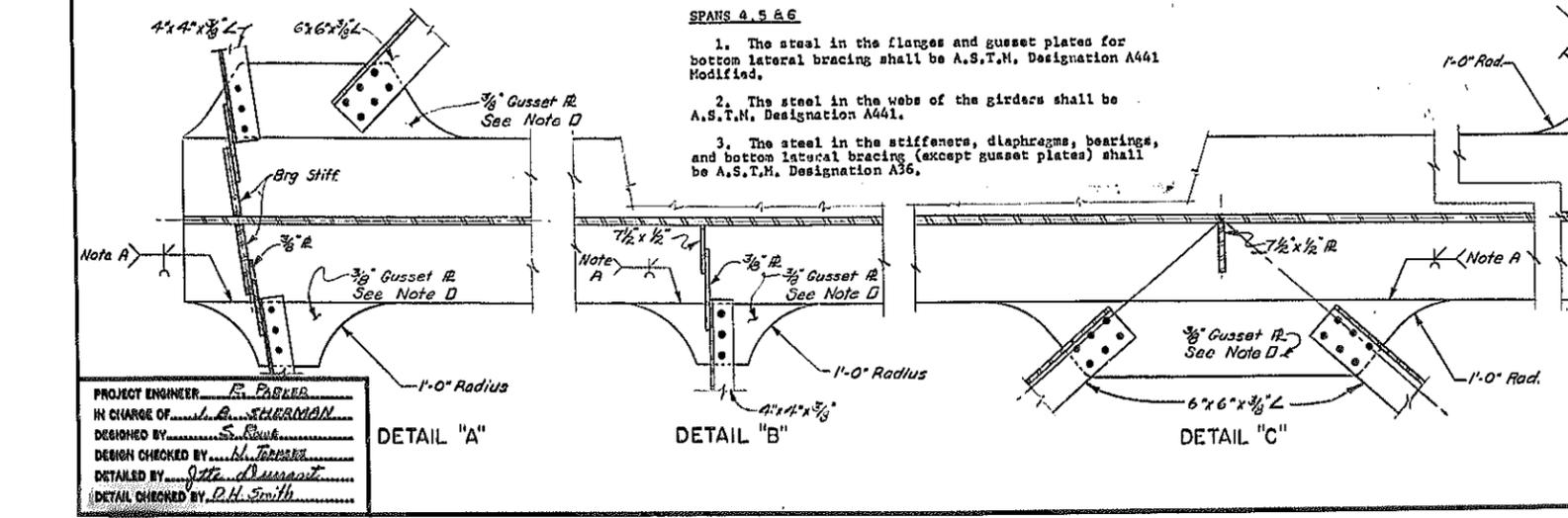
FED. RD. REG. NO.	STATE	FEDERAL AID PROJECT NO.	SHEET NO.	TOTAL SHEETS
1	NEW YORK	1-090-3(28) 1-481-2(116)	200	309

INTERSTATE ROUTE CONNECTION 570
BUTTERNUT INTERCHANGE (PHASE 2)
ONONDAGA COUNTY



STEEL LAYOUT - SPANS 4, 5 & 6 (NB & SB)
Scale: 3/8" = 1'-0"

GIRDER	SOUTHBOUND				NORTHBOUND				DL DEFLECTION (FT)				CAMBER		
	THEO. BOT. OF SLAB EL.	± SO. BRG	± SPAN	± NO. BRG	THEO. BOT. OF SLAB EL.	± SO. BRG	± SPAN	± NO. BRG	STEEL	SLAB	SDL	TOTAL DL	VCC (FT)	TOTAL FEET	INCHES
SPAN 4															
G1	450.21	450.54	450.65	450.30	450.59	450.66	0.172	0.212	0.168	0.552	0.113	0.665	8"		
G2	450.45	450.77	450.86	450.53	450.82	450.89	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"		
G3	450.48	450.80	450.89	450.51	450.80	450.85	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"		
G4	450.27	450.38	450.67	450.29	450.57	450.63	0.172	0.212	0.168	0.552	0.113	0.665	8"		
SPAN 5															
G1	450.63	450.50	450.14	450.66	450.50	450.10	0.172	0.212	0.168	0.552	0.113	0.665	8"		
G2	450.86	450.72	450.36	450.89	450.72	450.32	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"		
G3	450.89	450.75	450.38	450.85	450.68	450.28	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"		
G4	450.66	450.52	450.15	450.63	450.45	450.05	0.172	0.212	0.168	0.552	0.113	0.665	8"		
SPAN 6															
G1	450.13	449.54	448.72	450.09	449.47	448.62	0.172	0.212	0.168	0.552	0.113	0.665	8"		
G2	450.35	449.75	448.93	450.31	449.68	448.83	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"		
G3	450.37	449.77	448.94	450.27	449.64	448.78	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"		
G4	450.14	449.53	448.70	450.04	449.40	448.54	0.172	0.212	0.168	0.552	0.113	0.665	8"		



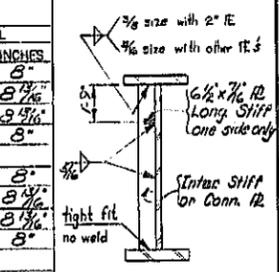
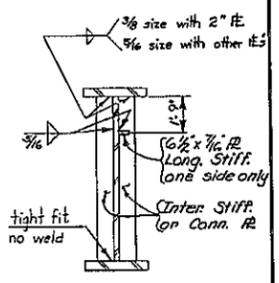
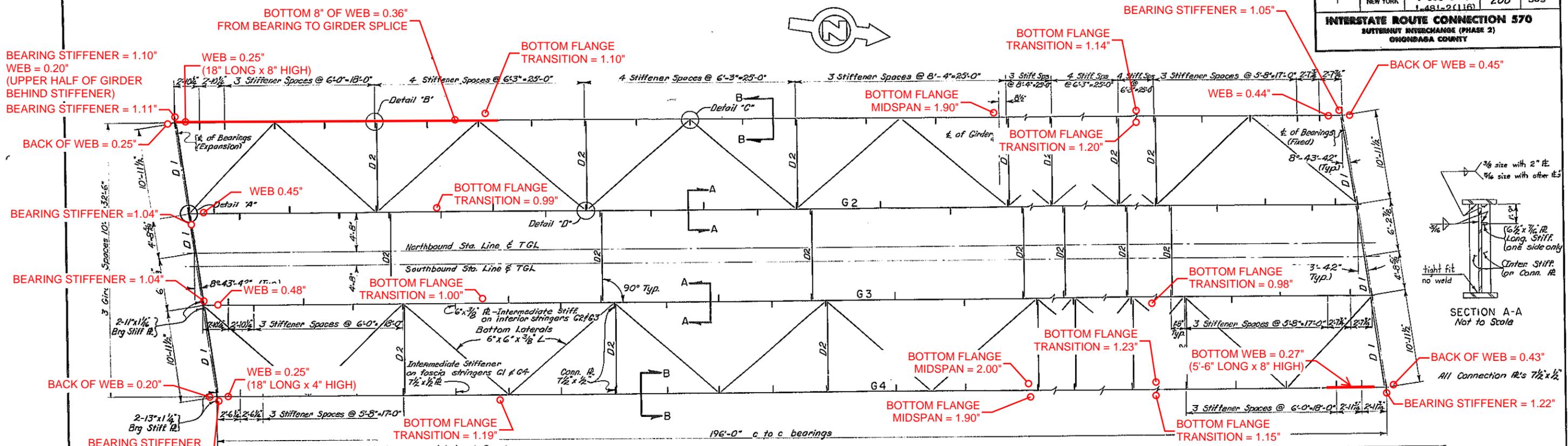
Notes:
 A. The connection plate shall be prepared and welded as a single bevel groove weld as shown in Detail #1. It shall then be Min Carbon-Arc gouged from the second side into sound weld metal and then welded as detailed. All welding shall be in the flat or "downhand" position.
 B. The plate may be of any shape that will provide after welding, cutting, and finish grinding a smooth transition from the flange edge of a minimum radius of 12".
 C. Lateral bracing shall be attached to gusset plates with 3/8" x high strength bolts. Field welding will not be permitted.
 D. The gusset plate shall be the same type of steel as the flange to which it is welded.

PROJECT ENGINEER: R. PARKER
 IN CHARGE OF: J. SHERMAN
 DESIGNED BY: S. ROSE
 DESIGN CHECKED BY: M. TARRANT
 DETAILED BY: J. D. MONTGOMERY
 DETAIL CHECKED BY: D.H. SMITH

BRIDGE NO. 2
 INTERSTATE ROUTE 481 OVER DEWITT YARDS
 SPANS 5
SOUTHBOUND BIN 1093571
 DRAWING NO. 2.2 OF 50

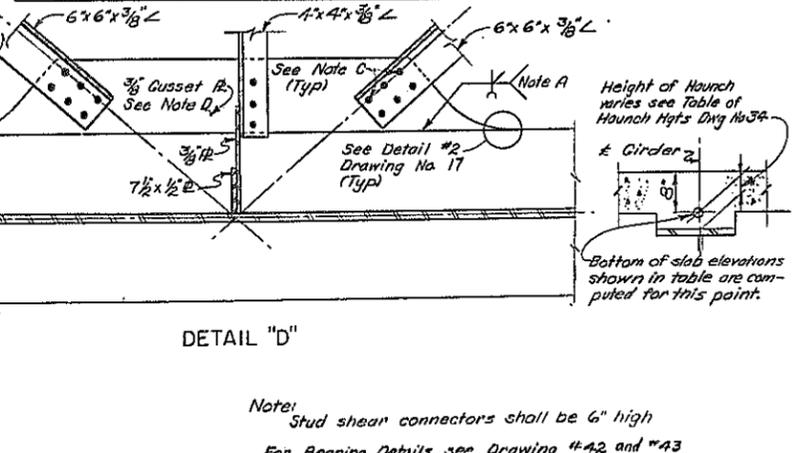
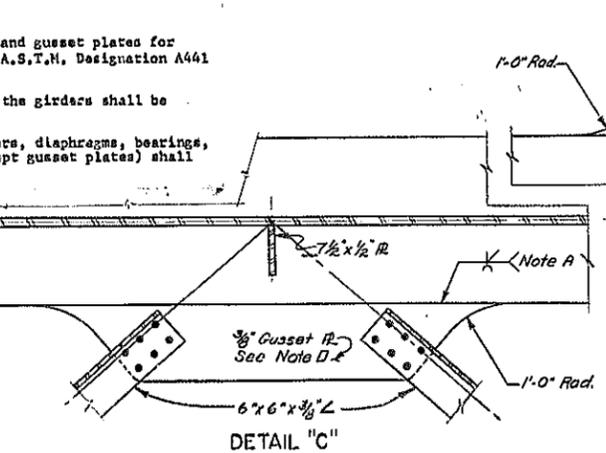
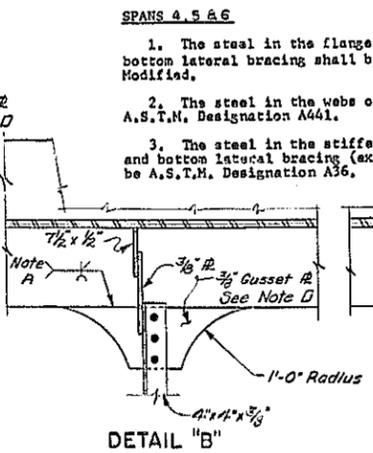
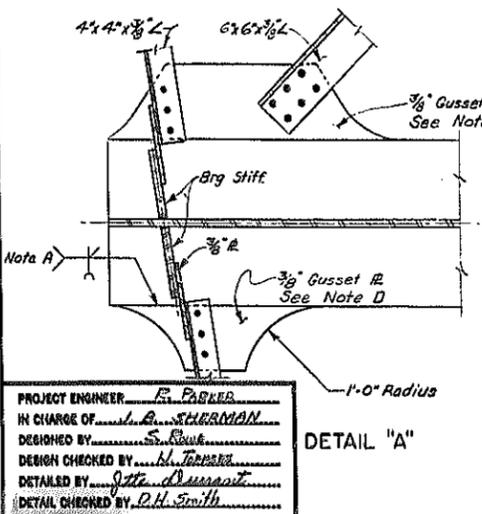
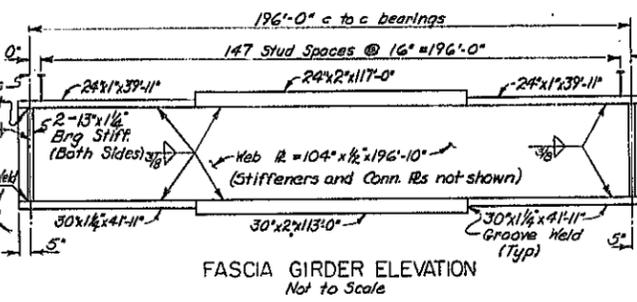
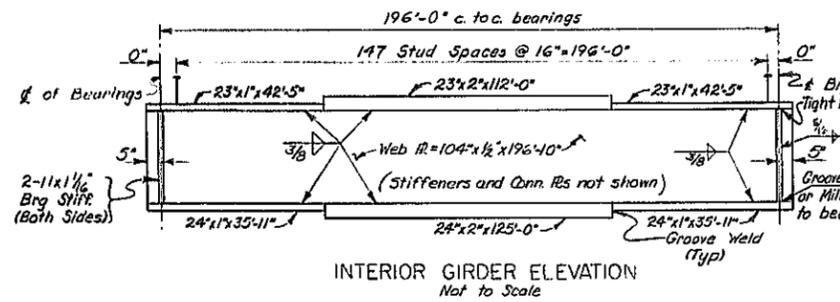
FED. RD. REG. NO.	STATE	FEDERAL AID PROJECT NO.	SHEET NO.	TOTAL SHEETS
1	NEW YORK	I-090-3(28) 1-481-2(116)	200	309

INTERSTATE ROUTE CONNECTION 570
BUTTERNUT INTERCHANGE (PHASE 2)
ONDONAGA COUNTY



STEEL LAYOUT - SPANS 4, 5 & 6 (NB & SB)
Scale: 3/8" = 1'-0"

GIRDER	SOUTHBOUND				NORTHBOUND				DL DEFLECTION (FT)				CAMBER		
	THEO. BOT. OF SLAB EL.	± SO. BRG	± NO. BRG	± NO. BRG	THEO. BOT. OF SLAB EL.	± SO. BRG	± NO. BRG	± NO. BRG	STEEL	SLAB	SDL	TOTAL DL	VCC (FT)	TOTAL FEET	INCHES
SPAN 4															
G1	450.21	450.54	450.65	450.30	450.59	450.66	450.66	450.66	0.172	0.212	0.168	0.552	0.113	0.665	8"
G2	450.45	450.77	450.86	450.53	450.82	450.89	450.89	450.89	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"
G3	450.48	450.80	450.89	450.51	450.80	450.85	450.85	450.85	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"
G4	450.27	450.38	450.67	450.29	450.57	450.63	450.63	450.63	0.172	0.212	0.168	0.552	0.113	0.665	8"
SPAN 5															
G1	450.63	450.50	450.14	450.66	450.50	450.10	450.10	450.10	0.172	0.212	0.168	0.552	0.113	0.665	8"
G2	450.86	450.72	450.36	450.89	450.72	450.32	450.32	450.32	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"
G3	450.89	450.75	450.38	450.85	450.68	450.28	450.28	450.28	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"
G4	450.66	450.52	450.15	450.63	450.45	450.05	450.05	450.05	0.172	0.212	0.168	0.552	0.113	0.665	8"
SPAN 6															
G1	450.13	449.54	448.72	450.09	449.47	448.62	448.62	448.62	0.172	0.212	0.168	0.552	0.113	0.665	8"
G2	450.35	449.75	448.93	450.31	449.68	448.83	448.83	448.83	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"
G3	450.37	449.77	448.94	450.27	449.64	448.78	448.78	448.78	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"
G4	450.14	449.53	448.70	450.04	449.40	448.54	448.54	448.54	0.172	0.212	0.168	0.552	0.113	0.665	8"



Notes:
 A. The connection plate shall be prepared and welded as a single bevel groove weld as shown in Detail #1. It shall then be Min Carbon-Arc gouged from the second side into sound weld metal and then welded as detailed. All welding shall be in the flat or "downhand" position.
 B. The plate may be of any shape that will provide after welding, cutting, and finish grinding a smooth transition from the flange edge of a minimum radius of 12".
 C. Lateral bracing shall be attached to gusset plates with 3/8" & high strength bolts. Field welding will not be permitted.
 D. The gusset plate shall be the same type of steel as the flange to which it is welded.

PROJECT ENGINEER: R. PASTER
 IN CHARGE OF: S. SHERMAN
 DESIGNED BY: S. ROWE
 DESIGN CHECKED BY: M. TARRANT
 DETAILED BY: J. D. DUMONT
 DETAIL CHECKED BY: D. H. SMITH

BRIDGE NO. 2
 INTERSTATE ROUTE 481 OVER DEWITT YARDS
 SPANS 6
 SOUTHBOUND BIN 1093571
 DRAWING NO. 2.2 OF 50

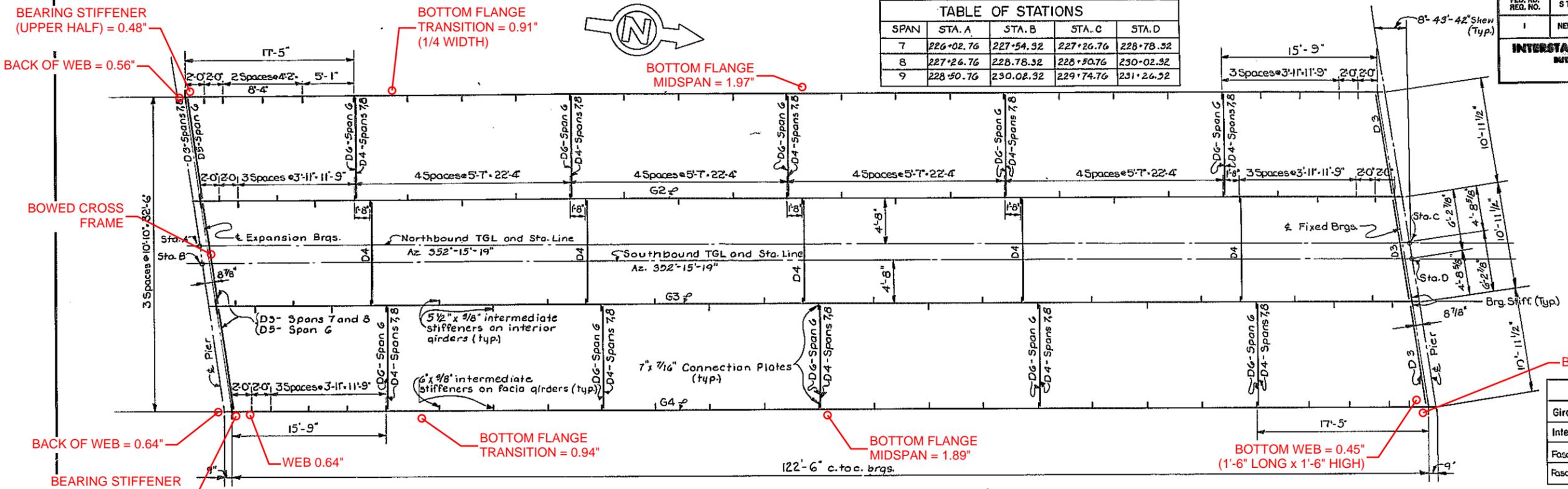
Note:
 Stud shear connectors shall be 6" high
 For Bearing Details see Drawing #42 and #43
 For Joint Details see Drawing #31
 For Trough Details see Drawing #39

FED. RD. REG. NO.	STATE	FEDERAL AID PROJECT NO.	SHEET NO.	TOTAL SHEETS
1	NEW YORK	1-690-3(26) 1-481-2(116)	202	309

INTERSTATE ROUTE CONNECTION 570
 INTERMITTENT INTERCHANGE (PHASE 2)
 ONONDAGA COUNTY

TABLE OF STATIONS

SPAN	STA. A	STA. B	STA. C	STA. D
7	226+02.76	227+54.32	227+26.76	228+78.52
8	227+26.76	228+78.52	228+50.76	230+02.32
9	228+50.76	230+02.32	229+74.76	231+26.32



STEEL FRAMING PLAN SPANS 7, 8 & 9 (NB+SB)
 Scale: 3/16" = 1'-0"

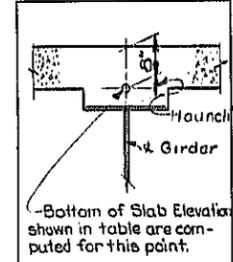
BEARING STIFFENER SIZES

Girder	Size of each plate (2 plates @ each end of girder)
Interior spans 7 & 8	8" x 3/4"
Fascia span 7	8" x 3/4"
Fascia spans 8 & 9	9" x 7/8"

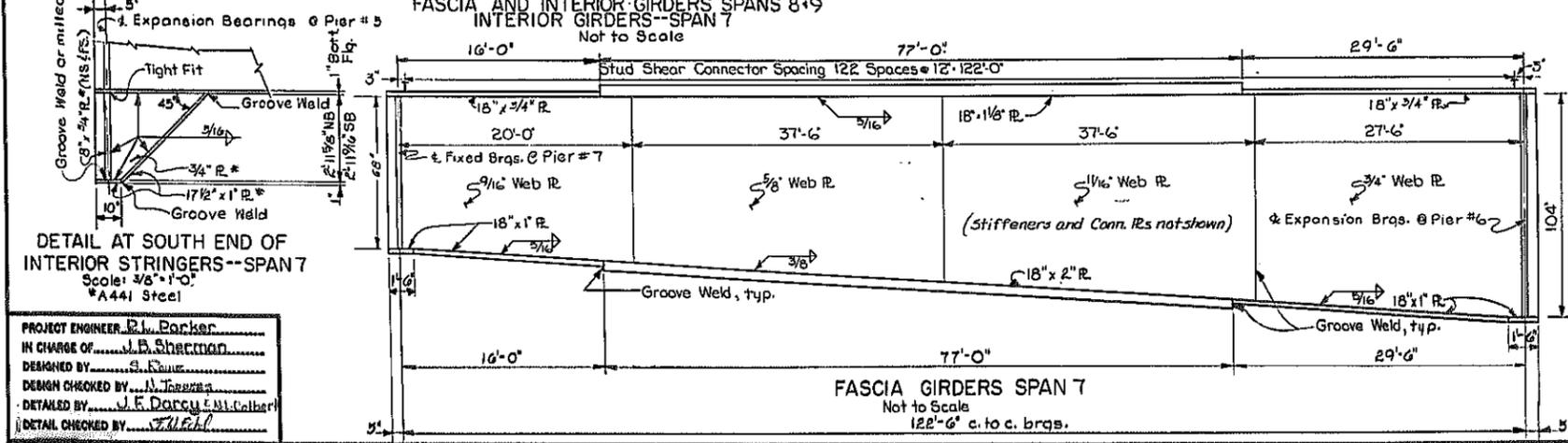
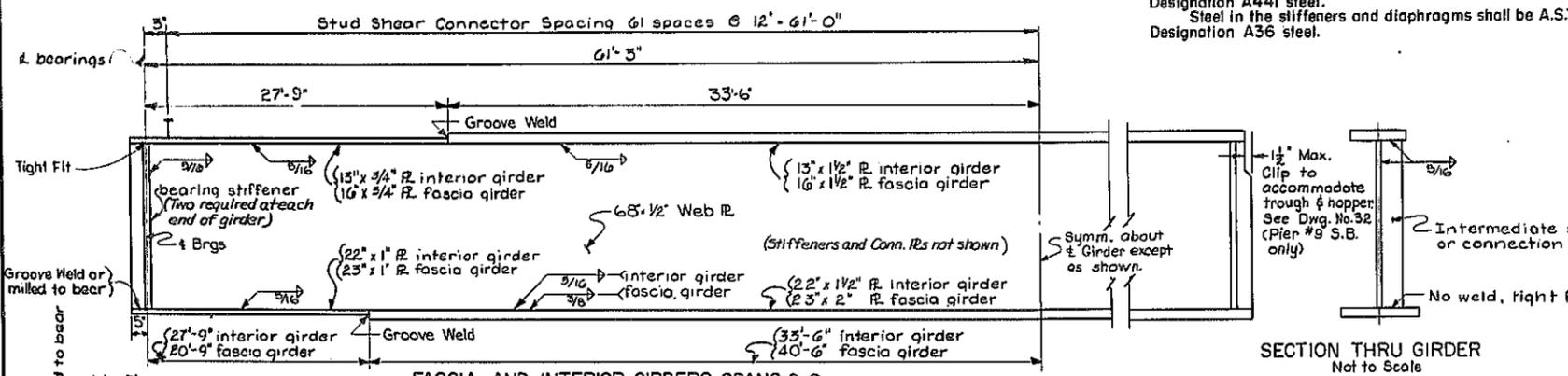
Steel in the girder webs and flanges shall be A.S.T.M. Designation A441 steel.
 Steel in the stiffeners and diaphragms shall be A.S.T.M. Designation A36 steel.

BOTTOM OF SLAB ELEVATIONS

NORTHBOUND				SOUTHBOUND					
GIRDER	SPAN	EXP. BRGS.	FIX. BRGS.	GIRDER	SPAN	EXP. BRGS.	FIX. BRGS.		
G1	7	448.60	447.95	447.22	G1	7	448.70	448.07	447.35
G2	7	448.81	448.16	447.42	G2	7	448.91	448.28	447.56
G3	7	448.77	448.11	447.37	G3	7	448.93	448.29	447.57
G4	7	448.52	447.87	447.12	G4	7	448.68	448.05	447.32
G1	8	447.20	446.37	445.45	G1	8	447.33	446.52	445.63
G2	8	447.40	446.57	445.65	G2	8	447.54	446.73	445.83
G3	8	447.35	446.52	445.59	G3	8	447.55	446.73	445.83
G4	8	447.10	446.27	445.34	G4	8	447.30	446.48	445.58
G1	9	445.43	444.42	443.53	G1	9	445.60	444.61	443.97
G2	9	445.63	444.62	443.88	G2	9	445.80	444.81	443.74
G3	9	445.57	444.86	443.46	G3	9	445.81	444.81	443.73
G4	9	445.32	444.50	443.20	G4	9	445.56	444.56	443.48



*Height of Haunch varies. See Table of Haunch Heights on Drawing No. 34.



DETAIL AT SOUTH END OF INTERIOR STRINGERS--SPAN 7
 Scale: 3/8" = 1'-0"
 A441 Steel

PROJECT ENGINEER: R.L. Barker
 IN CHARGE OF: J.D. Sherman
 DESIGNED BY: S. F. ...
 DESIGN CHECKED BY: J.F. Darcy & M. Calvert
 DETAILED BY: J.F. Darcy & M. Calvert
 DETAIL CHECKED BY: J.F. Darcy & M. Calvert

DEFLECTIONS

GIRDER	SPAN	DEFLECTIONS				CAMBER	
		STEEL (FT)	SLAB (FT)	SDL (FT)	TOTAL (FT)	V.C.C. (FT)	TOTAL (FT)
G1	7	.05	.09	.06	.20	.04	.24
G2	7	.08	.24	.03	.35	.04	.39
G3	7	.08	.24	.03	.35	.04	.39
G4	7	.05	.09	.06	.20	.04	.24
G1	8	.07	.12	.08	.27	.04	.31
G2	8	.08	.24	.03	.35	.04	.39
G3	8	.08	.24	.03	.35	.04	.39
G4	8	.07	.12	.08	.27	.04	.31
G1	9	.07	.12	.08	.27	.02	.29
G2	9	.08	.24	.03	.35	.03	.38
G3	9	.08	.24	.03	.35	.04	.39
G4	9	.07	.12	.08	.27	.04	.31
G1	9	.07	.12	.08	.27	-.06	.21
G2	9	.08	.24	.03	.35	.01	.36
G3	9	.08	.24	.03	.35	.04	.39
G4	9	.07	.12	.08	.27	.04	.31

V.C.C.=Vertical Curve Camber
 S.D.L.=Superimposed dead load, includes the weight of sidewalk railing.

BRIDGE NO. 2
 INTERSTATE ROUTE 481 OVER DEWITT YARDS
 SPANS 7
SOUTHBOUND BIN 1093571
 DRAWING NO. 24 OF 50

Bin #2 Dewitt Yds Steel Frame

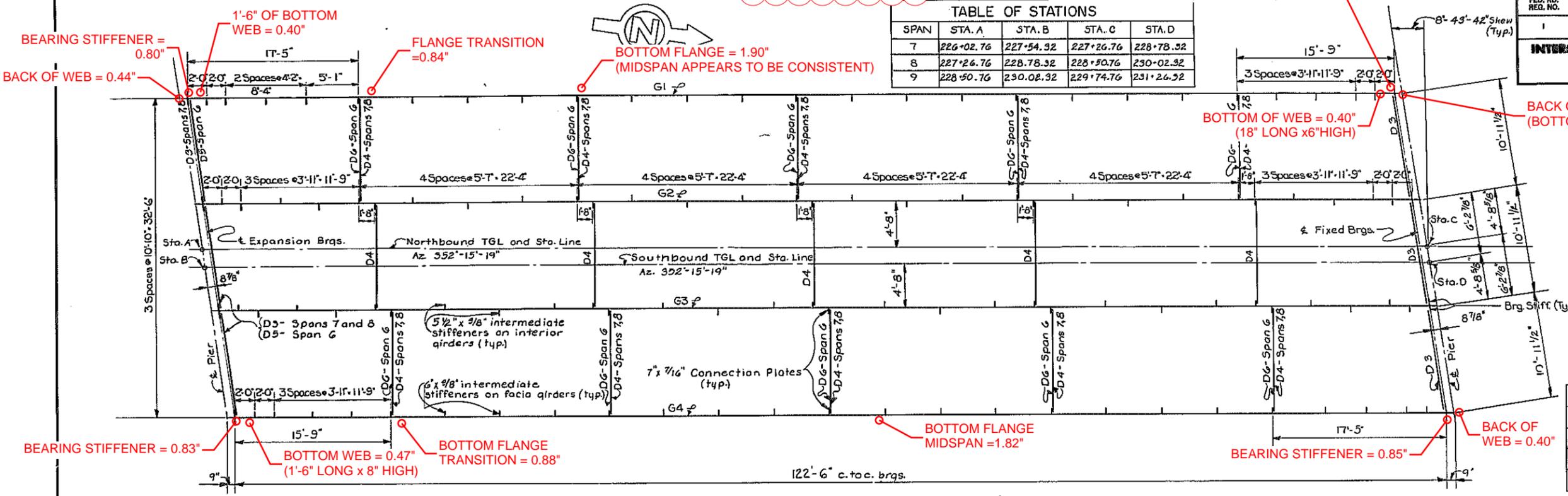
MIDSPAN INACCESSIBLE DUE TO TREES

F.I.S.H. 70-7

SPAN	STA. A	STA. B	STA. C	STA. D
7	226+02.76	227+54.32	227+26.76	228+78.52
8	227+26.76	228+78.52	228+50.76	230+02.32
9	228+50.76	230+02.32	229+74.76	231+26.32

FED. RD. REG. NO.	STATE	FEDERAL AID PROJECT NO.	SHEET NO.	TOTAL SHEETS
1	NEW YORK	1-690-3(26)	202	309

INTERSTATE ROUTE CONNECTION 570
MATHERN INTERCHANGE (PHASE 2)
ONEIDA COUNTY

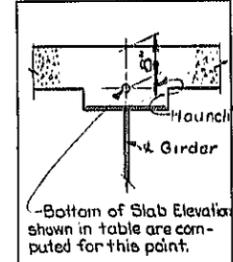


Girder	Size of each plate (2 plates @ each end of girder)
Interior spans 7 & 8	8" x 3/4"
Fascia span 7	8" x 3/4"
Fascia spans 8 & 9	9" x 7/8"

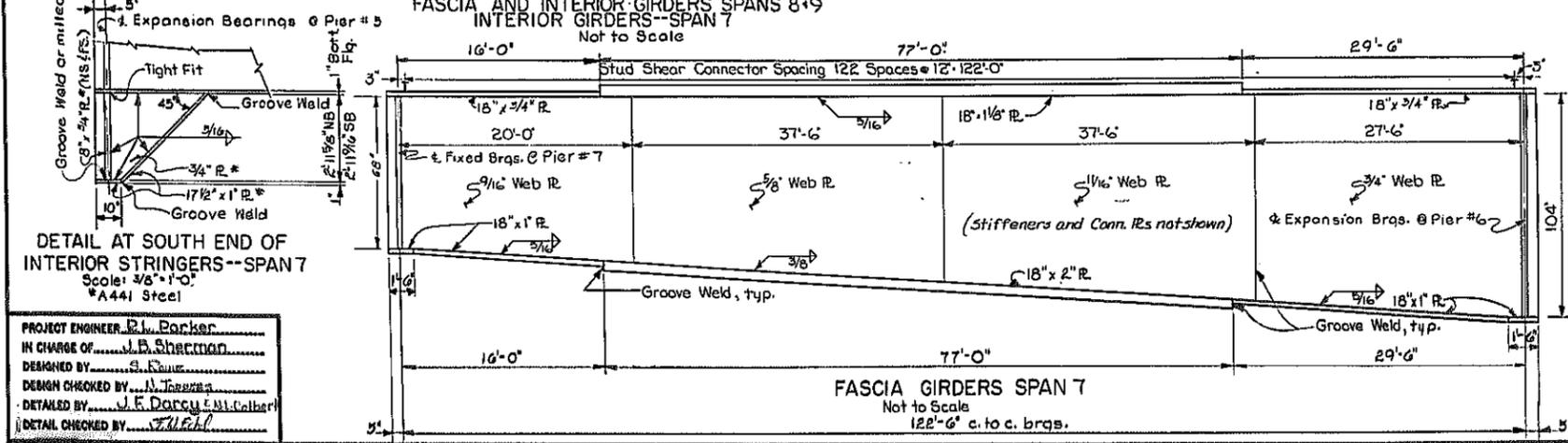
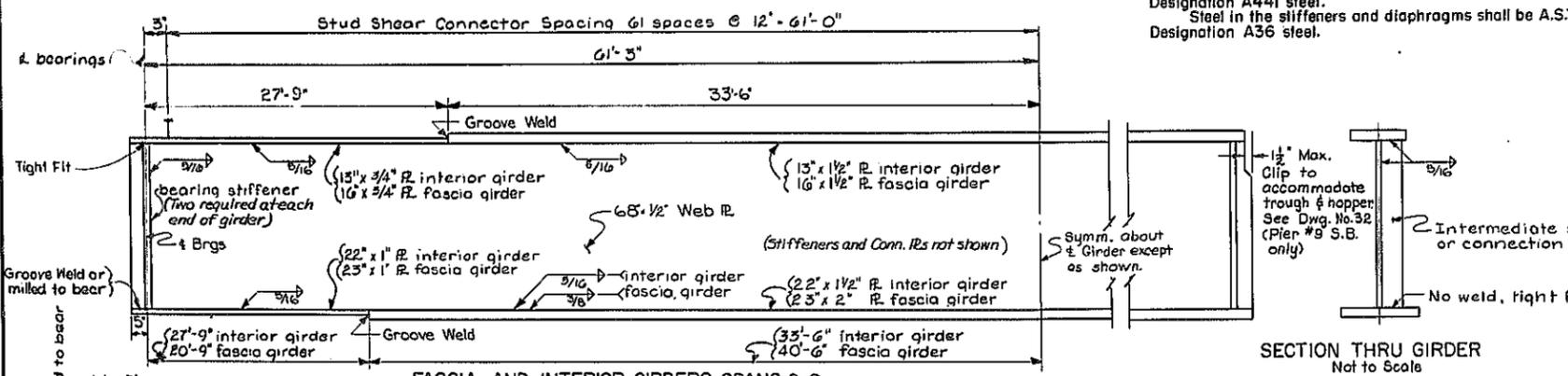
STEEL FRAMING PLAN SPANS 7, 8 & 9 (NB+SB)
Scale: 3/16" = 1'-0"

Steel in the girder webs and flanges shall be A.S.T.M. Designation A441 steel.
Steel in the stiffeners and diaphragms shall be A.S.T.M. Designation A36 steel.

NORTHBOUND				SOUTHBOUND					
GIRDER	SPAN	EXP. BRGS.	FIX. BRGS.	GIRDER	SPAN	EXP. BRGS.	FIX. BRGS.		
G1	7	448.60	447.95	447.22	G1	7	448.70	448.07	447.35
G2	7	448.81	448.16	447.42	G2	7	448.91	448.28	447.56
G3	7	448.77	448.11	447.37	G3	7	448.93	448.29	447.57
G4	7	448.52	447.87	447.12	G4	7	448.68	448.05	447.32
G1	8	447.20	446.37	445.45	G1	8	447.33	446.52	445.63
G2	8	447.40	446.57	445.65	G2	8	447.54	446.73	445.83
G3	8	447.35	446.52	445.59	G3	8	447.55	446.73	445.83
G4	8	447.10	446.27	445.34	G4	8	447.30	446.48	445.58
G1	9	445.43	444.42	443.53	G1	9	445.60	444.61	443.97
G2	9	445.63	444.62	443.88	G2	9	445.80	444.81	443.74
G3	9	445.57	444.86	443.46	G3	9	445.81	444.81	443.73
G4	9	445.32	444.50	443.20	G4	9	445.56	444.56	443.48



*Height of Haunch varies. See Table of Haunch Heights on Drawing No. 34.



PROJECT ENGINEER: R.L. Barker
IN CHARGE OF: J.D. Sherman
DESIGNED BY: S. F. ...
DESIGN CHECKED BY: J.F. Darcy & M. Calvert
DETAILED BY: J.F. Darcy & M. Calvert
DETAIL CHECKED BY: J.F. Darcy & M. Calvert

GIRDER	SPAN	DEFLECTIONS				CAMBER	
		STEEL (FT)	SLAB (FT)	SDL (FT)	TOTAL (FT)	V.C.C. (FT)	TOTAL (FT)
G1	7	.05	.09	.06	.20	.04	.24
G2	7	.08	.24	.03	.35	.04	.39
G3	7	.08	.24	.03	.35	.04	.39
G4	7	.05	.09	.06	.20	.04	.24
G1	8	.07	.12	.08	.27	.04	.31
G2	8	.08	.24	.03	.35	.04	.39
G3	8	.08	.24	.03	.35	.04	.39
G4	8	.07	.12	.08	.27	.04	.31
G1	9	.07	.12	.08	.27	.02	.29
G2	9	.08	.24	.03	.35	.03	.38
G3	9	.08	.24	.03	.35	.04	.39
G4	9	.07	.12	.08	.27	.04	.31
G1	9	.07	.12	.08	.27	-.06	.21
G2	9	.08	.24	.03	.35	.01	.36
G3	9	.08	.24	.03	.35	.04	.39
G4	9	.07	.12	.08	.27	.04	.31

V.C.C.=Vertical Curve Camber
S.D.L.=Superimposed dead load, includes the weight of sidewalk railing.

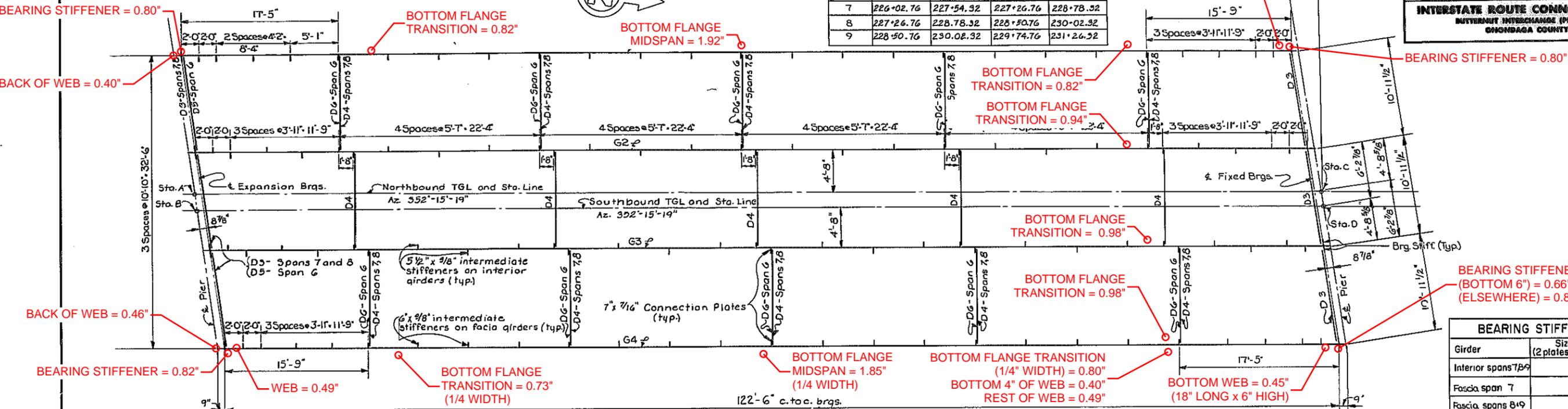
BRIDGE NO. 2
INTERSTATE ROUTE 481 OVER DEWITT YARDS
SPANS 8
SOUTHBOUND BIN 1093571
DRAWING NO. 24 OF 50

Bin #2 DeWitt Yds Steel Frame

FED. RD. REG. NO.	STATE	FEDERAL AID PROJECT NO.	SHEET NO.	TOTAL SHEETS
1	NEW YORK	1-690-3(26) 1-481-2(116)	202	309

INTERSTATE ROUTE CONNECTION 570
 INTERMITTENT INTERCHANGE (PHASE 2)
 ONONDAGA COUNTY

SPAN	STA. A	STA. B	STA. C	STA. D
7	226+02.76	227+54.32	227+26.76	228+78.52
8	227+26.76	228+78.52	228+50.76	230+02.32
9	228+50.76	230+02.32	229+74.76	231+26.32

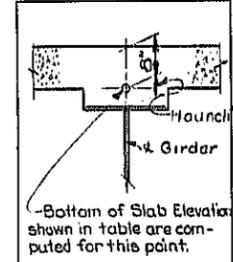


STEEL FRAMING PLAN SPANS 7, 8 & 9 (NB+SB)
 Scale: 3/16" = 1'-0"

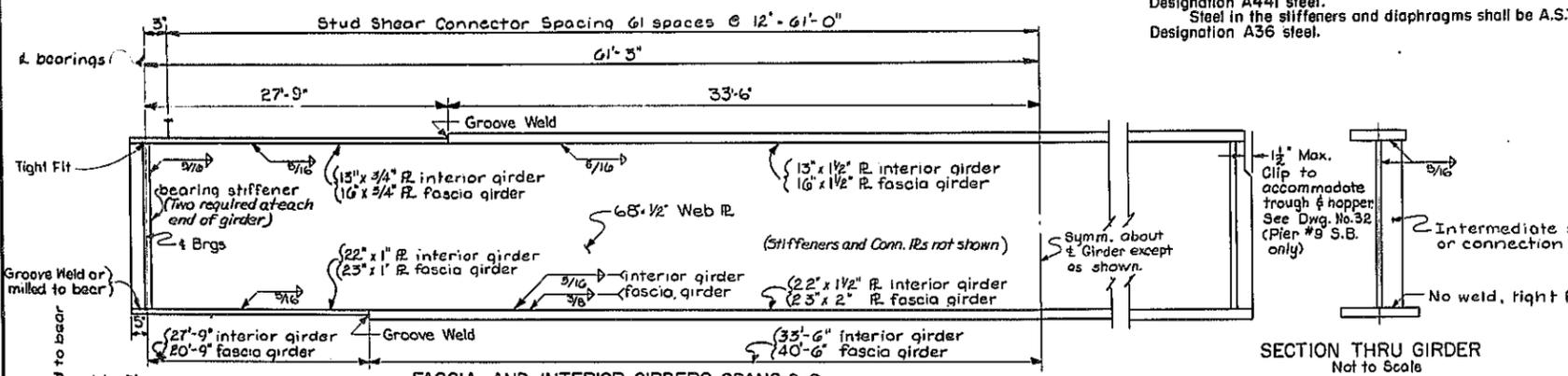
Girder	Size of each plate (2 plates @ each end of girder)
Interior spans 7 & 8	8" x 3/4"
Fascia span 7	8" x 3/4"
Fascia spans 8 & 9	9" x 7/8"

Steel in the girder webs and flanges shall be A.S.T.M. Designation A441 steel.
 Steel in the stiffeners and diaphragms shall be A.S.T.M. Designation A36 steel.

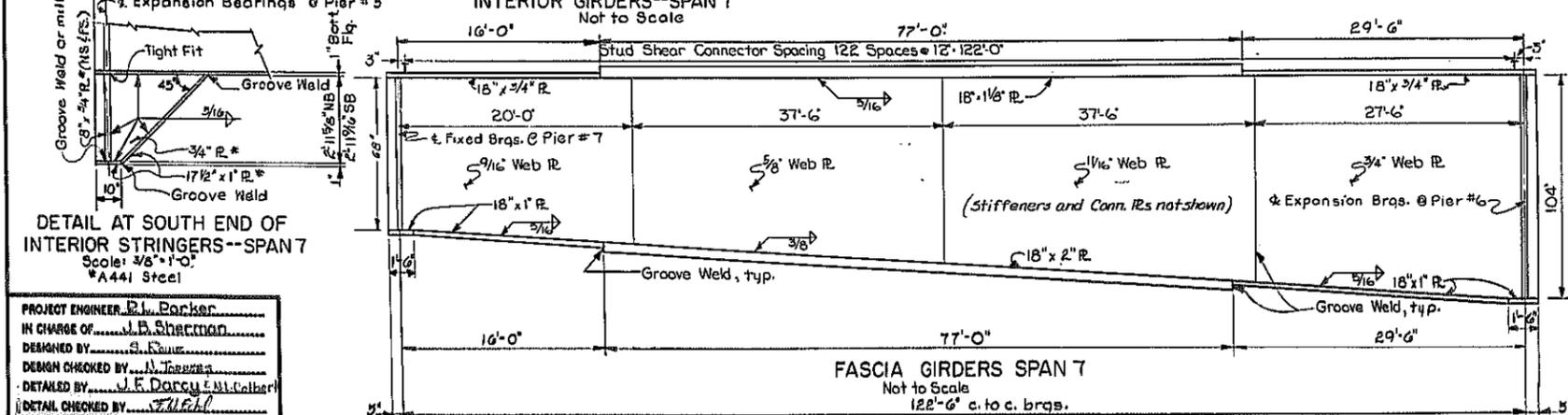
NORTHBOUND				SOUTHBOUND					
GIRDER	SPAN	EXP. BRGS.	FIX. BRGS.	GIRDER	SPAN	EXP. BRGS.	FIX. BRGS.		
G1	7	448.60	447.95	447.22	G1	7	448.70	448.07	447.35
G2	7	448.81	448.16	447.42	G2	7	448.91	448.28	447.56
G3	7	448.77	448.11	447.37	G3	7	448.93	448.29	447.57
G4	7	448.52	447.87	447.12	G4	7	448.68	448.05	447.32
G1	8	447.20	446.37	445.45	G1	8	447.33	446.52	445.63
G2	8	447.40	446.57	445.65	G2	8	447.54	446.73	445.83
G3	8	447.35	446.52	445.59	G3	8	447.55	446.73	445.83
G4	8	447.10	446.27	445.34	G4	8	447.30	446.48	445.58
G1	9	445.43	444.42	443.53	G1	9	445.60	444.61	443.97
G2	9	445.63	444.62	443.88	G2	9	445.80	444.81	443.74
G3	9	445.57	444.86	443.46	G3	9	445.81	444.81	443.73
G4	9	445.32	444.50	443.20	G4	9	445.56	444.56	443.48



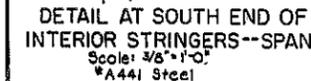
*Height of Haunch varies. See Table of Haunch Heights on Drawing No. 34.



SECTION THRU GIRDER
 Not to Scale



FASCIA GIRDERS SPAN 7
 Not to Scale
 122'-6" c. to c. brgs.



PROJECT ENGINEER: R.L. Barker
 IN CHARGE OF: J.D. Sherman
 DESIGNED BY: S. F. ...
 DESIGN CHECKED BY: J.F. Darcy & M. Calvert
 DETAILED BY: J.F. Darcy & M. Calvert
 DETAIL CHECKED BY: J.F. Darcy & M. Calvert

GIRDER	SPAN	DEFLECTIONS				CAMBER	
		STEEL (FT)	SLAB (FT)	S.D.L. (FT)	TOTAL (FT)	V.C.C. (FT)	TOTAL (FT)
G1	7	.05	.09	.06	.20	.04	.24
G2	7	.08	.24	.03	.35	.04	.39
G3	7	.08	.24	.03	.35	.04	.39
G4	7	.05	.09	.06	.20	.04	.24
G1	8	.07	.12	.08	.27	.04	.31
G2	8	.08	.24	.03	.35	.04	.39
G3	8	.08	.24	.03	.35	.04	.39
G4	8	.07	.12	.08	.27	.04	.31
G1	9	.07	.12	.08	.27	.02	.29
G2	9	.08	.24	.03	.35	.03	.38
G3	9	.08	.24	.03	.35	.04	.39
G4	9	.07	.12	.08	.27	.04	.31
G1	9	.07	.12	.08	.27	-.06	.21
G2	9	.08	.24	.03	.35	.01	.36
G3	9	.08	.24	.03	.35	.04	.39
G4	9	.07	.12	.08	.27	.04	.31

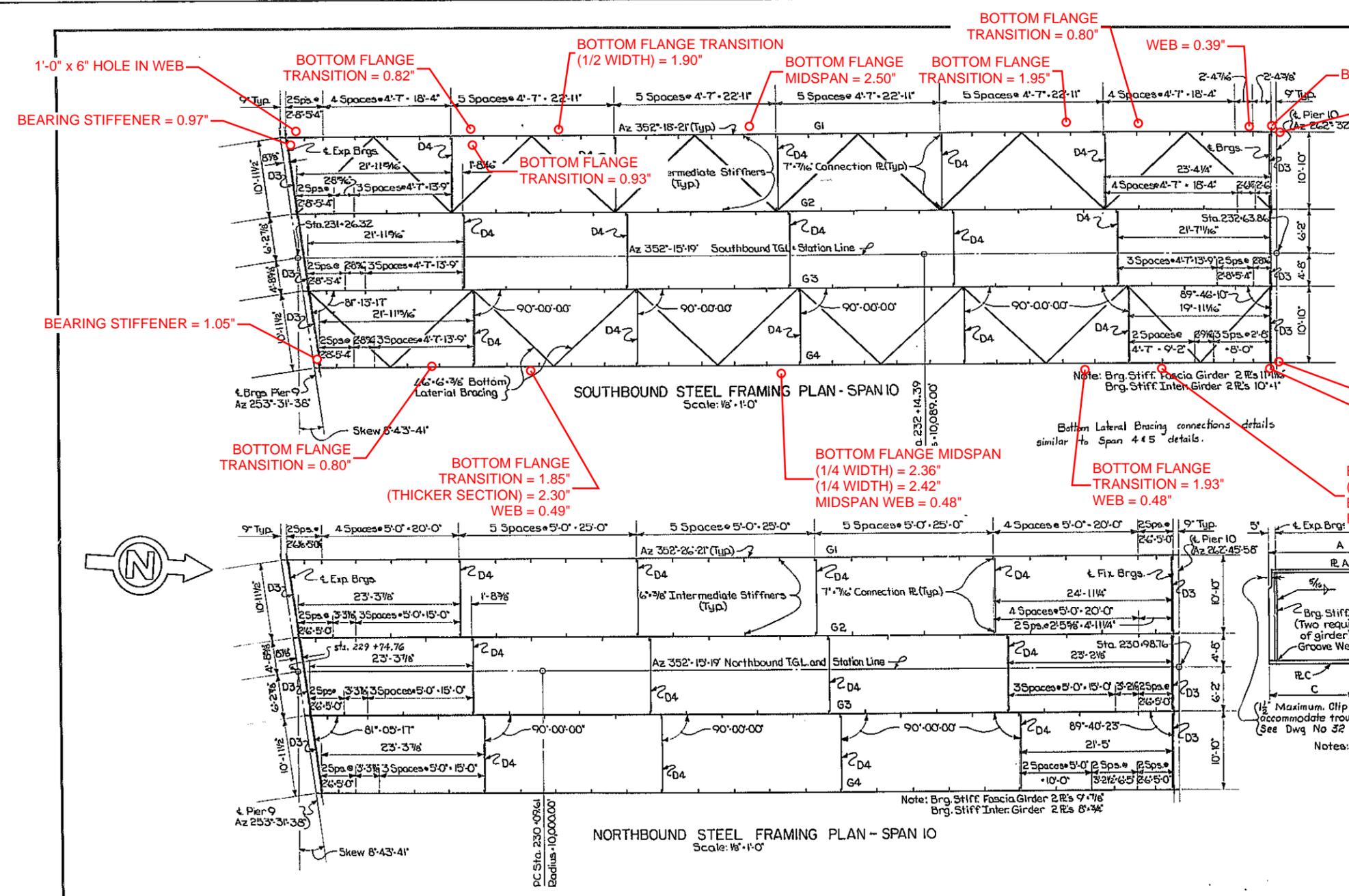
V.C.C.=Vertical Curve Camber
 S.D.L.=Superimposed dead load, includes the weight of sidewalk railing.

BRIDGE NO. 2
 INTERSTATE ROUTE 481 OVER DEWITT YARDS
 SPANS 9
SOUTHBOUND BIN 1093571
 DRAWING NO. 24 OF 50

Bin #2 Detail Yds Steel Frame

FED. RD. REG. NO.	STATE	FEDERAL AID PROJECT NO.	SHEET NO.	TOTAL SHEETS
1	NEW YORK	1-690-3(2B) 1-481-2(116)	204	309

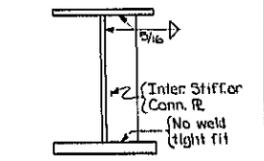
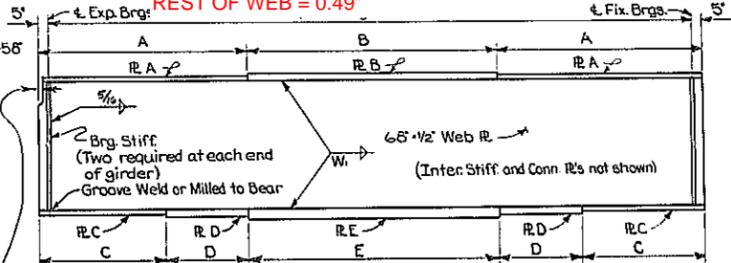
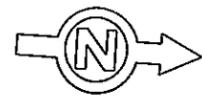
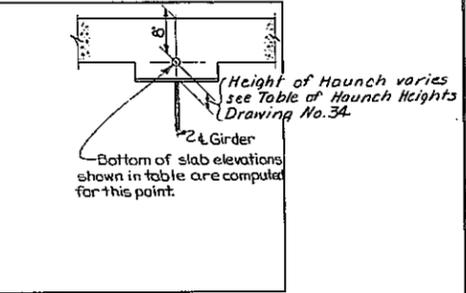
INTERSTATE ROUTE CONNECTION 570
NUTTENUT INTERCHANGE (PHASE 2)
ONONDAGA COUNTY



BOTTOM OF SLAB ELEVATIONS

SOUTHBOUND				
GIRDER	SPAN	EXP BRGS	CL. SPAN	CL. FIX. BRGS
G1	10	443.54	442.46	441.26
G2	10	443.72	442.49	441.15
G3	10	443.70	442.40	440.99
G4	10	443.45	442.16	440.77

NORTHBOUND				
GIRDER	SPAN	EXP BRGS	CL. SPAN	CL. FIX. BRGS
G1	10	443.50	442.49	441.39
G2	10	443.56	442.42	441.19
G3	10	443.43	442.24	440.98
G4	10	443.17	442.00	440.75



SPAN IO GIRDER TABLE

GIRDER	SPAN LENGTH CL. TO CL. BRGS.	FLANGE PLATE SIZES					FLANGE PLATE LENGTHS					DEFLECTIONS (FT)				CAMBER		STUD SHEAR CONNECTOR SPACING	
		R A	R B	R C	R D	R E	A	B	C	D	E	STEEL	CONC.	S. D.L.	TOTAL	V.C.C. (FT)	TOTAL		
Southbound																			
G1	138'-8 3/4"	23'-1"	23'-1 1/2"	24'-1"	24'-2"	24'-2 1/2"	36'-9 3/4"	66'-0"	20'-3 3/4"	19'-0"	61'-0"	0.10	0.15	0.11	0.36	.06	0.42'	5'	138 Spaces • 12' • 138'-0"
G2	137'-0 3/4"	19'-7 3/8"	19'-1 1/2"	24'-1"	None	24'-2"	32'-11 1/4"	72'-0"	23'-5 1/4"	0	91'-0"	0.11	0.28	0.04	0.43	.06	0.49	5 1/2"	137 Spaces • 12' • 137'-0"
G3	135'-3 3/8"	19'-7 3/8"	19'-1 1/2"	24'-1"	None	24'-2"	32'-0 3/4"	72'-0"	22'-6 3/4"	0	91'-0"	0.10	0.27	0.04	0.41	.05	0.46	5 1/2"	135 Spaces • 12' • 135'-0"
G4	133'-7"	23'-1"	23'-1 1/2"	24'-1"	24'-2"	24'-2 1/2"	34'-2 1/2"	66'-0"	17'-8 1/2"	19'-0"	61'-0"	0.09	0.13	0.09	0.31	.05	0.36	4 3/4"	133 Spaces • 12' • 133'-0"
Northbound																			
G1	125'-0 1/4"	17'-3 3/4"	17'-1 1/2"	24'-1"	None	24'-2"	28'-11 1/8"	68'-0"	21'-5 1/2"	0	83'-0"	0.07	0.12	0.09	0.28	.05	0.33	4'	125 Spaces • 12' • 125'-0"
G2	123'-3 1/8"	14'-3 3/4"	14'-1 1/2"	23'-1"	None	23'-1 1/2"	27'-6 3/4"	69'-0"	28'-0 3/4"	0	68'-0"	0.08	0.24	0.03	0.35	.05	0.40	4 3/4"	123 Spaces • 12' • 123'-0"
G3	121'-6"	14'-3 3/4"	14'-1 1/2"	23'-1"	None	23'-1 1/2"	26'-8"	69'-0"	27'-2"	0	68'-0"	0.07	0.23	0.03	0.33	.04	0.37	4 1/2"	121 Spaces • 12' • 121'-0"
G4	119'-8 1/8"	17'-3 3/4"	17'-1 1/2"	24'-1"	None	24'-2"	26'-3 3/4"	68'-0"	18'-9 1/4"	0	83'-0"	0.06	0.10	0.08	0.24	.04	0.28	3 3/4"	119 Spaces • 12' • 119'-0"

S.D.L. = superimposed dead load, includes weight of railing and parapet.
V.C.C. = vertical curve correction

WELD SIZE TABLE

Thickness of Flange	W. Size of fillet weld joining web to flange
1/2" and under	5/16"
over 1/2" to 2 1/4"	3/8"
over 2 1/4"	1/2"

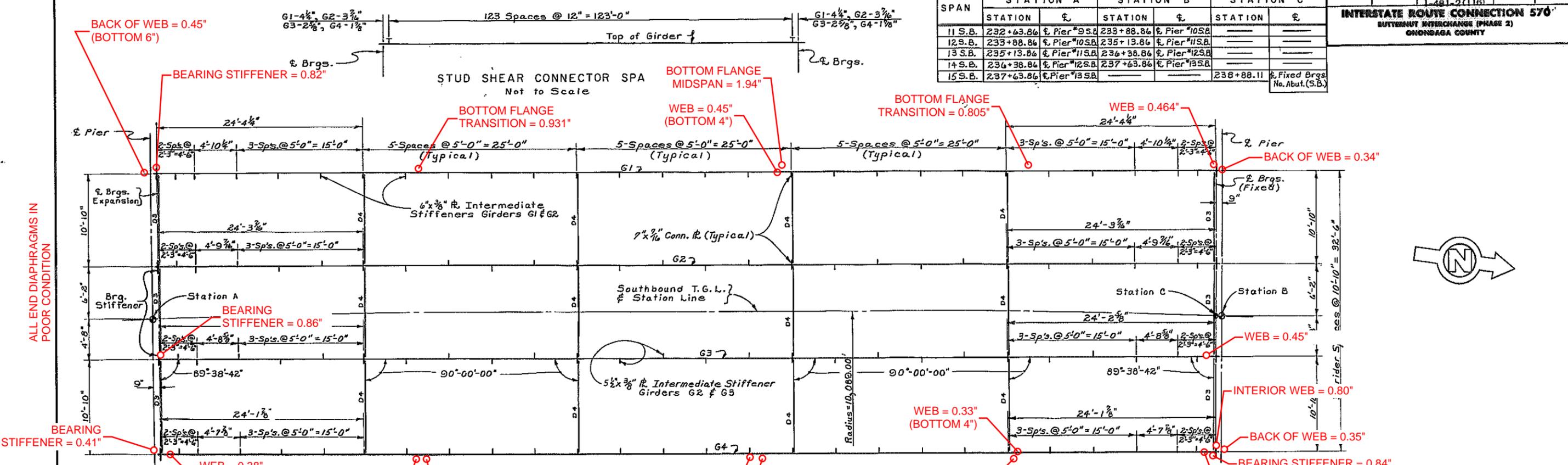
PROJECT ENGINEER R.L. Parker
IN CHARGE OF E. Finkel
DESIGNED BY E. Finkel
DESIGN CHECKED BY J. Darcy
DETAILED BY J.F. Darcy
DETAIL CHECKED BY J.F. Darcy

BRIDGE NO. 2
INTERSTATE ROUTE 481 OVER DEWITT YARDS
SPAN IO
SOUTHBOUND BIN 1093571
DRAWING NO. 26 OF 50

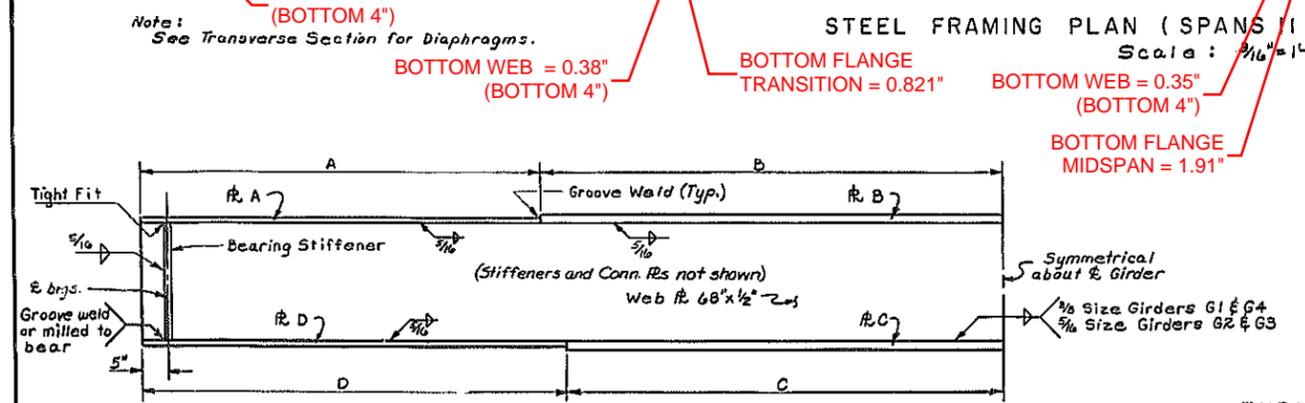
TABLE OF STATIONS						
SPAN	STATION "A"		STATION "B"		STATION "C"	
	STATION	℄	STATION	℄	STATION	℄
11 S.B.	232+63.86	℄ Pier #9 S.B.	233+88.86	℄ Pier #10 S.B.		
12 S.B.	233+88.86	℄ Pier #10 S.B.	235+13.86	℄ Pier #11 S.B.		
13 S.B.	235+13.86	℄ Pier #11 S.B.	236+38.86	℄ Pier #12 S.B.		
14 S.B.	236+38.86	℄ Pier #12 S.B.	237+63.86	℄ Pier #13 S.B.		
15 S.B.	237+63.86	℄ Pier #13 S.B.		238+88.11	℄ Fixed Brgs. No. Abut. (S.B.)	

FED. AID REG. NO.	STATE	FEDERAL AID PROJECT NO.	SHEET NO.	TOTAL SHEETS
1	NEW YORK	1-690-3(2B) 1-481-2(116)	206	309

INTERSTATE ROUTE CONNECTION 570
SUFFERN INTERCHANGE (PHASE 2)
ONDAGA COUNTY



ALL END DIAPHRAGMS IN POOR CONDITION

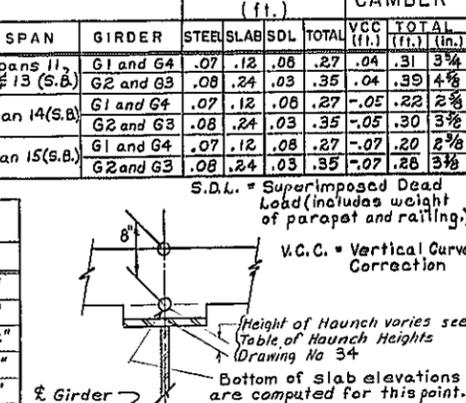


GIRDER	SPAN C. to C. BRGS.	FLANGE SIZES				FLANGE DIMENSIONS				Brg. Stiffener 2-℄s. required each end of Girder	Intermediate Stiffener Size
		RA	RB	RC	RD	A	B	C	D		
G1	123'-8 1/2"	16" x 3/4"	16" x 1 1/2"	23 x 2"	23 x 1"	28'-9 1/4"	33'-6"	40'-6"	21'-9 1/4"	9" x 7/8"	6" x 3/8"
G2	123'-6 1/16"	15" x 3/4"	15" x 1 1/2"	22 x 1 1/2"	22 x 1"	28'-8 3/8"	33'-6"	33'-6"	28'-8 3/8"	8" x 3/4"	5 1/2" x 3/8"
G3	123'-5 5/16"	15" x 3/4"	15" x 1 1/2"	22 x 1 1/2"	22 x 1"	28'-7 7/8"	33'-6"	33'-6"	28'-7 7/8"	8" x 3/4"	5 1/2" x 3/8"
G4	123'-3 1/16"	16" x 3/4"	16" x 1 1/2"	23 x 2"	23 x 1"	28'-6 7/8"	33'-6"	40'-6"	21'-6 7/8"	9" x 7/8"	6" x 3/8"

GIRDER	SPAN	℄ So. Brg.	℄ Span	℄ No. Brg.
G1	11	441.24	440.03	438.56
G2	11	441.12	439.30	438.33
G3	11	440.96	439.58	438.11
G4	11	440.74	439.35	437.88

SPAN	GIRDER	STEEL SLAB			TOTAL	CAMBER	
		S	D	L		(ft.)	(in.)
Spans 11, 12 & 13 (S.B.)	G1 and G4	.07	.12	.06	.27	.04	.31
	G2 and G3	.08	.24	.03	.35	.04	.39
Span 14 (S.B.)	G1 and G4	.07	.12	.08	.27	-.05	.22
	G2 and G3	.08	.24	.03	.35	-.05	.30
Span 15 (S.B.)	G1 and G4	.07	.12	.08	.27	-.07	.20
	G2 and G3	.08	.24	.03	.35	-.07	.28

LOCATION	AZIMUTHS
℄ Pier #10 (S.B.)	262°-32'-11"
℄ Pier #11 (S.B.)	263°-14'-46"
℄ Pier #12 (S.B.)	263°-57'-22"
℄ Pier #13 (S.B.)	264°-39'-57"
℄ Pier #14 (S.B.)	265°-22'-33"



PROJECT ENGINEER R. Parker
IN CHARGE OF V. B. Sherman
DESIGNED BY N. A. Spitzer
DESIGN CHECKED BY S. Rowe
DETAILED BY V. C. Thompson
DETAIL CHECKED BY J. J. Hill

BRIDGE NO. 2
INTERSTATE ROUTE 481 OVER DEWITT YARDS
SPANS 11 SOUTHBOUND BIN 1093571
DRAWING NO. 28 OF 50

SPAN	STATION "A"		STATION "B"		STATION "C"	
	STATION	℄	STATION	℄	STATION	℄
11 S.B.	232+63.86	℄ Pier #9 S.B.	233+88.86	℄ Pier #10 S.B.		
12 S.B.	233+88.86	℄ Pier #10 S.B.	235+13.86	℄ Pier #11 S.B.		
13 S.B.	235+13.86	℄ Pier #11 S.B.	236+38.86	℄ Pier #12 S.B.		
14 S.B.	236+38.86	℄ Pier #12 S.B.	237+63.86	℄ Pier #13 S.B.		
15 S.B.	237+63.86	℄ Pier #13 S.B.		238+88.11	℄ Fixed Brgs. No. Abut. (S.B.)	

FED. AID REG. NO.	STATE	FEDERAL AID PROJECT NO.	SHEET NO.	TOTAL SHEETS
1	NEW YORK	1-690-3(2B) 1-481-2(116)	206	309

INTERSTATE ROUTE CONNECTION 570
SUFFERN INTERCHANGE (PHASE 2)
OHONDAGA COUNTY

BOTTOM WEB = 0.40"
(BOTTOM 4")

BEARING STIFFENER = 0.51"
(BOTTOM 4")

BACK OF WEB = 0.37"

BACK OF WEB = 0.35"
(BOTTOM 4")

BEARING STIFFENER = 0.65"

BACK OF WEB = 0.30"
(BOTTOM 4")

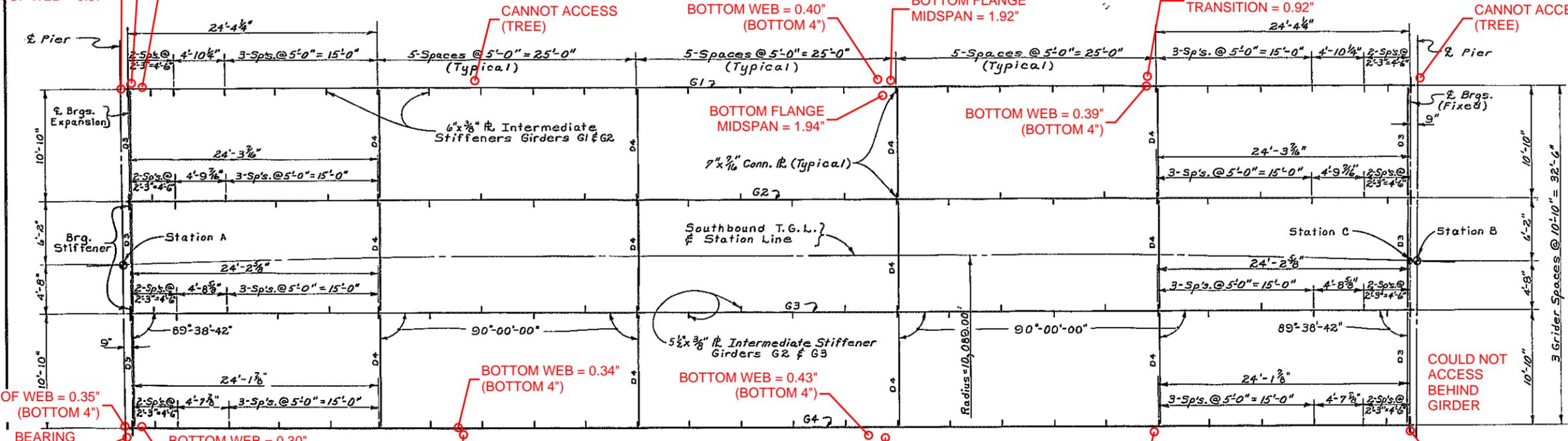
BEARING STIFFENER = 0.850"

123 Spaces @ 12" = 123'-0"

Top of Girder

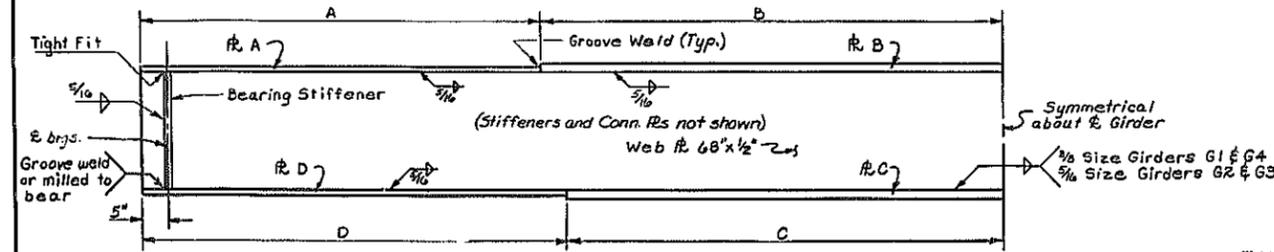
℄ Brgs.

STUD SHEAR CONNECTOR SPACING
Not to Scale



STEEL FRAMING PLAN (SPANS 11, 12, 13, 14 & 15 SOUTHBOUND)
Scale: 3/16" = 1'-0"

Notes:
Web and flanges shall be A.S.T.M. designation A441 Steel.
Stiffeners, connection plates, and diaphragms shall be A.S.T.M. designation A36 Steel.



TYPICAL SECTION
Not to Scale

Note: Where connection plates are used in pairs, the fillet weld at top of stiffener is optional. See Note "Sup 4" in the Super-structure Notes.

GIRDER ELEVATION
Not to Scale

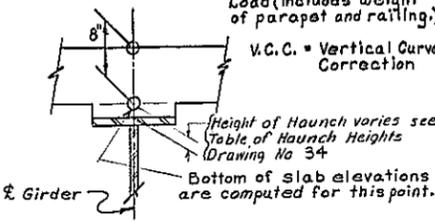
GIRDER	SPAN C. to C. BRGS.	FLANGE SIZES				FLANGE DIMENSIONS				Brg. Stiffener 2-℄s. required each end of Girder	Intermediate Stiffener Size
		RA	RB	RC	RD	A	B	C	D		
G1	123'-8 1/2"	16" x 3/4"	16" x 1 1/2"	23 x 2"	23 x 1"	28'-9 1/4"	33'-6"	40'-6"	21'-9 1/4"	9" x 7/8"	6" x 3/8"
G2	123'-6 1/16"	15" x 3/4"	15" x 1 1/2"	22 x 1 1/2"	22 x 1"	28'-8 3/8"	33'-6"	33'-6"	28'-8 3/8"	8" x 3/4"	5 1/2" x 3/8"
G3	123'-5 5/16"	15" x 3/4"	15" x 1 1/2"	22 x 1 1/2"	22 x 1"	28'-7 7/8"	33'-6"	33'-6"	28'-7 7/8"	8" x 3/4"	5 1/2" x 3/8"
G4	123'-3 1/16"	16" x 3/4"	16" x 1 1/2"	23 x 2"	23 x 1"	28'-6 7/8"	33'-6"	40'-6"	21'-6 7/8"	9" x 7/8"	6" x 3/8"

PROJECT ENGINEER R. Parker
IN CHARGE OF V. B. Sherman
DESIGNED BY N. A. Spitzer
DESIGN CHECKED BY S. Rowe
DETAILED BY V. C. Thompson
DETAIL CHECKED BY J. J. Hill

GIRDER	SPAN	℄ So. Brg.	℄ Span	℄ No. Brg.
G1	11	441.24	440.03	438.56
G2	11	441.12	439.30	438.33
G3	11	440.96	439.58	438.11
G4	11	440.74	439.35	437.88
G1	12	438.52	436.95	435.30
G2	12	438.29	436.73	435.07
G3	12	438.07	436.50	434.85
G4	12	437.84	436.27	434.62
G1	13	435.26	433.51	431.67
G2	13	435.03	433.28	431.45
G3	13	434.81	433.06	431.22
G4	13	434.58	432.83	431.00
G1	14	431.63	429.72	427.89
G2	14	431.40	429.49	427.66
G3	14	431.18	429.26	427.44
G4	14	430.95	429.04	427.21
G1	15	427.85	426.16	424.62
G2	15	427.62	425.93	424.39
G3	15	427.40	425.71	424.16
G4	15	427.17	425.48	423.94

LOCATION	AZIMUTHS
℄ Pier #10 (S.B.)	262°-32'-11"
℄ Pier #11 (S.B.)	263°-14'-46"
℄ Pier #12 (S.B.)	263°-57'-22"
℄ Pier #13 (S.B.)	264°-39'-57"
℄ Pier #14 (S.B.)	265°-22'-33"
℄ Fixed Brgs. @ North Abut. (S.B.)	266°-05'-08"
℄ Girders, Span 11 (S.B.)	352°-53'-28"
℄ Girders, Span 12 (S.B.)	353°-36'-04"
℄ Girders, Span 13 (S.B.)	354°-18'-40"
℄ Girders, Span 14 (S.B.)	355°-01'-15"
℄ Girders, Span 15 (S.B.)	355°-43'-51"

SPAN	GIRDER	DEFLECTIONS (ft.)				CAMBER	
		STEEL	SLAB	SDL	TOTAL	V.C.C. (ft.)	TOTAL (ft.)
Spans 11, 12 & 13 (S.B.)	G1 and G4	.07	.12	.06	.27	.04	.31
	G2 and G3	.08	.24	.03	.35	.04	.39
Span 14 (S.B.)	G1 and G4	.07	.12	.08	.27	-.05	.22
	G2 and G3	.08	.24	.03	.35	-.05	.30
Span 15 (S.B.)	G1 and G4	.07	.12	.08	.27	-.07	.20
	G2 and G3	.08	.24	.03	.35	-.07	.28

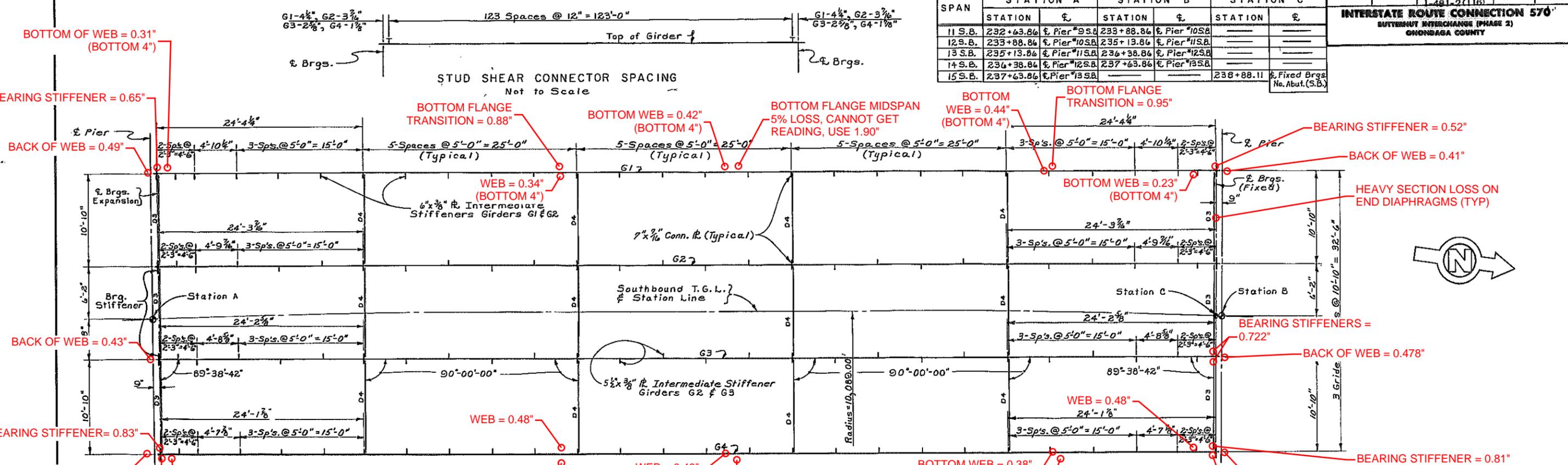


BRIDGE NO. 2
INTERSTATE ROUTE 481 OVER DEMITT YARDS
SPANS 12 SOUTHBOUND BIN 1093571
DRAWING NO. 28 OF 50

TABLE OF STATIONS						
SPAN	STATION "A"		STATION "B"		STATION "C"	
	STATION	℄	STATION	℄	STATION	℄
11 S.B.	232+63.86	℄ Pier #9 S.B.	233+88.86	℄ Pier #10 S.B.		
12 S.B.	233+88.86	℄ Pier #10 S.B.	235+13.86	℄ Pier #11 S.B.		
13 S.B.	235+13.86	℄ Pier #11 S.B.	236+38.86	℄ Pier #12 S.B.		
14 S.B.	236+38.86	℄ Pier #12 S.B.	237+63.86	℄ Pier #13 S.B.		
15 S.B.	237+63.86	℄ Pier #13 S.B.		238+88.11	℄ Fixed Brgs. No. Abut. (S.B.)	

FED. AID REG. NO.	STATE	FEDERAL AID PROJECT NO.	SHEET NO.	TOTAL SHEETS
	NEW YORK	1-690-3(2B) 1-481-2(116)	206	309

INTERSTATE ROUTE CONNECTION 570
SUFFERN INTERCHANGE (PHASE 2)
OHONDAGA COUNTY



BOTTOM OF WEB = 0.31" (BOTTOM 4")

BEARING STIFFENER = 0.65"

BACK OF WEB = 0.49"

BACK OF WEB = 0.43"

BEARING STIFFENER = 0.83"

BACK OF WEB = 0.45"

BEARING STIFFENER = 0.83"

BEARING STIFFENER = 0.52"

BACK OF WEB = 0.41"

HEAVY SECTION LOSS ON END DIAPHRAGMS (TYP)

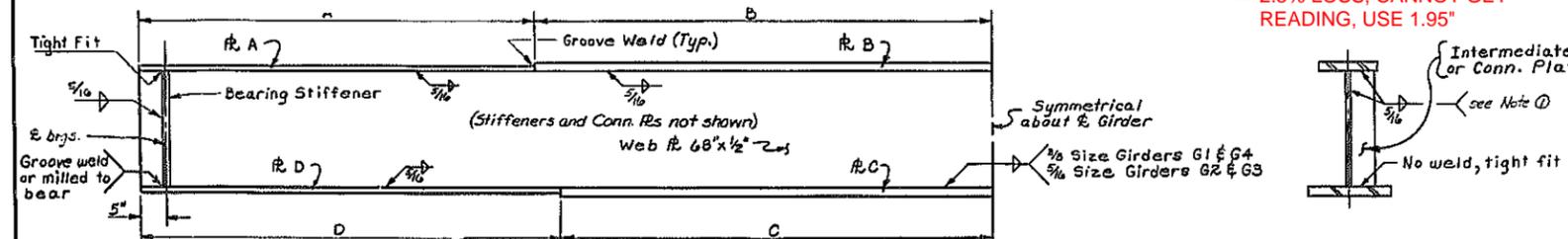
BEARING STIFFENERS = 0.722"

BACK OF WEB = 0.478"

BEARING STIFFENER = 0.81"

BACK OF WEB = 0.46"

BEARING STIFFENER = 0.73"



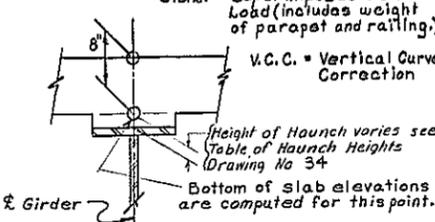
GIRDER ELEVATION
Not to Scale

SOUTHBOUND GIRDER TABLE - SPANS 11 thru 15											
GIRDER	SPAN C. to C. BRGS.	FLANGE SIZES				FLANGE DIMENSIONS				Brg. Stiffener 2-℄s. required each end of Girder	Intermediate Stiffener Size
		RA	RB	RC	RD	A	B	C	D		
G1	123'-8 1/2"	16" x 3/4"	16" x 1 1/2"	23" x 2"	23" x 1"	28'-9 1/4"	33'-6"	40'-6"	21'-9 1/4"	9" x 7/8"	6" x 3/8"
G2	123'-6 15/16"	15" x 3/4"	15" x 1 1/2"	22" x 1 1/2"	22" x 1"	28'-8 3/8"	33'-6"	33'-6"	28'-8 3/8"	8" x 3/4"	5 1/2" x 3/8"
G3	123'-5 5/16"	15" x 3/4"	15" x 1 1/2"	22" x 1 1/2"	22" x 1"	28'-7 7/8"	33'-6"	33'-6"	28'-7 7/8"	8" x 3/4"	5 1/2" x 3/8"
G4	123'-3 1/16"	16" x 3/4"	16" x 1 1/2"	23" x 2"	23" x 1"	28'-6 7/8"	33'-6"	40'-6"	21'-6 7/8"	9" x 7/8"	6" x 3/8"

THEO. BOTTOM OF SLAB ELEV. (Southbound)				
GIRDER	SPAN	℄ So. Brg.	℄ Span	℄ No. Brg.
G1	11	441.24	440.03	438.56
G2	11	441.12	439.30	438.33
G3	11	440.96	439.58	438.11
G4	11	440.74	439.35	437.88
G1	12	438.52	436.95	435.30
G2	12	438.29	436.73	435.07
G3	12	438.07	436.50	434.85
G4	12	437.84	436.27	434.62
G1	13	435.26	433.51	431.67
G2	13	435.03	433.28	431.45
G3	13	434.81	433.06	431.22
G4	13	434.58	432.83	431.00
G1	14	431.63	429.72	427.89
G2	14	431.40	429.49	427.66
G3	14	431.18	429.26	427.44
G4	14	430.95	429.04	427.21
G1	15	427.85	426.16	424.62
G2	15	427.62	425.93	424.39
G3	15	427.40	425.71	424.16
G4	15	427.17	425.48	423.94

TABLE OF AZIMUTHS	
LOCATION	AZIMUTHS
℄ Pier #10 (S.B.)	262°-32'-11"
℄ Pier #11 (S.B.)	263°-14'-46"
℄ Pier #12 (S.B.)	263°-57'-22"
℄ Pier #13 (S.B.)	264°-39'-57"
℄ Pier #14 (S.B.)	265°-22'-33"
℄ Fixed Brgs. @ North Abut. (S.B.)	266°-05'-08"
℄ Girders, Span 11 (S.B.)	352°-53'-28"
℄ Girders, Span 12 (S.B.)	353°-36'-04"
℄ Girders, Span 13 (S.B.)	354°-18'-40"
℄ Girders, Span 14 (S.B.)	355°-01'-15"
℄ Girders, Span 15 (S.B.)	355°-43'-51"

SPAN	GIRDER	DEFLECTIONS (ft.)				CAMBER	
		STEEL	SLAB	SDL	TOTAL	V.C.C. (ft.)	TOTAL (ft.) (in.)
Spans 11, 12 & 13 (S.B.)	G1 and G4	.07	.12	.06	.27	.04	.31 3/4
	G2 and G3	.08	.24	.03	.35	.04	.39 1/2
Span 14 (S.B.)	G1 and G4	.07	.12	.06	.27	-.05	.22 1/2
	G2 and G3	.08	.24	.03	.35	-.05	.30 1/2
Span 15 (S.B.)	G1 and G4	.07	.12	.06	.27	-.07	.20 1/2
	G2 and G3	.08	.24	.03	.35	-.07	.28 1/2



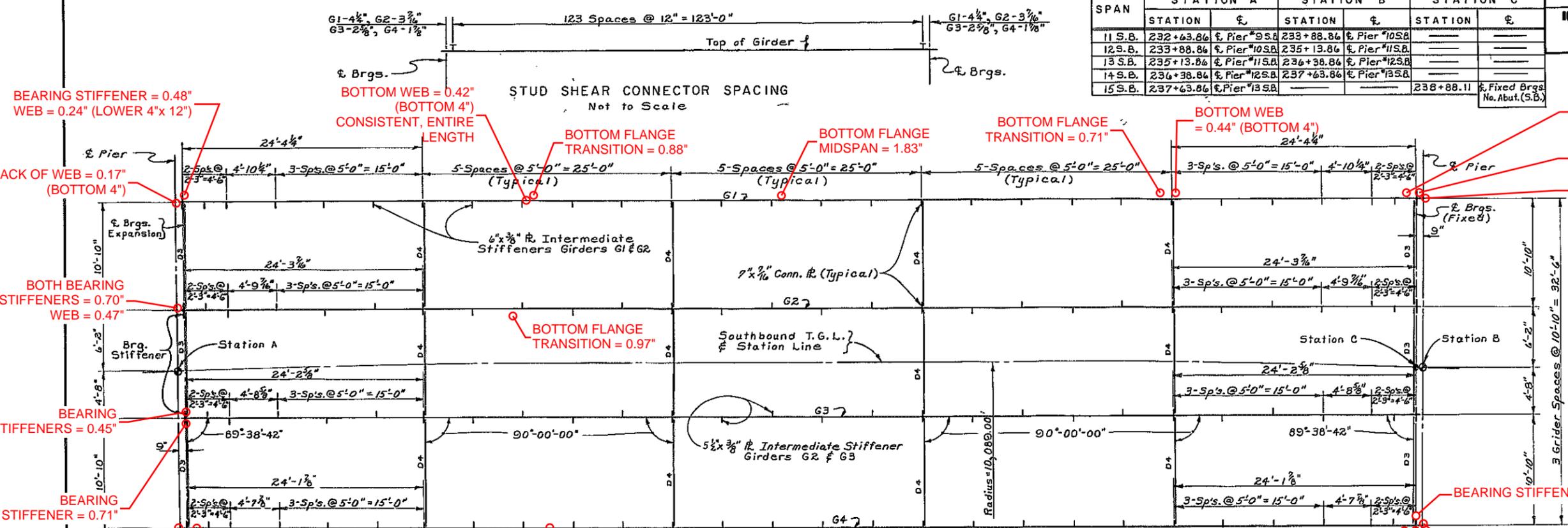
PROJECT ENGINEER R. Parker
IN CHARGE OF V. B. Sherman
DESIGNED BY N. A. Toppase
DESIGN CHECKED BY S. Rowe
DETAILED BY V. C. Thompson
DETAIL CHECKED BY J. J. Hill

BRIDGE NO. 2
INTERSTATE ROUTE 481 OVER DEWITT YARDS
SPANS 13 SOUTHBOUND
BIN 1093571
DRAWING NO. 28 OF 50

SPAN	STATION "A"		STATION "B"		STATION "C"	
	STATION	℄	STATION	℄	STATION	℄
11 S.B.	232+63.86	℄ Pier #9 S.B.	233+88.86	℄ Pier #10 S.B.		
12 S.B.	233+88.86	℄ Pier #10 S.B.	235+13.86	℄ Pier #11 S.B.		
13 S.B.	235+13.86	℄ Pier #11 S.B.	236+38.86	℄ Pier #12 S.B.		
14 S.B.	236+38.86	℄ Pier #12 S.B.	237+63.86	℄ Pier #13 S.B.		
15 S.B.	237+63.86	℄ Pier #13 S.B.		238+88.11	℄ Fixed Brgs. No. Abut. (S.B.)	

FED. AID PROJ. NO.	STATE	FEDERAL AID PROJECT NO.	SHEET NO.	TOTAL SHEETS
1-481-2(116)	NEW YORK	1-690-3(28)	206	309

INTERSTATE ROUTE CONNECTION 570
SUFFERN INTERCHANGE (PHASE 2)
OHONDAAGA COUNTY



BEARING STIFFENER = 0.48"
WEB = 0.24" (LOWER 4"x 12")

BOTTOM WEB = 0.42" (BOTTOM 4")
CONSISTENT, ENTIRE LENGTH

BOTTOM FLANGE TRANSITION = 0.88"

BOTTOM FLANGE MIDSPAN = 1.83"

BOTTOM FLANGE TRANSITION = 0.71"

BOTTOM WEB = 0.44" (BOTTOM 4")

BOTTOM WEB = 0.33" (BOTTOM 4")

BEARING STIFFENER = 0.54"

BACK OF WEB = 0.46"

BOTH BEARING STIFFENERS = 0.70"
WEB = 0.47"

BEARING STIFFENERS = 0.45"

BEARING STIFFENER = 0.71"

WEB = 0.28" (BOTTOM 4")

WEB = 0.27" (BOTTOM 4"x 12" LONG)

WEB = 0.40" (BOTTOM 4")

BOTTOM FLANGE TRANSITION = 0.87"

BOTTOM FLANGE MIDSPAN = 1.667"

BOTTOM FLANGE TRANSITION = 0.872"

BEARING STIFFENER = 0.82"

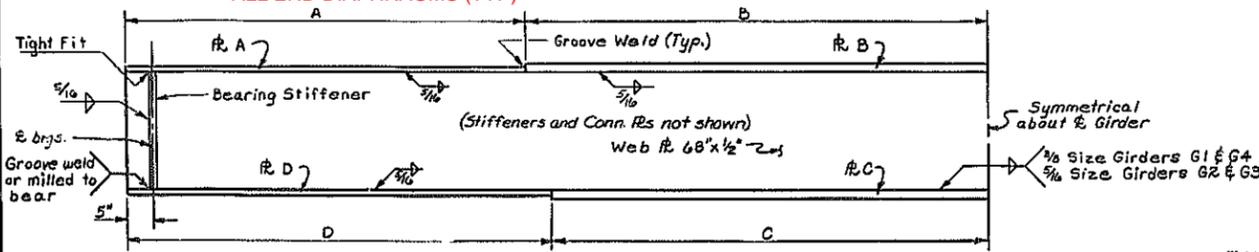
BEARING STIFFENER = 0.24"

HEAVY SECTION LOSS ON ALL END DIAPHRAGMS (TYP)

BEARING STIFFENER = 0.81"

STEEL FRAMING PLAN (SPANS 11, 12, 13, 14 & 15 SOUTHBOUND)
Scale: 3/16" = 1'-0"

Notes:
Web and flanges shall be A.S.T.M. designation A441 Steel.
Stiffeners, connection plates, and diaphragms shall be A.S.T.M. designation A36 Steel.



TYPICAL SECTION
Not to Scale

Note D: Where connection plates are used in pairs, the fillet weld at top of stiffener is optional. See Note "Sup 4" in the Super-structure Notes.

GIRDER ELEVATION
Not to Scale

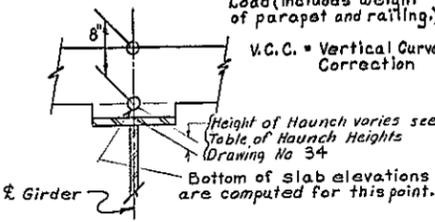
GIRDER	SPAN C. to C. BRGS.	FLANGE SIZES				FLANGE DIMENSIONS				Brg. Stiffener 2 ℄s. required each end of Girder	Intermediate Stiffener Size
		RA	RB	RC	RD	A	B	C	D		
G1	123'-8 1/2"	16" x 3/4"	16" x 1 1/2"	23 x 2"	23 x 1"	28'-9 1/4"	33'-6"	40'-6"	21'-9 1/4"	9" x 7/8"	6" x 3/8"
G2	123'-6 15/16"	15" x 3/4"	13" x 1 1/2"	22 x 1 1/2"	22 x 1"	28'-8 3/8"	33'-6"	33'-6"	28'-8 3/8"	8" x 3/4"	5 1/2" x 3/8"
G3	123'-5 5/16"	15" x 3/4"	13" x 1 1/2"	22 x 1 1/2"	22 x 1"	28'-7 7/8"	33'-6"	33'-6"	28'-7 7/8"	8" x 3/4"	5 1/2" x 3/8"
G4	123'-3 1/16"	16" x 3/4"	16" x 1 1/2"	23 x 2"	23 x 1"	28'-6 7/8"	33'-6"	40'-6"	21'-6 7/8"	9" x 7/8"	6" x 3/8"

PROJECT ENGINEER R. Parker
IN CHARGE OF V. B. Sherman
DESIGNED BY N. A. Toppase
DESIGN CHECKED BY S. Rowe
DETAILED BY V. C. Thompson
DETAIL CHECKED BY J. J. Hill

GIRDER	SPAN	℄ So. Brg.	℄ Span	℄ No. Brg.
G1	11	441.24	440.03	438.56
G2	11	441.12	439.30	438.33
G3	11	440.96	439.58	438.11
G4	11	440.74	439.35	437.88
G1	12	438.52	436.95	435.30
G2	12	438.29	436.73	435.07
G3	12	438.07	436.50	434.85
G4	12	437.84	436.27	434.62
G1	13	435.26	433.51	431.67
G2	13	435.03	433.28	431.45
G3	13	434.81	433.06	431.22
G4	13	434.58	432.83	431.00
G1	14	431.63	429.72	427.89
G2	14	431.40	429.49	427.66
G3	14	431.18	429.26	427.44
G4	14	430.95	429.04	427.21
G1	15	427.85	426.16	424.62
G2	15	427.62	425.93	424.39
G3	15	427.40	425.71	424.16
G4	15	427.17	425.48	423.94

LOCATION	AZIMUTHS
℄ Pier #10 (S.B.)	262°-32'-11"
℄ Pier #11 (S.B.)	263°-14'-46"
℄ Pier #12 (S.B.)	263°-57'-22"
℄ Pier #13 (S.B.)	264°-39'-57"
℄ Pier #14 (S.B.)	265°-22'-33"
℄ Fixed Brgs. @ North Abut. (S.B.)	266°-05'-08"
℄ Girders, Span 11 (S.B.)	352°-53'-28"
℄ Girders, Span 12 (S.B.)	353°-36'-04"
℄ Girders, Span 13 (S.B.)	354°-18'-40"
℄ Girders, Span 14 (S.B.)	355°-01'-15"
℄ Girders, Span 15 (S.B.)	355°-43'-51"

SPAN	GIRDER	DEFLECTIONS (ft.)				CAMBER	
		STEEL	SLAB	SDL	TOTAL	V.C.C. (ft.)	TOTAL (ft.) (in.)
Spans 11, 12 & 13 (S.B.)	G1 and G4	.07	.12	.06	.27	.04	.31 3/4
	G2 and G3	.08	.24	.03	.35	.04	.39 1/2
Span 14 (S.B.)	G1 and G4	.07	.12	.08	.27	-.05	.22 1/2
	G2 and G3	.08	.24	.03	.35	-.05	.30 1/2
Span 15 (S.B.)	G1 and G4	.07	.12	.08	.27	-.07	.20 1/2
	G2 and G3	.08	.24	.03	.35	-.07	.28 1/2

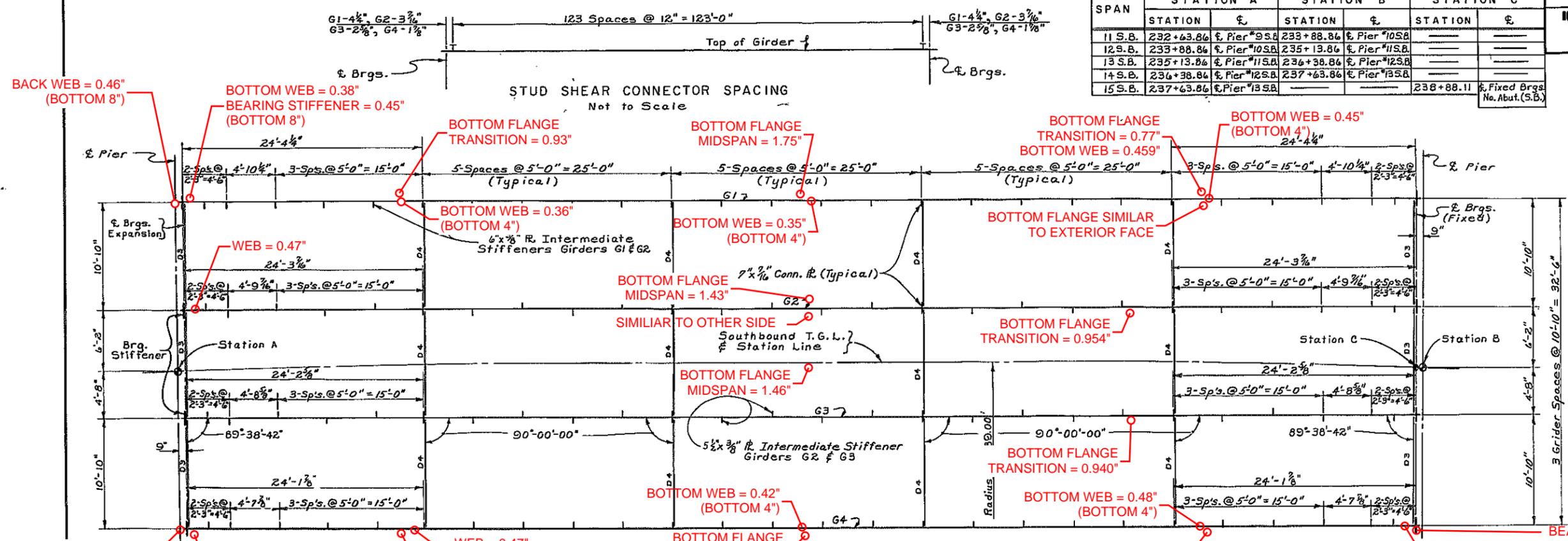


BRIDGE NO. 2
INTERSTATE ROUTE 481 OVER DEWITT YARDS
SPANS 14 SOUTHBOUND BIN 1093571
DRAWING NO. 28 OF 50

TABLE OF STATIONS						
SPAN	STATION "A"		STATION "B"		STATION "C"	
	STATION	℄	STATION	℄	STATION	℄
11 S.B.	232+63.86	℄ Pier #9 S.B.	233+88.86	℄ Pier #10 S.B.		
12 S.B.	233+88.86	℄ Pier #10 S.B.	235+13.86	℄ Pier #11 S.B.		
13 S.B.	235+13.86	℄ Pier #11 S.B.	236+38.86	℄ Pier #12 S.B.		
14 S.B.	236+38.86	℄ Pier #12 S.B.	237+63.86	℄ Pier #13 S.B.		
15 S.B.	237+63.86	℄ Pier #13 S.B.		238+88.11	℄ Fixed Brgs. No. Abut. (S.B.)	

FED. AID REG. NO.	STATE	FEDERAL AID PROJECT NO.	SHEET NO.	TOTAL SHEETS
1	NEW YORK	1-690-3(2B) 1-481-2(116)	206	309

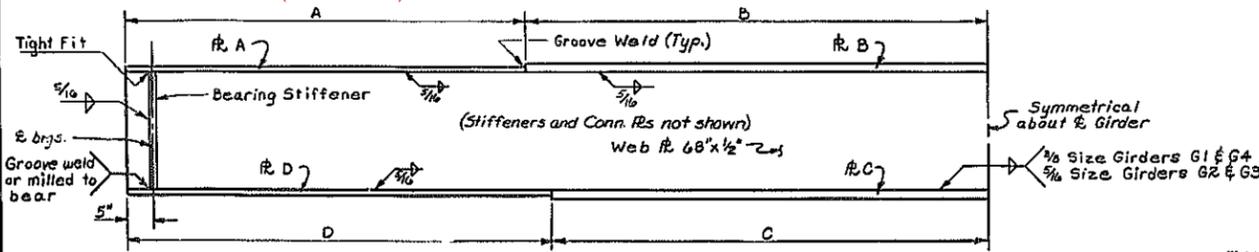
INTERSTATE ROUTE CONNECTION 570
SUFFERN INTERCHANGE (PHASE 2)
OHONDAGA COUNTY



STEEL FRAMING PLAN (SPANS 11, 12, 13, 14 & 15 SOUTHBOUND)
Scale : 3/16" = 1'-0"

Notes:
Web and flanges shall be A.S.T.M. designation A441 Steel.
Stiffeners, connection plates, and diaphragms shall be A.S.T.M. designation A36 Steel.

BACK WEB = 0.46" (BOTTOM 8")
BOTTOM WEB = 0.38" BEARING STIFFENER = 0.45" (BOTTOM 8")
BOTTOM FLANGE TRANSITION = 0.93"
BOTTOM FLANGE MIDSPAN = 1.75"
BOTTOM FLANGE TRANSITION = 0.77" BOTTOM WEB = 0.459"
BOTTOM WEB = 0.45" (BOTTOM 4")
WEB = 0.47"
BOTTOM WEB = 0.36" (BOTTOM 4")
BOTTOM WEB = 0.35" (BOTTOM 4")
BOTTOM FLANGE SIMILAR TO EXTERIOR FACE
BOTTOM FLANGE TRANSITION = 0.954"
SIMILAR TO OTHER SIDE Southbound T.G.L. Station Line
BOTTOM FLANGE MIDSPAN = 1.46"
BOTTOM FLANGE TRANSITION = 0.940"
BOTTOM WEB = 0.42" (BOTTOM 4")
BOTTOM WEB = 0.48" (BOTTOM 4")
BACK OF WEB = 0.358" (BOTTOM 4")
WEB = 0.47" (BOTTOM 4")
BOTTOM FLANGE TRANSITION = 0.87"
BOTTOM WEB = 0.34" BEARING STIFFENER = 0.69" (BOTTOM 4")
BEARING STIFFENER = 0.70"
BOTTOM WEB = 0.400" (BACK OF WEB IS SIMILAR)



TYPICAL SECTION
Not to Scale

Note D: Where connection plates are used in pairs, the fillet weld at top of stiffener is optional. See Note "Sup 4" in the Super-structure Notes.

GIRDER ELEVATION
Not to Scale

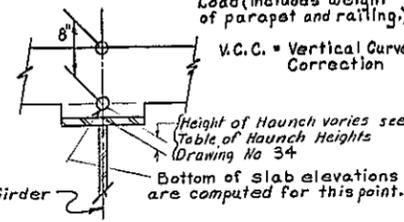
SOUTHBOUND GIRDER TABLE - SPANS 11 thru 15											
GIRDER	SPAN C. to C. BRGS.	FLANGE SIZES				FLANGE DIMENSIONS				Brg. Stiffener 2 ℄s. required each end of Girder	Intermediate Stiffener Size
		RA	RB	RC	RD	A	B	C	D		
G1	123'-8 1/2"	16" x 3/4"	16" x 1 1/2"	23" x 2"	23" x 1"	28'-9 1/4"	33'-6"	40'-6"	21'-9 1/4"	9" x 7/8"	6" x 3/8"
G2	123'-6 15/16"	15" x 3/4"	13" x 1 1/2"	22" x 1 1/2"	22" x 1"	28'-8 3/8"	33'-6"	33'-6"	28'-8 3/8"	8" x 3/4"	5 1/2" x 3/8"
G3	123'-5 5/16"	15" x 3/4"	13" x 1 1/2"	22" x 1 1/2"	22" x 1"	28'-7 7/8"	33'-6"	33'-6"	28'-7 7/8"	8" x 3/4"	5 1/2" x 3/8"
G4	123'-3 1/16"	16" x 3/4"	16" x 1 1/2"	23" x 2"	23" x 1"	28'-6 7/8"	33'-6"	40'-6"	21'-6 7/8"	9" x 7/8"	6" x 3/8"

THEO. BOTTOM OF SLAB ELEV. (Southbound)				
GIRDER	SPAN	℄ So. Brg.	℄ Span	℄ No. Brg.
G1	11	441.24	440.03	438.56
G2	11	441.12	439.30	438.33
G3	11	440.96	439.58	438.11
G4	11	440.74	439.35	437.88

TABLE OF AZIMUTHS

LOCATION	AZIMUTHS
℄ Pier #10 (S.B.)	262°-32'-11"
℄ Pier #11 (S.B.)	263°-14'-46"
℄ Pier #12 (S.B.)	263°-57'-22"
℄ Pier #13 (S.B.)	264°-39'-57"
℄ Pier #14 (S.B.)	265°-22'-33"
℄ Fixed Brgs. @ North Abut. (S.B.)	266°-05'-08"
℄ Girders, Span 11 (S.B.)	352°-53'-28"
℄ Girders, Span 12 (S.B.)	353°-36'-04"
℄ Girders, Span 13 (S.B.)	354°-18'-40"
℄ Girders, Span 14 (S.B.)	355°-01'-15"
℄ Girders, Span 15 (S.B.)	355°-43'-51"

SPAN	GIRDER	DEFLECTIONS (ft.)				CAMBER	
		STEEL	SLAB	S.D.L.	TOTAL	V.C.C.	TOTAL
Spans 11, 12 & 13 (S.B.)	G1 and G4	.07	.12	.06	.27	.04	.31
	G2 and G3	.08	.24	.03	.35	.04	.39
Span 14 (S.B.)	G1 and G4	.07	.12	.08	.27	-.05	.22
	G2 and G3	.08	.24	.03	.35	-.05	.30
Span 15 (S.B.)	G1 and G4	.07	.12	.08	.27	-.07	.20
	G2 and G3	.08	.24	.03	.35	-.07	.28



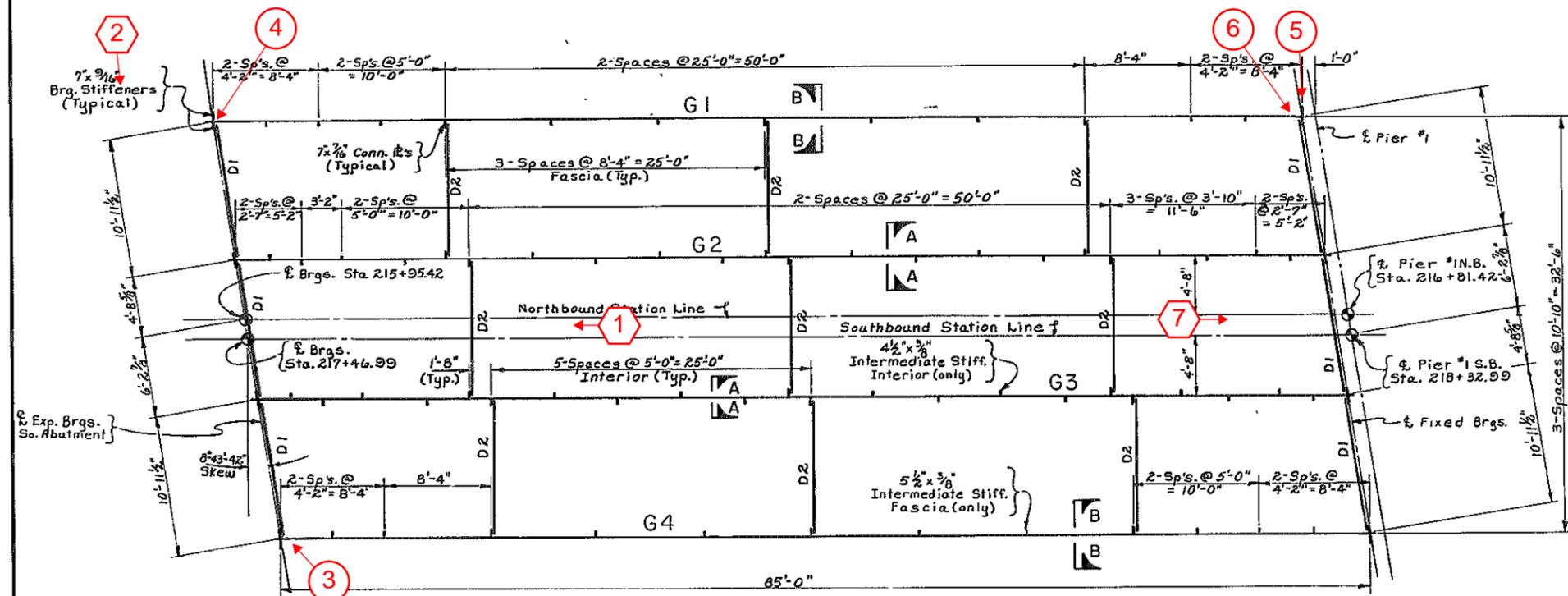
PROJECT ENGINEER R. Parker
IN CHARGE OF V. B. Sherman
DESIGNED BY N. A. Spitzer
DESIGN CHECKED BY S. Rowe
DETAILED BY V. C. Thompson
DETAIL CHECKED BY J. J. Hill

BRIDGE NO. 2
INTERSTATE ROUTE 481 OVER DEWITT YARDS
SPANS 15 SOUTHBOUND BIN 1093571
DRAWING NO. 28 OF 50

IN-DEPTH PHOTO DOCUMENTATION

FED. RD. REG. NO.	STATE	FEDERAL AID PROJECT NO.	SHEET NO.	TOTAL SHEETS
1	NEW YORK	1-690-3(2B) 1-481-2(116)	196	309

INTERSTATE ROUTE CONNECTION 570
 BUTTERNUT INTERCHANGE (PHASE 2)
 ONONDAGA COUNTY



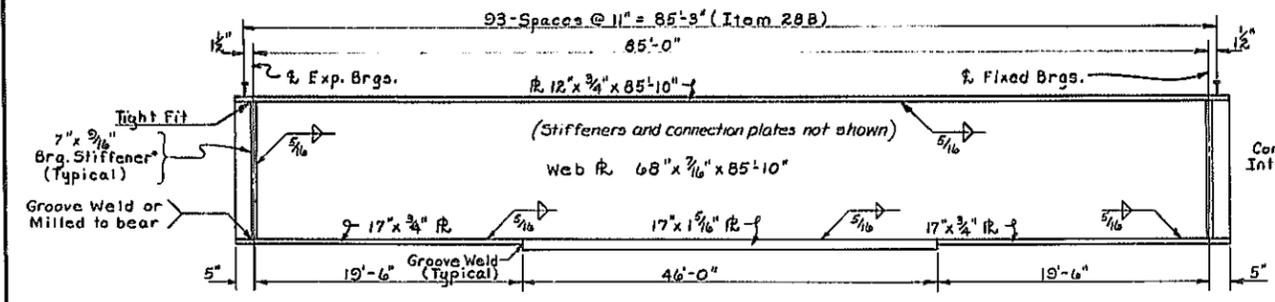
see Transverse Section for Diaphragms. **STEEL LAYOUT (S.B. & N.B.)**
 Scale: 3/16" = 1'-0"



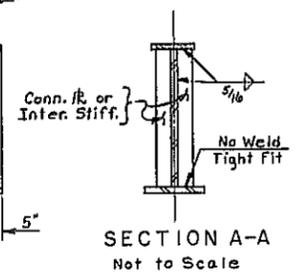
- LEGEND**
- # SUPERSTRUCTURE PHOTOS
 - # SUBSTRUCTURE PHOTOS

Notes:

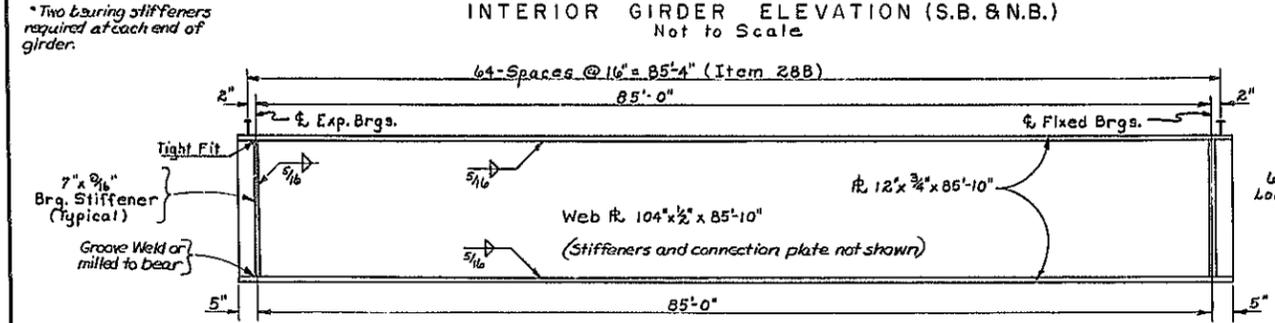
- Web and flanges shall be ASTM designation A36 steel.
- Stiffeners, Connection Plates, Diaphragm and Gasket Plates shall be ASTM designation A36 steel.
- For Bearing Details see Drawing Nos. 42 & 43.
- Stud Shear Connectors shall be 6" high and placed in pairs. See Drawing No. 42 for additional details.
- For Height of Haunch see Table of Haunch Heights Drawing No. 34.



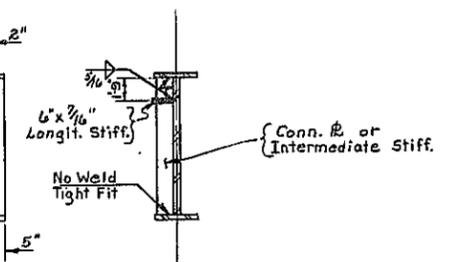
INTERIOR GIRDER ELEVATION (S.B. & N.B.)
 Not to Scale



SECTION A-A
 Not to Scale

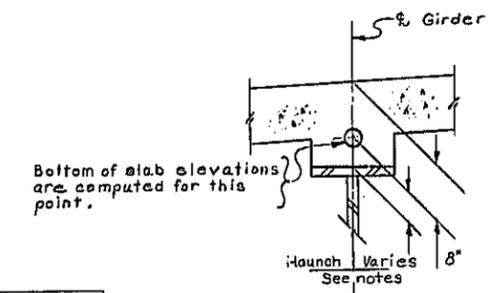


FASCIA GIRDER ELEVATION (S.B. & N.B.)
 Not to Scale



SECTION B-B
 Not to Scale

GIRDER	SPAN	THEO. BOTTOM OF SLAB ELEVATIONS			DEFLECTIONS (FT.)				CAMBER	
		% Exp. Brgs.	% Span	% Fix. Brgs.	STEEL	SLAB	SDL	TOTAL	(ft.)	(in.)
G1 N.B.	1	446.55	447.12	447.65	.01	.03	.01	.05	.02	.07
G2 N.B.	1	446.80	447.40	447.90	.02	.08	.02	.12	.02	.14
G3 N.B.	1	446.79	447.96	447.89	.02	.08	.02	.12	.02	.14
G4 N.B.	1	446.59	447.16	447.65	.01	.03	.01	.05	.02	.07
G1 S.B.	1	446.34	446.92	447.46	.01	.03	.01	.05	.02	.07
G2 S.B.	1	446.59	447.17	447.71	.02	.08	.02	.12	.02	.14
G3 S.B.	1	446.64	447.22	447.76	.02	.08	.02	.12	.02	.14
G4 S.B.	1	446.44	447.02	447.55	.01	.03	.01	.05	.02	.07



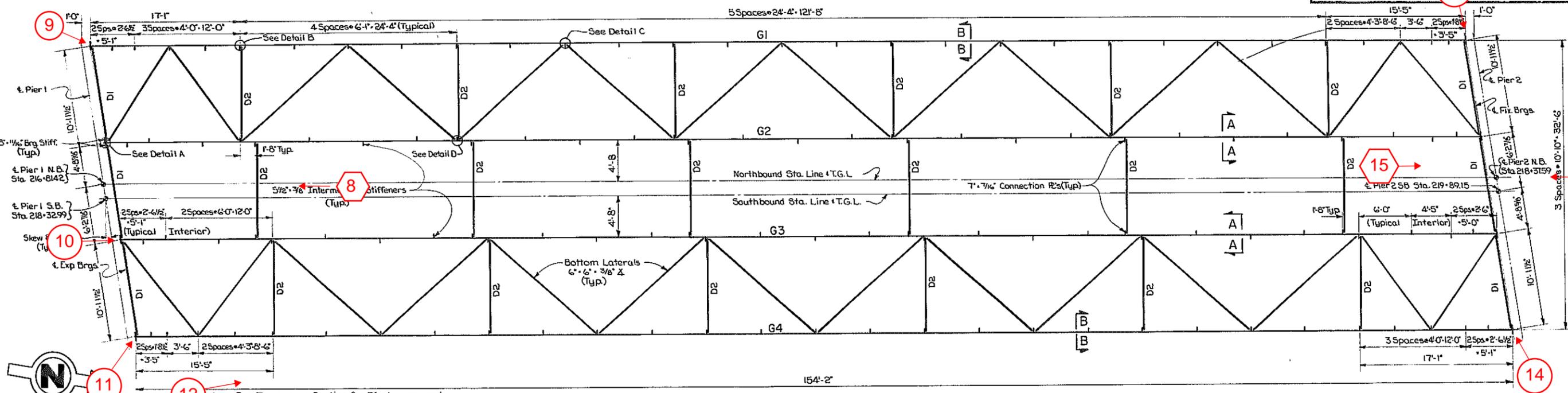
V.C.C. = Vertical Curve Correction
 S.D.L. = Superimposed Dead Load, includes weight of railing.

PROJECT ENGINEER R. Parker
 IN CHARGE OF E. E. K. E. J.
 DESIGNED BY N. TOPPES
 DESIGN CHECKED BY R. THIMBLE
 DETAILED BY J. C. THOMPSON
 DETAIL CHECKED BY H. A. THOMPSON

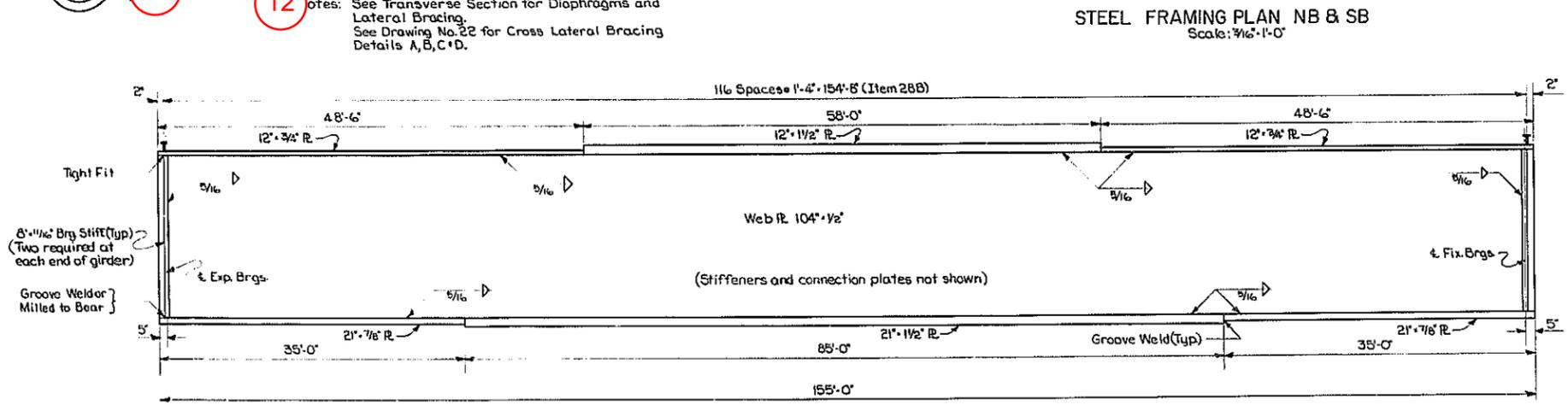
BRIDGE NO. 2
 INTERSTATE ROUTE 481 OVER DEWITT YARDS
 SPAN 1 SOUTHBOUND
 BIN 1093571
PHOTO LOCATION PLAN

FED. RD. REG. NO.	STATE	FEDERAL AID PROJECT NO.	SHEET NO.	TOTAL SHEETS
1	NEW YORK	1-800-3(28) 1-481-2(116)	198	309

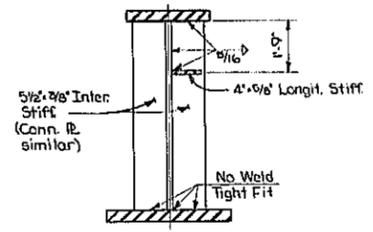
INTERSTATE ROUTE CONNECTION 570
BUTTERNUT INTERCHANGE
ORONDA AGONY



STEEL FRAMING PLAN NB & SB
Scale: 3/16" = 1'-0"

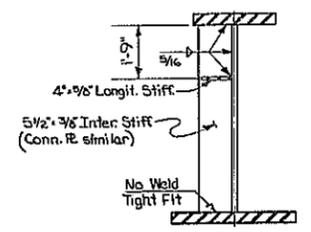


FASCIA & INTERIOR GIRDER ELEVATION NB & SB
Not to Scale



SECTION A-A
Not to Scale

Notes:
Web and flanges shall be A.S.T.M designation A441 Steel.
Stiffeners, connection plates, diaphragms, lateral bracing and gusset plates shall be A.S.T.M. designation A36 Steel.
For details of bearings see Dwg. No. 42143.
Stud Shear Connectors shall be 6" high.



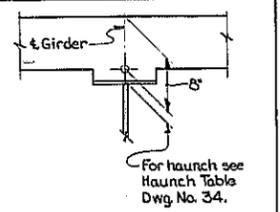
SECTION B-B
Not to Scale

LEGEND

- # SUPERSTRUCTURE PHOTOS
- # SUBSTRUCTURE PHOTOS

GIRDER	SPAN	THEO. BOTTOM OF SLAB ELEVATIONS			DEFLECTIONS (FT.)			CAMBER			
		EXP. BRGS.	SPAN	FIX. BRGS.	STEEL	SLAB	SDL	TOT.	(FT.)	(IN.)	
G1 NB	2	447.67	448.52	449.23	.08	.16	.07	.31	.07	.38	4 7/8
G2 NB	2	447.92	448.76	449.47	.08	.24	.07	.39	.07	.46	5 1/2
G3 NB	2	447.91	448.75	449.45	.08	.24	.07	.39	.07	.46	5 1/2
G4 NB	2	447.70	448.54	449.24	.08	.16	.07	.31	.07	.38	4 7/8
G1 SB	2	447.48	448.35	449.08	.08	.16	.07	.31	.07	.38	4 7/8
G2 SB	2	447.73	448.60	449.32	.08	.24	.07	.39	.07	.46	5 1/2
G3 SB	2	447.78	448.65	449.37	.08	.24	.07	.39	.07	.46	5 1/2
G4 SB	2	447.58	448.44	449.16	.08	.16	.07	.31	.07	.38	4 7/8

V.C.C. - Vertical Curve Correction
S.D.L. - Superimposed Dead Load, includes weight of railing.



PROJECT ENGINEER: R.L. Parker
IN CHARGE OF: F. Eckel
DESIGNED BY: N.A. Toppsset
DESIGN CHECKED BY: R. Thimble
DETAILED BY: J.F. Darcy
DETAIL CHECKED BY: R.L. Parker

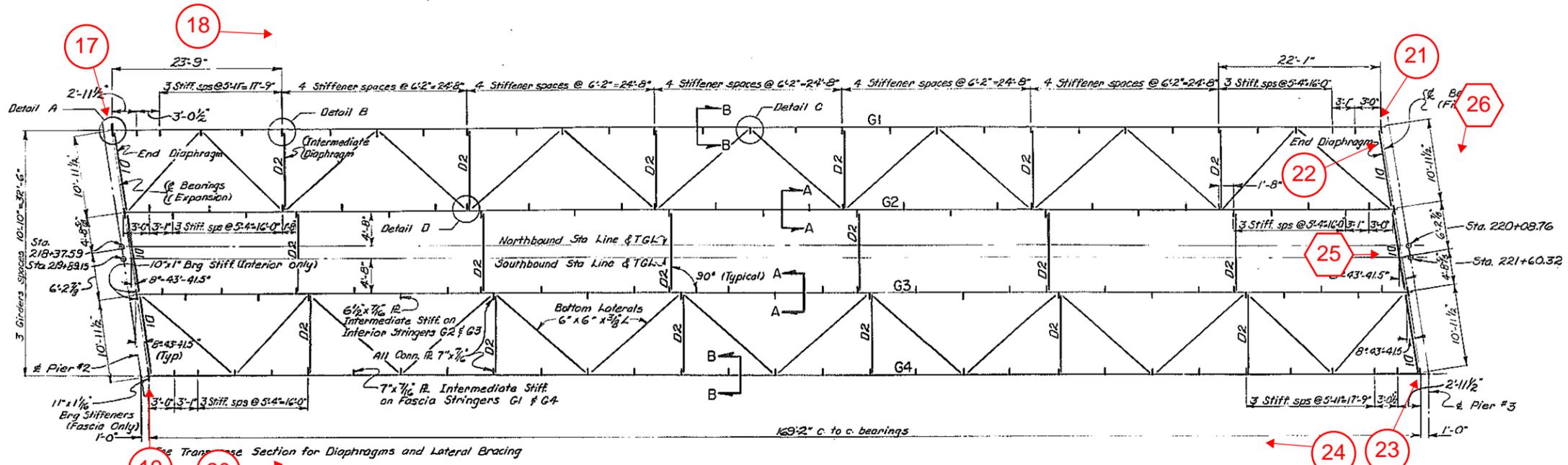
BRIDGE NO. 2
INTERSTATE ROUTE 381 OVER
DEWITT YARDS
SPAN 2 SOUTHBOUND
BIN 1093571 10.20 OF 50

PHOTO LOCATION PLAN

De Witt Yds. Span 2 Steel layout

FED. RD. REG. NO.	STATE	FEDERAL AID PROJECT NO.	SHEET NO.	TOTAL SHEETS
1	NEW YORK	1-690-3(28) 1-481-2(116)	199	309

INTERSTATE ROUTE CONNECTION 570
 BUTTERNUT INTERCHANGE (PHASE 2)
 ONONDAGA COUNTY

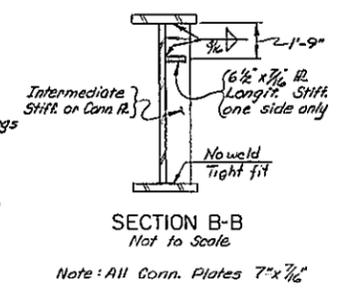
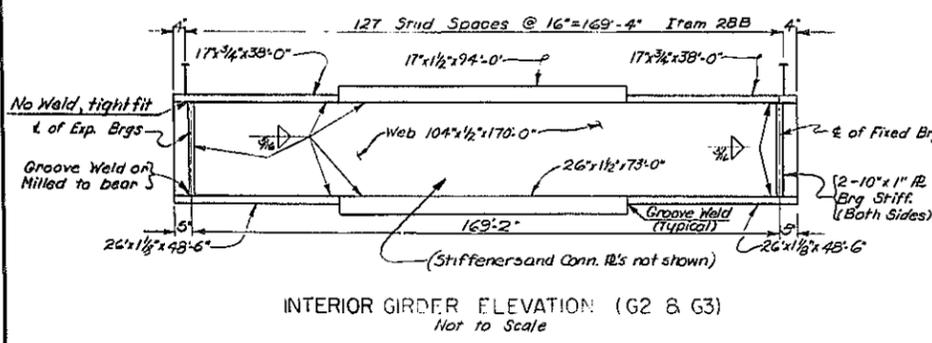
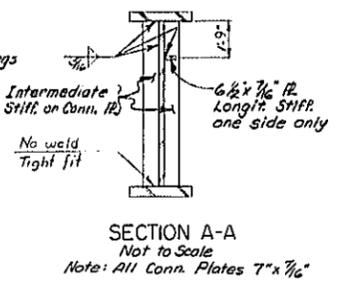
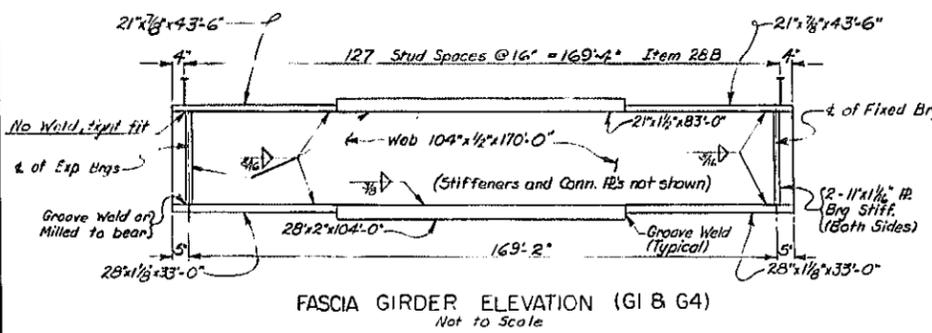


Note: Stud Shear Connectors shall be 6" high.

For Bearings Details see Drawing # 42 and 43
 For Joint Details see Drawing # 31
 For Trough Details see Drawing # 39

- LEGEND**
- # SUPERSTRUCTURE PHOTOS
 - # SUBSTRUCTURE PHOTOS

STEEL LAYOUT (SPAN #3 NB & SB)
 Scale 1/8" = 1'-0"

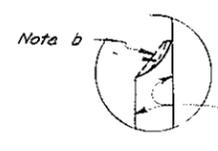
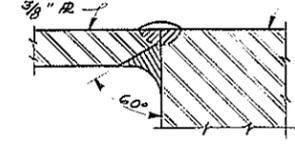


Height of Haunch varies (see Table of Haunch Heights Drawing No. 34)
 Bottom of Slab elevations shown in table are computed for this

GIRDER	NORTHBOUND			SOUTHBOUND			DI. DEFLECTION				CAMBER	
	THEO. BOT. OF SLAB EL.	THEO. BOT. OF SLAB EL.	THEO. BOT. OF SLAB EL.	STEEL (F.T.)	SLAB (F.T.)	S.D.L. (F.T.)	TOTAL (F.T.)	V.C.C. (F.T.)	TOTAL (F.T.)	(I.I.D.)	(I.I.D.)	
SPAN 3	± SO. BRG	± SPAN	± NO. BRG	± SO. BRG	± SPAN	± NO. BRG						
G1	449.24	449.85	450.29	449.10	449.74	450.20	.11	.15	.11	.37	.08	.45 5 3/8
G2	449.48	450.09	450.52	449.34	449.97	450.44	.12	.29	.04	.45	.08	.53 6 3/8
G3	449.46	450.07	450.50	449.39	450.02	450.48	.12	.29	.04	.45	.08	.53 6 3/8
G4	449.25	449.85	450.28	449.18	449.80	450.26	.11	.15	.11	.37	.08	.45 5 3/8

S.D.L. = superimposed dead load, includes weight of parapet and railing.
 V.C.C. = vertical curve correction

Top of Bottom Flange when bottom flange is in tension
 Bottom of Top Flange when top flange is in tension



Note:
 a. The connection plate shall be prepared and welded as a single bevel groove weld as shown in Detail #1. It shall then be Air Carbon-Arc gauged from the second side into sound weld metal and then welded as detailed. All welding shall be in the flat or "downhand" position.
 b. The plate may be of any shape that will provide after welding, cutting, and finish grinding a smooth transition from the flange edge of a minimum radius of 12".
 c. Both the connection plate and flange are to be the same type of steel.
 d. Field welding to the connection plate will not be permitted.

LATERAL BRACING DETAILS
 Not to Scale

For additional details of bottom lateral bracing see drawing No. 22

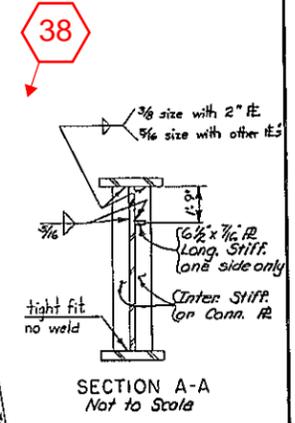
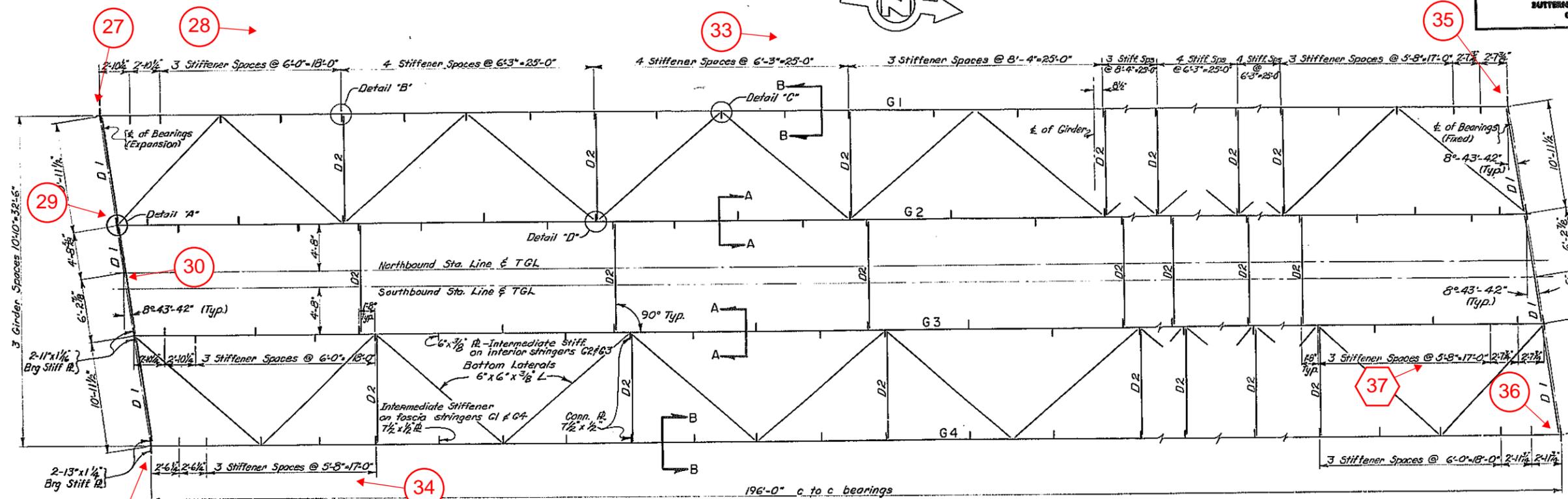
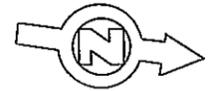
The webs, flanges and gusset plates for bottom lateral bracing for the Span 3 girders shall be A.S.T.M. Designation A441 steel. Stiffeners, diaphragms, bearings, and bottom lateral bracing (except gusset plates) shall be A.S.T.M. A36 steel.

PROJECT ENGINEER: R. PARKER
 IN CHARGE OF: F.W. ECKEL
 DESIGNED BY: S. PAUL
 DESIGN CHECKED BY: H. THOMPSON
 DETAILED BY: J. QUAYSON
 DETAIL CHECKED BY: D.H. SMITH

BRIDGE NO. 2
 INTERSTATE ROUTE 481 OVER DEWITT YARDS
 BIN 1093571 SOUTHBOUND
 SPAN 3
 PHOTO LOCATION PLAN

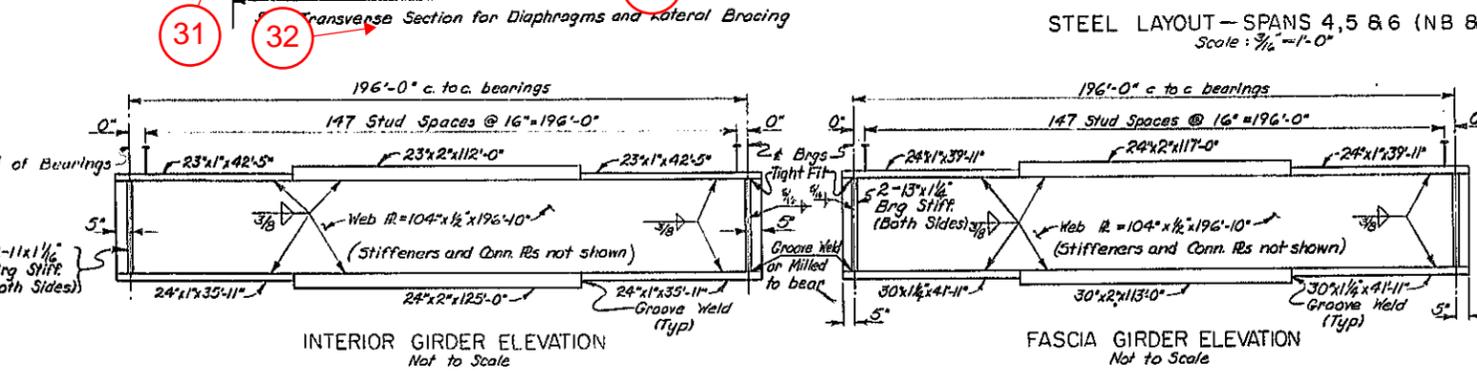
FED. RD. REG. NO.	STATE	FEDERAL AID PROJECT NO.	SHEET NO.	TOTAL SHEETS
1	NEW YORK	I-690-3(28) I-481-2(116)	200	309

INTERSTATE ROUTE CONNECTION 570
BUTTERNUT INTERCHANGE (PHASE 2)
ONONDAGA COUNTY



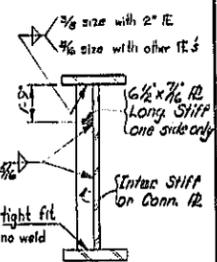
SECTION A-A Not to Scale

All Connection R's 7 1/2 x 1/2



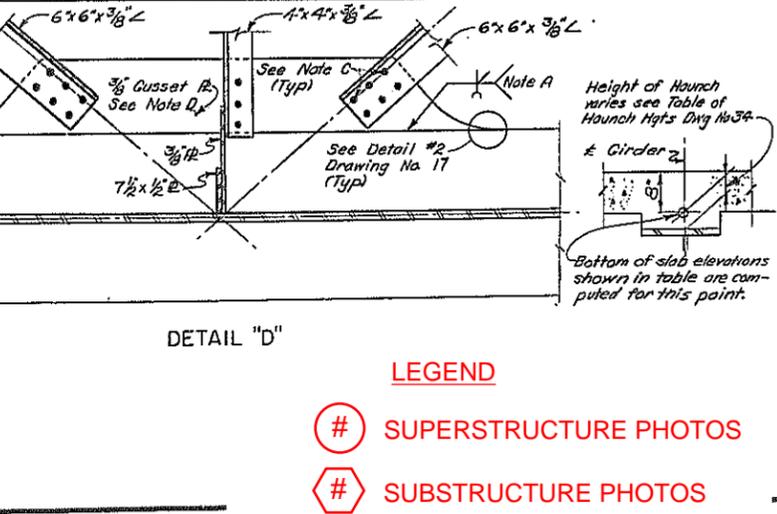
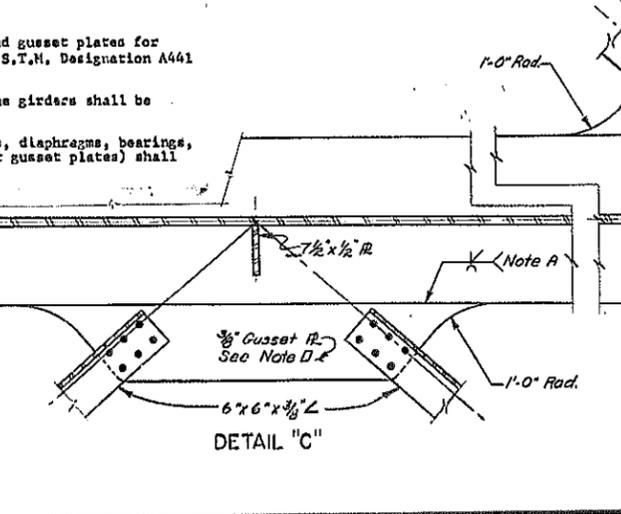
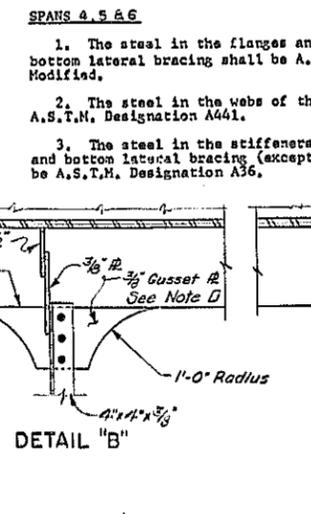
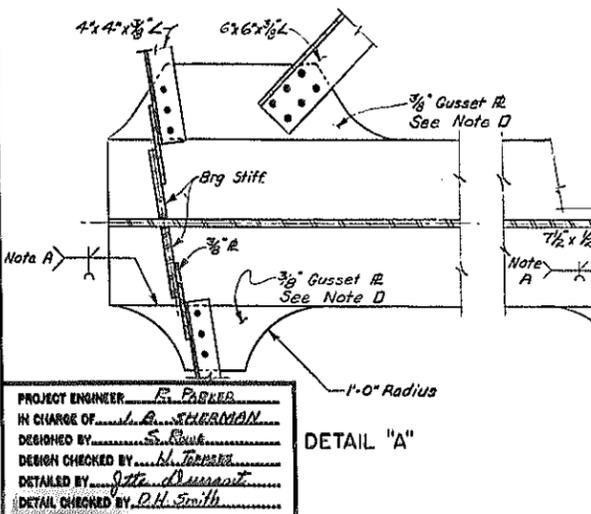
STEEL LAYOUT - SPANS 4, 5 & 6 (NB & SB)
Scale: 3/8" = 1'-0"

GIRDER	SOUTHBOUND				NORTHBOUND				DL DEFLECTION (FT)				CAMBER		
	THEO. BOT. OF SLAB EL.	± SPAN	± NO. BRG	± SO. BRG	THEO. BOT. OF SLAB EL.	± SPAN	± NO. BRG	± SO. BRG	STEEL	SLAB	SDL	TOTAL DL	VCC (FT)	TOTAL FEET	INCHES
SPAN 4															
G1	450.21	450.54	450.65	450.30	450.59	450.66	0.172	0.212	0.168	0.552	0.113	0.665	8"		
G2	450.45	450.77	450.86	450.53	450.82	450.89	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"		
G3	450.48	450.80	450.89	450.51	450.80	450.85	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"		
G4	450.27	450.38	450.67	450.29	450.57	450.63	0.172	0.212	0.168	0.552	0.113	0.665	8"		
SPAN 5															
G1	450.63	450.50	450.14	450.66	450.50	450.10	0.172	0.212	0.168	0.552	0.113	0.665	8"		
G2	450.86	450.72	450.36	450.89	450.72	450.32	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"		
G3	450.89	450.75	450.38	450.85	450.68	450.28	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"		
G4	450.66	450.52	450.15	450.63	450.45	450.05	0.172	0.212	0.168	0.552	0.113	0.665	8"		
SPAN 6															
G1	450.13	449.54	448.72	450.09	449.47	448.62	0.172	0.212	0.168	0.552	0.113	0.665	8"		
G2	450.35	449.75	448.93	450.31	449.68	448.83	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"		
G3	450.37	449.77	448.94	450.27	449.64	448.78	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"		
G4	450.14	449.53	448.70	450.04	449.40	448.54	0.172	0.212	0.168	0.552	0.113	0.665	8"		



SECTION B-B Not to Scale

- SPANS 4, 5 & 6
- The steel in the flanges and gusset plates for bottom lateral bracing shall be A.S.T.M. Designation A441 Modified.
 - The steel in the webs of the girders shall be A.S.T.M. Designation A441.
 - The steel in the stiffeners, diaphragms, bearings, and bottom lateral bracing (except gusset plates) shall be A.S.T.M. Designation A36.



Note:
A. The connection plate shall be prepared and welded as a single bevel groove weld as shown in Detail #1. It shall then be Min Carbon-Arc gouged from the second side into sound weld metal and then welded as detailed. All welding shall be in the flat or "downhand" position.
B. The plate may be of any shape that will provide after welding, cutting, and finish grinding a smooth transition from the flange edge of a minimum radius of 12".
C. Lateral bracing shall be attached to gusset plates with 3/8 inch high strength bolts. Field welding will not be permitted.
D. The gusset plate shall be the same type of steel as the flange to which it is welded.

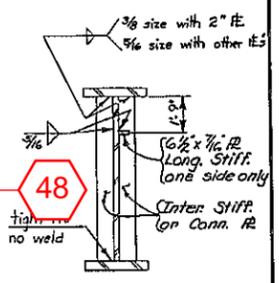
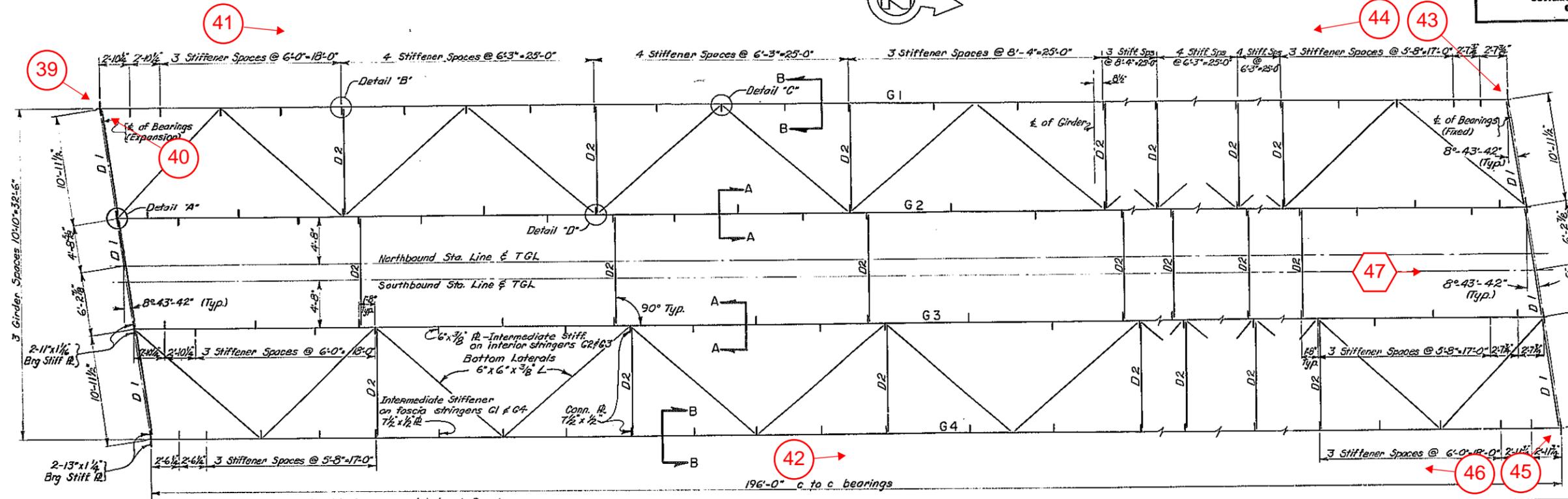
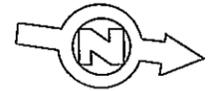
PROJECT ENGINEER: R. PARKER
IN CHARGE OF: J. R. SHERMAN
DESIGNED BY: S. ROWE
DESIGN CHECKED BY: M. TAYLOR
DETAILED BY: J. D. MURPHY
DETAIL CHECKED BY: D. H. SMITH

LEGEND
SUPERSTRUCTURE PHOTOS
SUBSTRUCTURE PHOTOS

BRIDGE NO. 2
INTERSTATE ROUTE 481 OVER DEWITT YARDS
SPANS 4
SOUTHBOUND BIN 1093571
PHOTO LOCATION PLAN

FED. RD. REG. NO.	STATE	FEDERAL AID PROJECT NO.	SHEET NO.	TOTAL SHEETS
1	NEW YORK	1-090-3(28) 1-481-2(116)	200	309

INTERSTATE ROUTE CONNECTION 570
BUTTERNUT INTERCHANGE (PHASE 2)
ONEIDA COUNTY

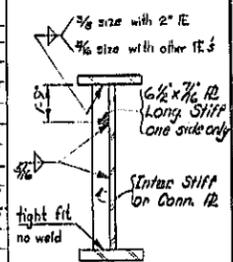


SECTION A-A
Not to Scale

All Connection R's 7/2 x 1/2

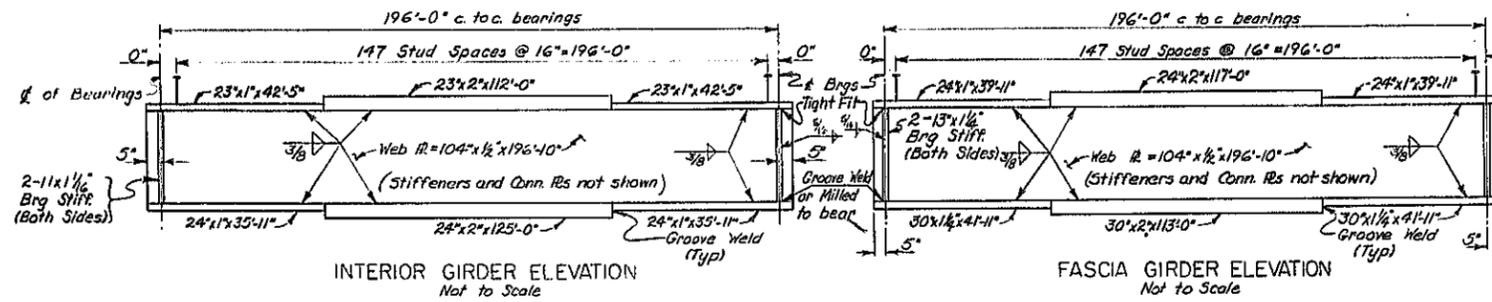
STEEL LAYOUT - SPANS 4, 5 & 6 (NB & SB)
Scale: 3/8" = 1'-0"

GIRDER	SOUTHBOUND				NORTHBOUND				DL DEFLECTION (FT)				CAMBER	
	± SO. BRG	± SPAN	± NO. BRG	± SO. BRG	± SPAN	± NO. BRG	STEEL	SLAB	SDL	TOTAL DL	VCC (FT)	TOTAL	FEET	INCHES
SPAN 4														
G1	450.21	450.54	450.65	450.30	450.59	450.66	0.172	0.212	0.168	0.552	0.113	0.665	8"	
G2	450.45	450.77	450.86	450.53	450.82	450.89	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"	
G3	450.48	450.80	450.89	450.51	450.80	450.85	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"	
G4	450.27	450.38	450.67	450.29	450.57	450.63	0.172	0.212	0.168	0.552	0.113	0.665	8"	
SPAN 5														
G1	450.63	450.50	450.14	450.66	450.50	450.10	0.172	0.212	0.168	0.552	0.113	0.665	8"	
G2	450.86	450.72	450.36	450.89	450.72	450.32	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"	
G3	450.89	450.75	450.38	450.85	450.68	450.28	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"	
G4	450.66	450.52	450.15	450.63	450.45	450.05	0.172	0.212	0.168	0.552	0.113	0.665	8"	
SPAN 6														
G1	450.13	449.54	448.72	450.09	449.47	448.62	0.172	0.212	0.168	0.552	0.113	0.665	8"	
G2	450.35	449.75	448.93	450.31	449.68	448.83	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"	
G3	450.37	449.77	448.94	450.27	449.64	448.78	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"	
G4	450.14	449.53	448.70	450.04	449.40	448.54	0.172	0.212	0.168	0.552	0.113	0.665	8"	



SECTION B-B
Not to Scale

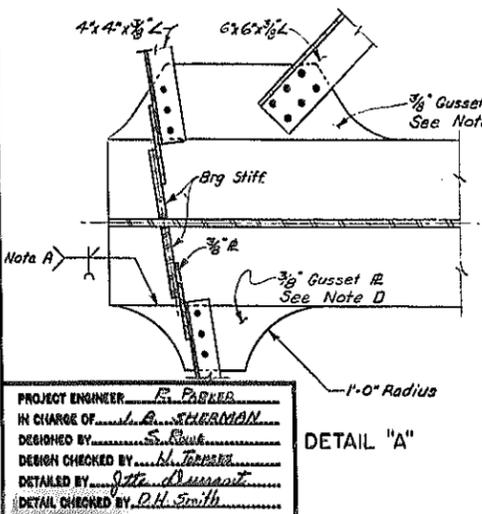
All Connection R's 7/2 x 1/2



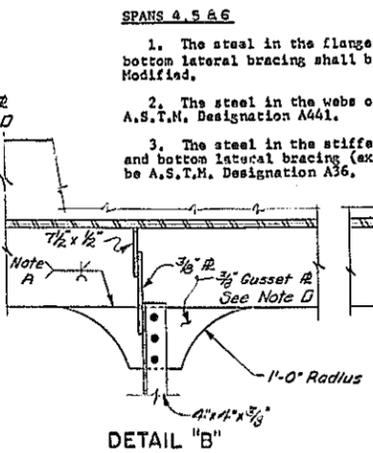
INTERIOR GIRDER ELEVATION
Not to Scale

FASCIA GIRDER ELEVATION
Not to Scale

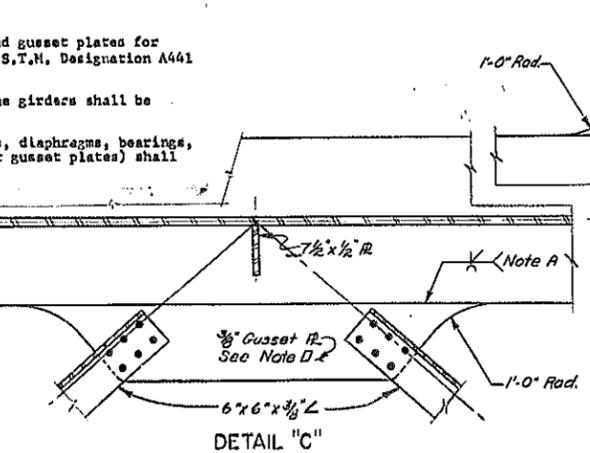
- SPANS 4, 5 & 6
- The steel in the flanges and gusset plates for bottom lateral bracing shall be A.S.T.M. Designation A441 Modified.
 - The steel in the webs of the girders shall be A.S.T.M. Designation A441.
 - The steel in the stiffeners, diaphragms, bearings, and bottom lateral bracing (except gusset plates) shall be A.S.T.M. Designation A36.



DETAIL "A"



DETAIL "B"



DETAIL "C"

DETAIL "D"

LEGEND

- # SUPERSTRUCTURE PHOTOS
- # SUBSTRUCTURE PHOTOS

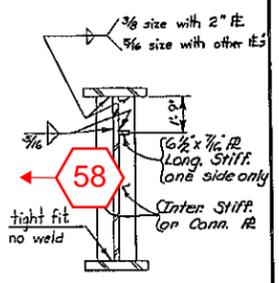
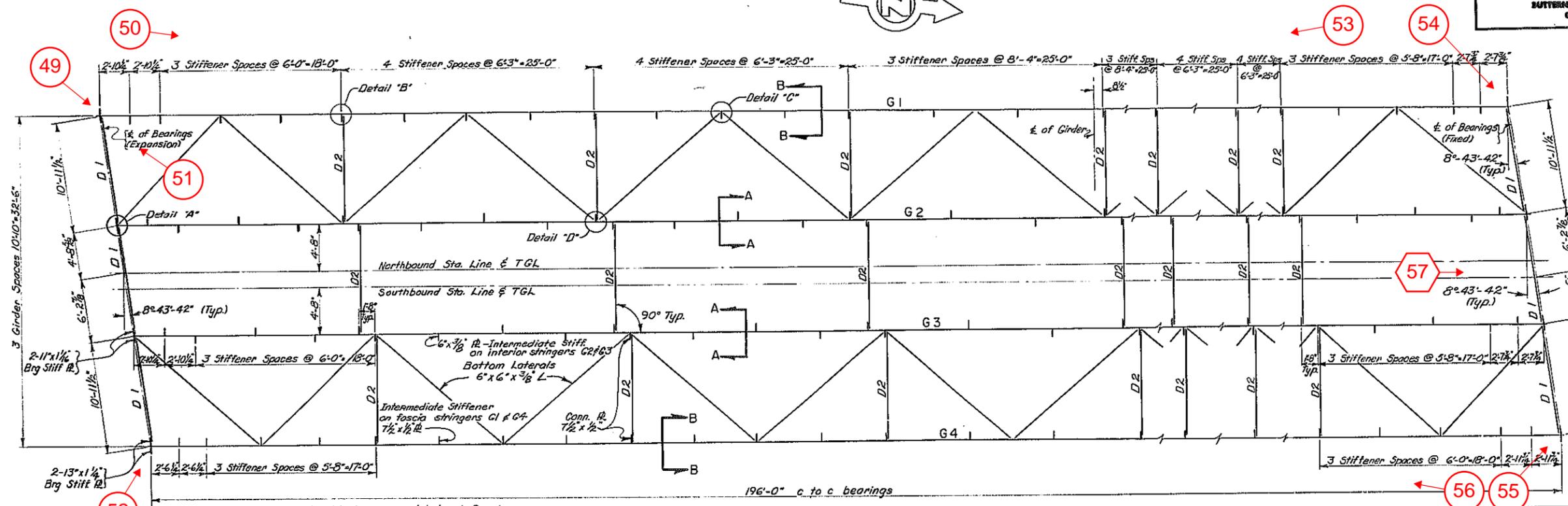
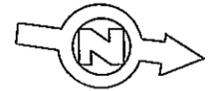
Note:
A. The connection plate shall be prepared and welded as a single bevel groove weld as shown in Detail #1. It shall then be lin Carbon-Arc gouged from the second side into sound weld metal and then welded as detailed. All welding shall be in the flat or "downhand" position.
B. The plate may be of any shape that will provide after welding, cutting, and finish grinding a smooth transition from the flange edge of a minimum radius of 12".
C. Lateral bracing shall be attached to gusset plates with 3/8" high strength bolts. Field welding will not be permitted.
D. The gusset plate shall be the same type of steel as the flange to which it is welded.

PROJECT ENGINEER: R. PARKER
IN CHARGE OF: J. R. SHERMAN
DESIGNED BY: S. ROWE
DESIGN CHECKED BY: M. TAYLOR
DETAILED BY: J. D. DUNN
DETAIL CHECKED BY: D. H. SMITH

BRIDGE NO. 2
INTERSTATE ROUTE 481 OVER DEWITT YARDS
SPANS 5
SOUTHBOUND BIN 1093571
PHOTO LOCATION PLAN

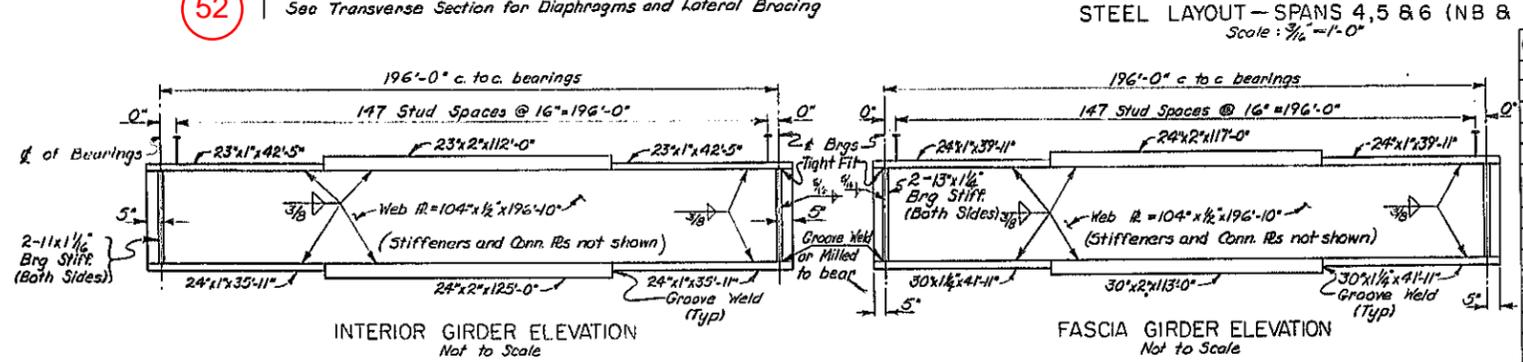
FED. RD. REG. NO.	STATE	FEDERAL AID PROJECT NO.	SHEET NO.	TOTAL SHEETS
1	NEW YORK	I-090-3(28) L-481-2(116)	200	309

INTERSTATE ROUTE CONNECTION 570
BUTTERNUT INTERCHANGE (PHASE 2)
ONEIDA COUNTY



SECTION A-A
Not to Scale

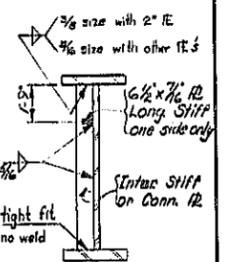
All Connection R's 7 1/2 x 1/2



INTERIOR GIRDER ELEVATION
Not to Scale

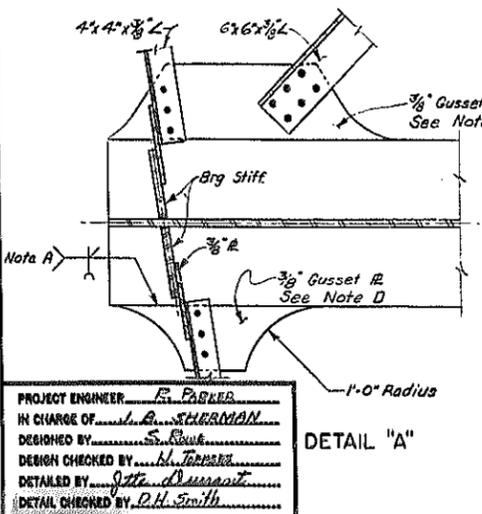
FASCIA GIRDER ELEVATION
Not to Scale

GIRDER	SOUTHBOUND				NORTHBOUND				DL DEFLECTION (FT)				CAMBER	
	THEO. BOT. OF SLAB EL.	± SO. BRG	± NO. BRG	± NO. BRG	± SO. BRG	± SPAN	± NO. BRG	STEEL	SLAB	SDL	TOTAL DL	VCC (FT)	TOTAL FEET	INCHES
SPAN 4														
G1	450.21	450.54	450.65	450.30	450.59	450.66	450.66	0.172	0.212	0.168	0.552	0.113	0.665	8"
G2	450.45	450.77	450.86	450.53	450.82	450.89	450.89	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"
G3	450.48	450.80	450.89	450.51	450.80	450.85	450.85	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"
G4	450.27	450.38	450.67	450.29	450.57	450.63	450.63	0.172	0.212	0.168	0.552	0.113	0.665	8"
SPAN 5														
G1	450.63	450.50	450.14	450.66	450.50	450.10	450.10	0.172	0.212	0.168	0.552	0.113	0.665	8"
G2	450.86	450.72	450.36	450.89	450.72	450.32	450.32	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"
G3	450.89	450.75	450.38	450.85	450.68	450.28	450.28	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"
G4	450.66	450.52	450.15	450.63	450.45	450.05	450.05	0.172	0.212	0.168	0.552	0.113	0.665	8"
SPAN 6														
G1	450.13	449.54	448.72	450.09	449.47	448.62	448.62	0.172	0.212	0.168	0.552	0.113	0.665	8"
G2	450.35	449.75	448.93	450.31	449.68	448.83	448.83	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"
G3	450.37	449.77	448.94	450.27	449.64	448.78	448.78	0.178	0.378	0.062	0.618	0.113	0.731	8 1/2"
G4	450.14	449.53	448.70	450.04	449.40	448.54	448.54	0.172	0.212	0.168	0.552	0.113	0.665	8"

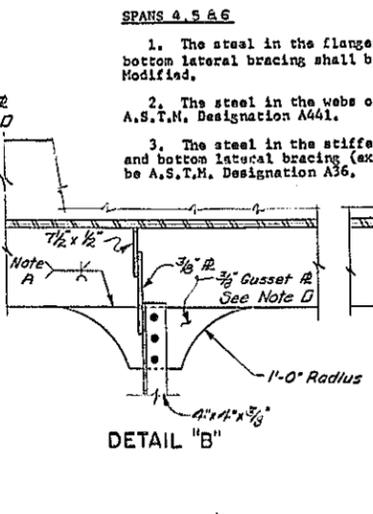


SECTION B-B
Not to Scale

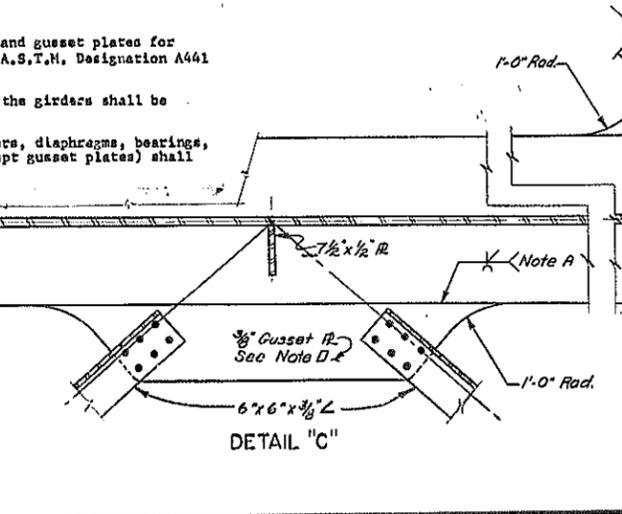
- SPANS 4, 5 & 6**
- The steel in the flanges and gusset plates for bottom lateral bracing shall be A.S.T.M. Designation A441 Modified.
 - The steel in the webs of the girders shall be A.S.T.M. Designation A441.
 - The steel in the stiffeners, diaphragms, bearings, and bottom lateral bracing (except gusset plates) shall be A.S.T.M. Designation A36.



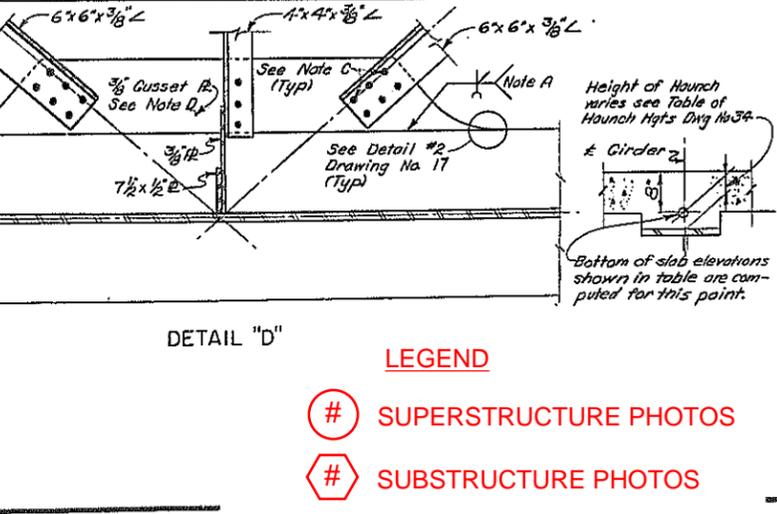
DETAIL "A"



DETAIL "B"



DETAIL "C"



DETAIL "D"

LEGEND

- # SUPERSTRUCTURE PHOTOS
- # SUBSTRUCTURE PHOTOS

Notes:

A. The connection plate shall be prepared and welded as a single bevel groove weld as shown in Detail #1. It shall then be lin Carbon-Arc gouged from the second side into sound weld metal and then welded as detailed. All welding shall be in the flat or "downhand" position.

B. The plate may be of any shape that will provide after welding, cutting, and finish grinding a smooth transition from the flange edge of a minimum radius of 12".

C. Lateral bracing shall be attached to gusset plates with 3/8 inch high strength bolts. Field welding will not be permitted.

D. The gusset plate shall be the same type of steel as the flange to which it is welded.

PROJECT ENGINEER: R. PARKER
IN CHARGE OF: J. SHERMAN
DESIGNED BY: S. ROWE
DESIGN CHECKED BY: M. TAYLOR
DETAILED BY: J. D. MURPHY
DETAIL CHECKED BY: D.H. SMITH

BRIDGE NO. 2
INTERSTATE ROUTE 481 OVER DEWITT YARDS

SPANS 6
SOUTHBOUND BIN 1093571
PHOTO LOCATION PLAN

FED. RD. REG. NO.	STATE	FEDERAL AID PROJECT NO.	SHEET NO.	TOTAL SHEETS
1	NEW YORK	1-690-3(25) 1-481-2(116)	202	309

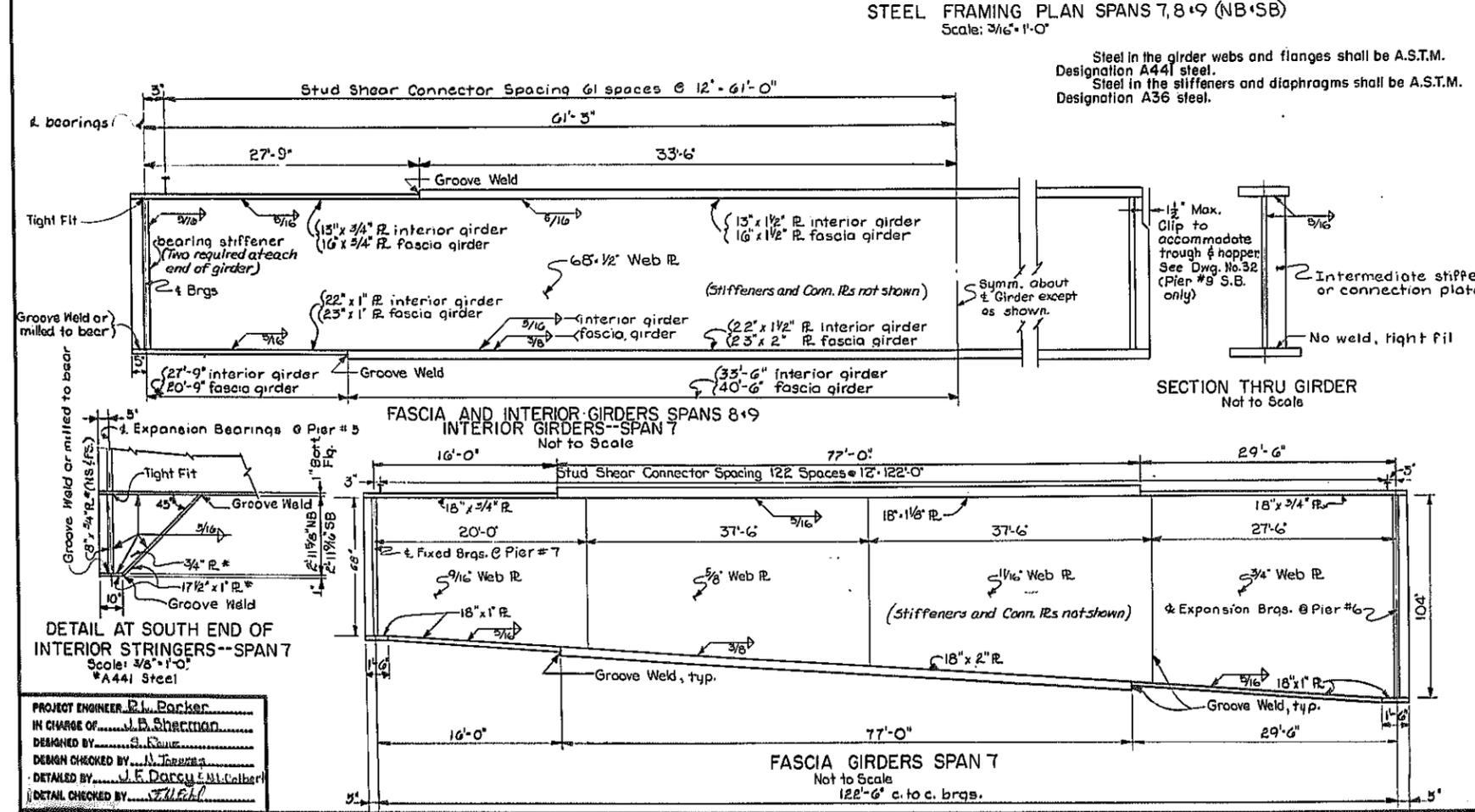
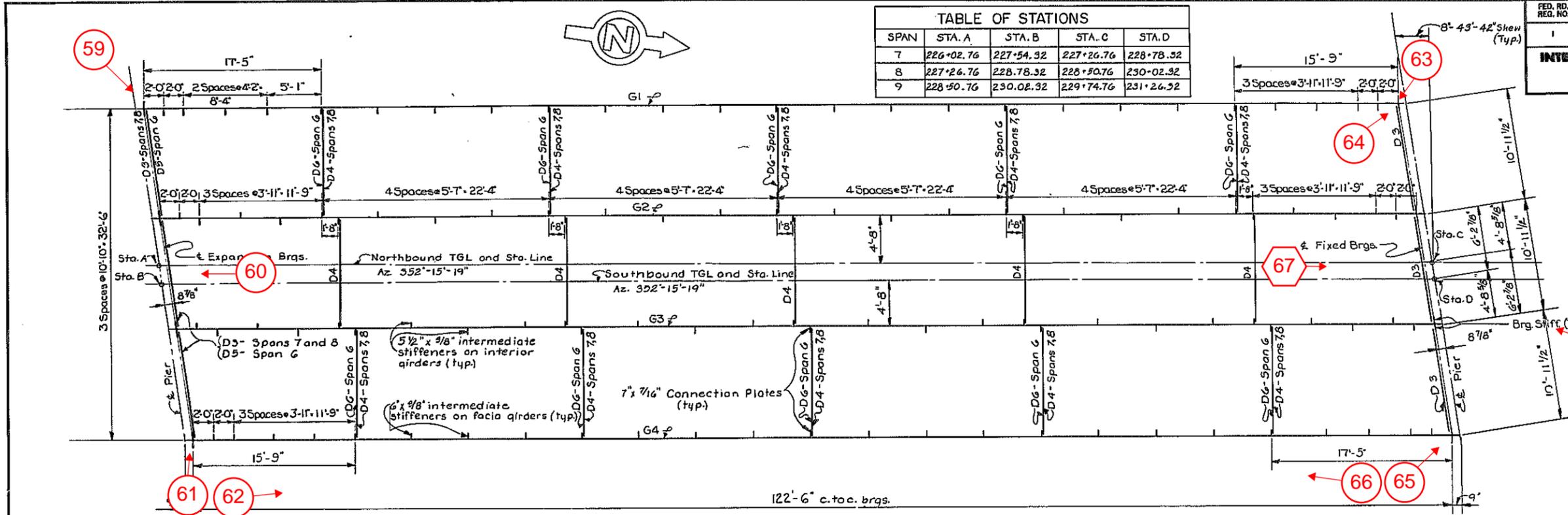
INTERSTATE ROUTE CONNECTION 570
INTERMITTENT INTERCHANGE (PHASE 2)
ONEIDA COUNTY

SPAN	STA. A	STA. B	STA. C	STA. D
7	226+02.76	227+54.32	227+26.76	228+78.52
8	227+26.76	228+78.52	228+50.76	230+02.32
9	228+50.76	230+02.32	229+74.76	231+26.32

LEGEND

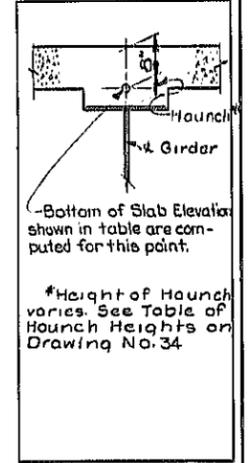
- # SUPERSTRUCTURE PHOTOS
- # SUBSTRUCTURE PHOTOS

Girder	Size of each plate (2 plates @ each end of girder)
Interior spans 7 & 8	8" x 3/4"
Fascia span 7	8" x 3/4"
Fascia spans 8 & 9	9" x 7/8"



NORTHBOUND				SOUTHBOUND					
GIRDER	SPAN	EXP. BRGS.	FIX. BRGS.	GIRDER	SPAN	EXP. BRGS.	FIX. BRGS.		
G1	7	448.60	447.95	447.22	G1	7	448.70	448.07	447.35
G2	7	448.81	448.16	447.42	G2	7	448.91	448.28	447.56
G3	7	448.77	448.11	447.37	G3	7	448.93	448.29	447.57
G4	7	448.52	447.87	447.12	G4	7	448.68	448.05	447.32
G1	8	447.20	446.37	445.45	G1	8	447.33	446.52	445.63
G2	8	447.40	446.57	445.65	G2	8	447.54	446.73	445.83
G3	8	447.35	446.52	445.59	G3	8	447.55	446.73	445.83
G4	8	447.10	446.27	445.34	G4	8	447.30	446.48	445.58
G1	9	445.43	444.42	443.53	G1	9	445.60	444.61	443.97
G2	9	445.63	444.62	443.88	G2	9	445.80	444.81	443.74
G3	9	445.57	444.86	443.46	G3	9	445.81	444.81	443.73
G4	9	445.32	444.50	443.20	G4	9	445.56	444.56	443.48

GIRDER	SPAN	DEFLECTIONS				CAMBER	
		STEEL (FT)	SLAB (FT)	SDL (FT)	TOTAL (FT)	V.C.C. (FT)	TOTAL (FT)
G1	7	.05	.09	.06	.20	.04	.24
G2	7	.08	.24	.03	.35	.04	.39
G3	7	.08	.24	.03	.35	.04	.39
G4	7	.05	.09	.06	.20	.04	.24
G1	8	.07	.12	.08	.27	.04	.31
G2	8	.08	.24	.03	.35	.04	.39
G3	8	.08	.24	.03	.35	.04	.39
G4	8	.07	.12	.08	.27	.04	.31
G1	9	.07	.12	.08	.27	.02	.29
G2	9	.08	.24	.03	.35	.03	.38
G3	9	.08	.24	.03	.35	.04	.39
G4	9	.07	.12	.08	.27	.04	.31
G1	9	.07	.12	.08	.27	-.06	.21
G2	9	.08	.24	.03	.35	.01	.36
G3	9	.08	.24	.03	.35	.04	.39
G4	9	.07	.12	.08	.27	.04	.31



V.C.C.=Vertical Curve Camber
S.D.L.=Superimposed dead load, includes the weight of sidewalk railing.

BRIDGE NO. 2
INTERSTATE ROUTE 481 OVER DEWITT YARDS
SPANS 7
SOUTHBOUND BIN 1093571
PHOTO LOCATION PLAN

PROJECT ENGINEER: R.L. Barker
IN CHARGE OF: J.D. Sherman
DESIGNED BY: S. F. ...
DESIGN CHECKED BY: J.F. Darcy & M. Calvert
DETAILED BY: J.F. Darcy & M. Calvert
DETAIL CHECKED BY: J.F. Darcy & M. Calvert

Bin #2 Dewitt Yds Steel Frame

FED. RD. REG. NO.	STATE	FEDERAL AID PROJECT NO.	SHEET NO.	TOTAL SHEETS
1	NEW YORK	1-690-3(26) 1-481-2(116)	202	309

INTERSTATE ROUTE CONNECTION 570
 INTERMITTENT INTERCHANGE (PHASE 2)
 ONONDAGA COUNTY

TABLE OF STATIONS

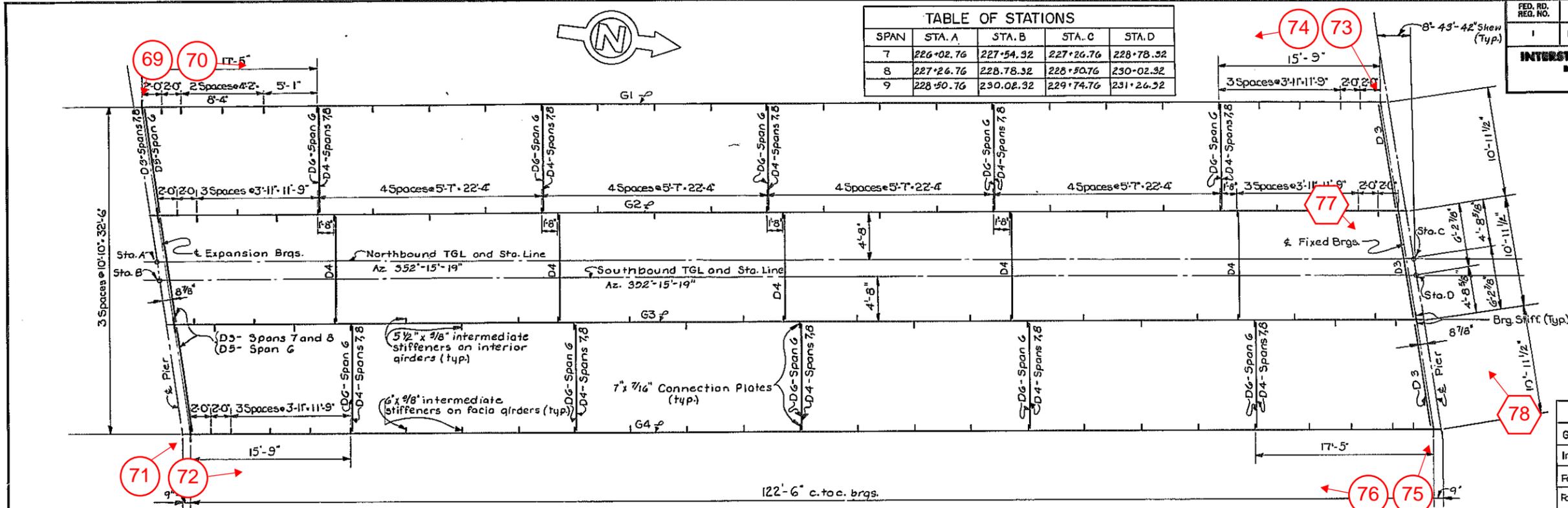
SPAN	STA. A	STA. B	STA. C	STA. D
7	226+02.76	227+54.32	227+26.76	228+78.52
8	227+26.76	228+78.52	228+50.76	230+02.32
9	228+50.76	230+02.32	229+74.76	231+26.32

LEGEND

- # SUPERSTRUCTURE PHOTOS
- # SUBSTRUCTURE PHOTOS

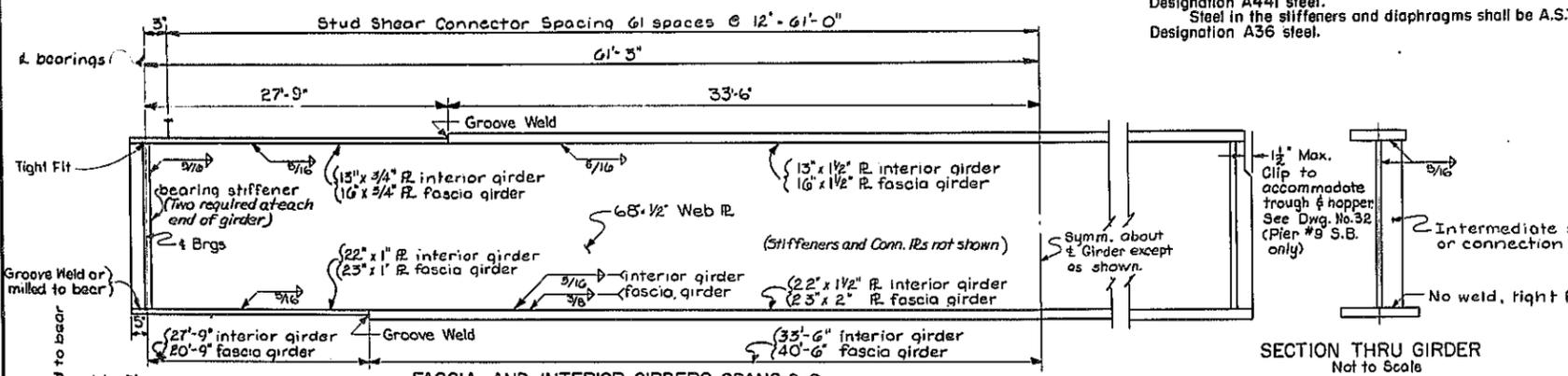
BEARING STIFFENER SIZES

Girder	Size of each plate (2 plates @ each end of girder)
Interior spans 7 & 8	8" x 3/4"
Fascia span 7	8" x 3/4"
Fascia spans 8 & 9	9" x 7/8"

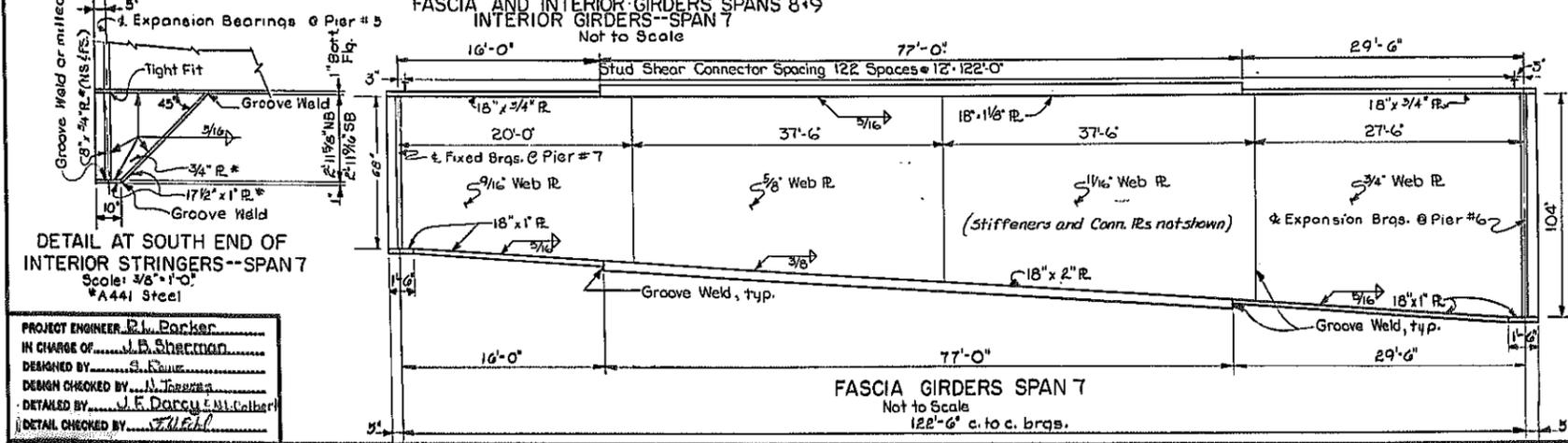


STEEL FRAMING PLAN SPANS 7, 8 & 9 (NB+SB)
 Scale: 3/16" = 1'-0"

Steel in the girder webs and flanges shall be A.S.T.M. Designation A441 steel.
 Steel in the stiffeners and diaphragms shall be A.S.T.M. Designation A36 steel.



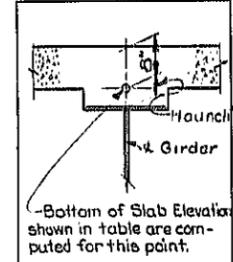
SECTION THRU GIRDER
 Not to Scale



FASCIA GIRDERS SPAN 7
 Not to Scale
 122'-6" c. to c. brgs.

BOTTOM OF SLAB ELEVATIONS

NORTHBOUND				SOUTHBOUND					
GIRDER	SPAN	EXP. BRGS.	FIX. BRGS.	GIRDER	SPAN	EXP. BRGS.	FIX. BRGS.		
G1	7	448.60	447.95	447.22	G1	7	448.70	448.07	447.35
G2	7	448.81	448.16	447.42	G2	7	448.91	448.28	447.56
G3	7	448.77	448.11	447.37	G3	7	448.93	448.29	447.57
G4	7	448.52	447.87	447.12	G4	7	448.68	448.05	447.32
G1	8	447.20	446.37	445.45	G1	8	447.33	446.52	445.63
G2	8	447.40	446.57	445.65	G2	8	447.54	446.73	445.83
G3	8	447.35	446.52	445.59	G3	8	447.55	446.73	445.83
G4	8	447.10	446.27	445.34	G4	8	447.30	446.48	445.58
G1	9	445.43	444.42	443.53	G1	9	445.60	444.61	443.97
G2	9	445.63	444.62	443.88	G2	9	445.80	444.81	443.74
G3	9	445.57	444.86	443.46	G3	9	445.81	444.81	443.73
G4	9	445.32	444.50	443.20	G4	9	445.56	444.56	443.48



*Height of Haunch varies. See Table of Haunch Heights on Drawing No. 34.

DEFLECTIONS

GIRDER	SPAN	DEFLECTIONS				CAMBER	
		STEEL (FT)	SLAB (FT)	SDL (FT)	TOTAL (FT)	V.C.C. (FT)	TOTAL (FT)
G1	7	.05	.09	.06	.20	.04	.24
G2	7	.08	.24	.03	.35	.04	.39
G3	7	.08	.24	.03	.35	.04	.39
G4	7	.05	.09	.06	.20	.04	.24
G1	8	.07	.12	.08	.27	.04	.31
G2	8	.08	.24	.03	.35	.04	.39
G3	8	.08	.24	.03	.35	.04	.39
G4	8	.07	.12	.08	.27	.04	.31
G1	9	.07	.12	.08	.27	.02	.29
G2	9	.08	.24	.03	.35	.03	.38
G3	9	.08	.24	.03	.35	.04	.39
G4	9	.07	.12	.08	.27	.04	.31
G1	9	.07	.12	.08	.27	-.06	.21
G2	9	.08	.24	.03	.35	.01	.36
G3	9	.08	.24	.03	.35	.04	.39
G4	9	.07	.12	.08	.27	.04	.31

V.C.C.=Vertical Curve Camber
 S.D.L.=Superimposed dead load, includes the weight of sidewalk railing.

BRIDGE NO. 2
 INTERSTATE ROUTE 481 OVER DEWITT YARDS
 SPANS 8
SOUTHBOUND BIN 1093571
PHOTO LOCATION PLAN

PROJECT ENGINEER: R.L. Barker
 IN CHARGE OF: J.D. Sherman
 DESIGNED BY: S. F. ...
 DESIGN CHECKED BY: J.F. Darcy & M. Calvert
 DETAILED BY: J.F. Darcy & M. Calvert
 DETAIL CHECKED BY: J.F. Darcy & M. Calvert

Bin #2 DeWitt Yds Steel Frame

FED. RD. REG. NO.	STATE	FEDERAL AID PROJECT NO.	SHEET NO.	TOTAL SHEETS
1	NEW YORK	1-690-3(26)	202	309

INTERSTATE ROUTE CONNECTION 570
 INTERMITTENT INTERCHANGE (PHASE 2)
 ONONDAGA COUNTY

SPAN	STA. A	STA. B	STA. C	STA. D
7	226+02.76	227+54.32	227+26.76	228+78.52
8	227+26.76	228+78.52	228+50.76	230+02.32
9	228+50.76	230+02.32	229+74.76	231+26.32

LEGEND

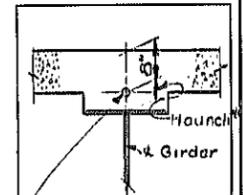
- # SUPERSTRUCTURE PHOTOS
- # SUBSTRUCTURE PHOTOS

Girder	Size of each plate (2 plates @ each end of girder)
Interior spans 7 & 8	8" x 3/4"
Fascia span 7	8" x 3/4"
Fascia spans 8 & 9	9" x 7/8"

STEEL FRAMING PLAN SPANS 7, 8 & 9 (NB+SB)
 Scale: 3/16" = 1'-0"

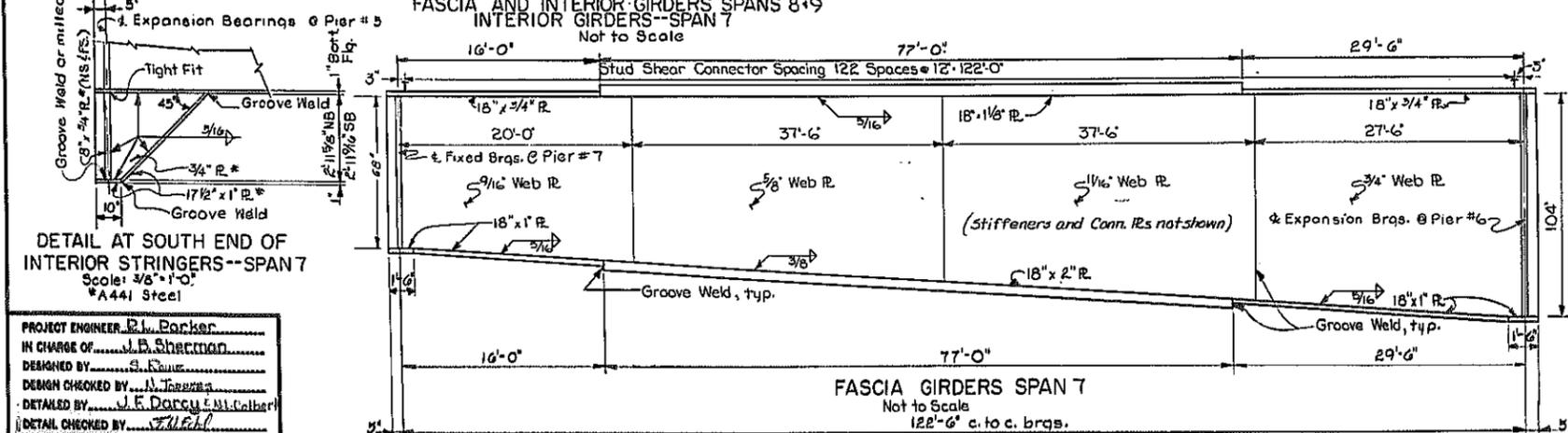
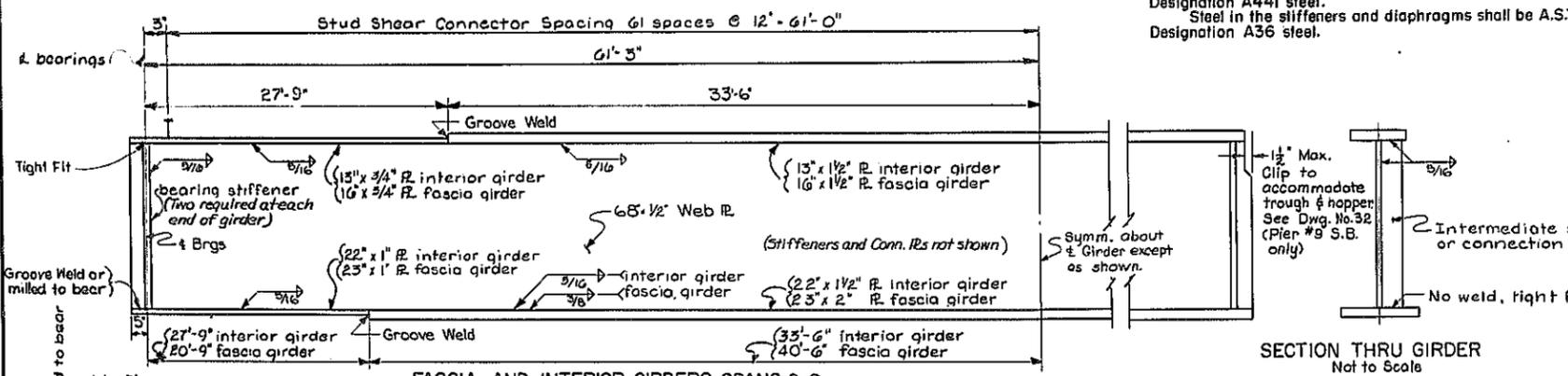
Steel in the girder webs and flanges shall be A.S.T.M. Designation A441 steel.
 Steel in the stiffeners and diaphragms shall be A.S.T.M. Designation A36 steel.

NORTHBOUND				SOUTHBOUND					
GIRDER	SPAN	EXP. BRGS.	FIX. BRGS.	GIRDER	SPAN	EXP. BRGS.	FIX. BRGS.		
G1	7	448.60	447.95	447.22	G1	7	448.70	448.07	447.35
G2	7	448.81	448.16	447.42	G2	7	448.91	448.28	447.66
G3	7	448.77	448.11	447.37	G3	7	448.93	448.29	447.57
G4	7	448.52	447.87	447.12	G4	7	448.68	448.05	447.32
G1	8	447.20	446.37	445.45	G1	8	447.33	446.52	445.63
G2	8	447.40	446.57	445.65	G2	8	447.54	446.73	445.83
G3	8	447.35	446.52	445.59	G3	8	447.55	446.73	445.83
G4	8	447.10	446.27	445.34	G4	8	447.30	446.48	445.58
G1	9	445.43	444.42	443.53	G1	9	445.60	444.61	443.97
G2	9	445.63	444.62	443.88	G2	9	445.80	444.81	443.74
G3	9	445.57	444.86	443.46	G3	9	445.81	444.81	443.73
G4	9	445.32	444.50	443.20	G4	9	445.56	444.56	443.48



Bottom of Slab Elevations shown in table are computed for this point.

*Height of Haunch varies. See Table of Haunch Heights on Drawing No. 34.



DETAIL AT SOUTH END OF INTERIOR STRINGERS--SPAN 7
 Scale: 3/8" = 1'-0"
 A441 Steel

PROJECT ENGINEER: R.L. Barker
 IN CHARGE OF: J.D. Sherman
 DESIGNED BY: S. F. ...
 DESIGN CHECKED BY: J.F. Darcy & M. Calvert
 DETAILED BY: J.F. Darcy & M. Calvert
 CHECKED BY: J.F. Darcy

GIRDER	SPAN	DEFLECTIONS				CAMBER	
		STEEL (FT)	SLAB (FT)	SDL (FT)	TOTAL (FT)	V.C.C. (FT)	TOTAL (FT)
G1	7	.05	.09	.06	.20	.04	.24
G2	7	.08	.24	.03	.35	.04	.39
G3	7	.08	.24	.03	.35	.04	.39
G4	7	.05	.09	.06	.20	.04	.24
G1	8	.07	.12	.08	.27	.04	.31
G2	8	.08	.24	.03	.35	.04	.39
G3	8	.08	.24	.03	.35	.04	.39
G4	8	.07	.12	.08	.27	.04	.31
G1	9	.07	.12	.08	.27	.02	.29
G2	9	.08	.24	.03	.35	.03	.38
G3	9	.08	.24	.03	.35	.04	.39
G4	9	.07	.12	.08	.27	.04	.31
G1	9	.07	.12	.08	.27	-.06	.21
G2	9	.08	.24	.03	.35	.01	.36
G3	9	.08	.24	.03	.35	.04	.39
G4	9	.07	.12	.08	.27	.04	.31

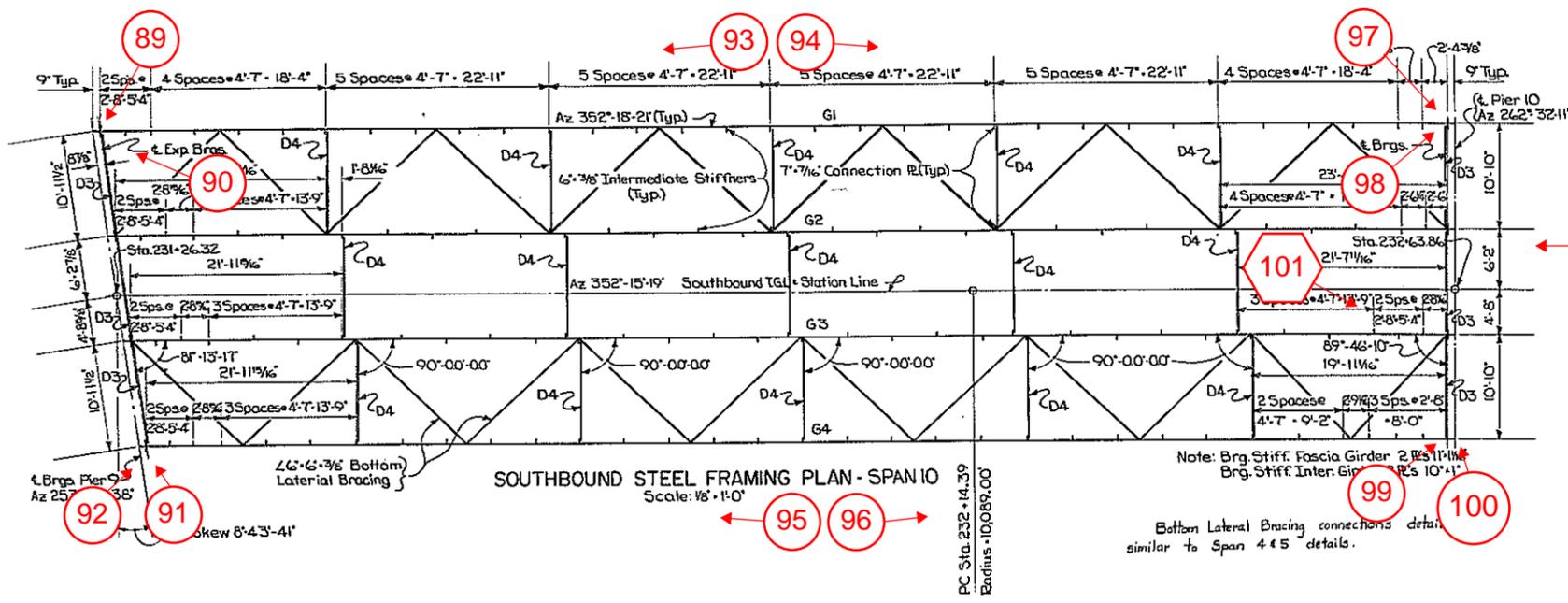
V.C.C.=Vertical Curve Camber
 S.D.L.=Superimposed dead load, includes the weight of sidewalk railing.

BRIDGE NO. 2
 INTERSTATE ROUTE 481 OVER DEWITT YARDS
 SPANS 9
SOUTHBOUND BIN 1093571
PHOTO LOCATION PLAN

Bin #2 DeWitt Yds Steel Frame

FED. RD. REG. NO.	STATE	FEDERAL AID PROJECT NO.	SHEET NO.	TOTAL SHEETS
1	NEW YORK	1-690-3(2B) 1-481-2(116)	204	309

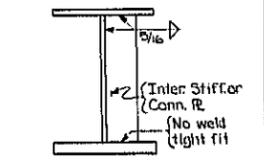
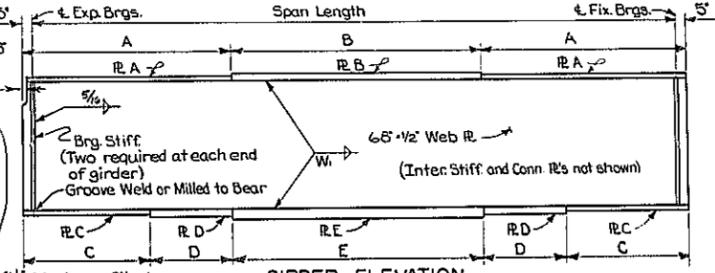
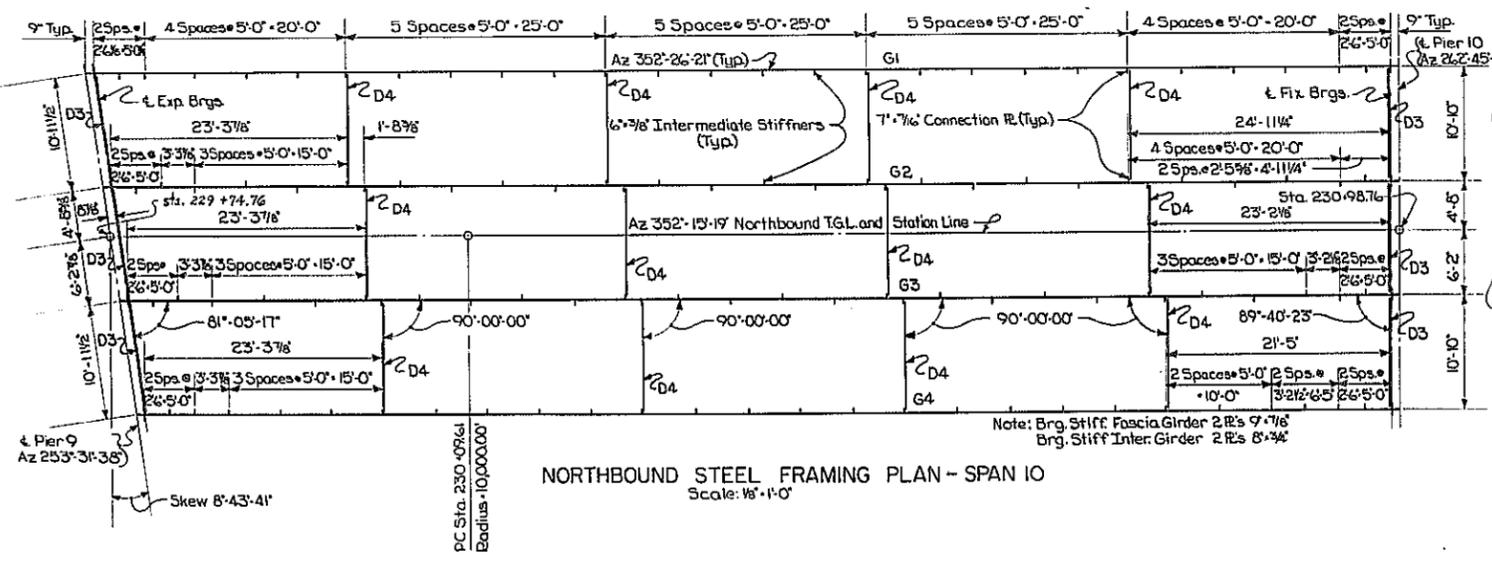
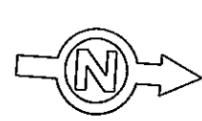
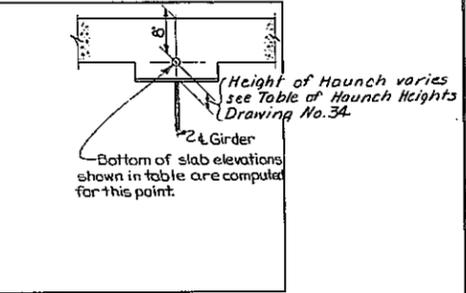
INTERSTATE ROUTE CONNECTION 570
BUTTERNUT INTERCHANGE (PHASE 2)
ONEIDA COUNTY



BOTTOM OF SLAB ELEVATIONS

SOUTHBOUND			
SPAN	EXP BRGS	CL. SPAN	CL. FIX. BRGS
G1	10	443.54	442.46
G2	10	443.72	442.49
G3	10	443.70	442.40
G4	10	443.45	442.16

NORTHBOUND			
GIRDER	SPAN	EXP BRGS	CL. SPAN
G1	10	443.50	442.49
G2	10	443.56	442.42
G3	10	443.43	442.24
G4	10	443.17	442.00



Notes: Flanges, webs, and gusset plates for bottom lateral bracing shall be A.S.T.M. Designation A441 Steel.
 Diaphragms, stiffeners, bearings, and bottom lateral bracing (except gusset plates) shall be A.S.T.M. Designation A36 Steel.

- LEGEND**
- # SUPERSTRUCTURE PHOTOS
 - # SUBSTRUCTURE PHOTOS

SPAN IO GIRDER TABLE

GIRDER	SPAN LENGTH CL TO CL BRGS	FLANGE PLATE SIZES				FLANGE PLATE LENGTHS					DEFLECTIONS (FT)			CAMBER		STUD SHEAR CONNECTOR SPACING			
		R A	R B	R C	R D	R E	A	B	C	D	E	STEEL	CONC.	S.D.L.	TOTAL		V.C.C. (FT)	TOTAL	
Southbound																			
G1	138'-8 3/4"	23'-1"	23'-1 1/2"	24'-1"	24'-2"	24'-2 1/2"	36'-9 7/8"	66'-0"	20'-3 3/8"	19'-0"	61'-0"	0.10	0.15	0.11	0.36	.06	0.42'	5'	138 Spaces @ 12' - 138'-0"
G2	137'-0 3/4"	19'-7/8"	19'-1 1/2"	24'-1"	None	24'-2"	32'-11 1/8"	72'-0"	23'-5 1/8"	0	91'-0"	0.11	0.28	0.04	0.43	.06	0.49	5 1/2"	137 Spaces @ 12' - 137'-0"
G3	135'-3 3/8"	19'-7/8"	19'-1 1/2"	24'-1"	None	24'-2"	32'-0 3/4"	72'-0"	22'-6 3/4"	0	91'-0"	0.10	0.27	0.04	0.41	.05	0.46	5 1/2"	135 Spaces @ 12' - 135'-0"
G4	133'-7"	23'-1"	23'-1 1/2"	24'-1"	24'-2"	24'-2 1/2"	34'-2 1/2"	66'-0"	17'-8 1/2"	19'-0"	61'-0"	0.09	0.13	0.09	0.31	.05	0.36	4 3/8"	133 Spaces @ 12' - 133'-0"
Northbound																			
G1	125'-0 1/4"	17'-3/4"	17'-1 1/2"	24'-1"	None	24'-2"	28'-11 1/8"	68'-0"	21'-5 1/8"	0	83'-0"	0.07	0.12	0.09	0.28	.05	0.33	4"	125 Spaces @ 12' - 125'-0"
G2	123'-3 1/8"	14'-3/4"	14'-1 1/2"	23'-1"	None	23'-1 1/2"	27'-6 3/4"	69'-0"	28'-0 3/4"	0	68'-0"	0.08	0.24	0.03	0.35	.05	0.40	4 3/4"	123 Spaces @ 12' - 123'-0"
G3	121'-6"	14'-3/4"	14'-1 1/2"	23'-1"	None	23'-1 1/2"	26'-8"	69'-0"	27'-2"	0	68'-0"	0.07	0.23	0.03	0.33	.04	0.37	4 1/2"	121 Spaces @ 12' - 121'-0"
G4	119'-8 1/8"	17'-3/4"	17'-1 1/2"	24'-1"	None	24'-2"	26'-3 1/8"	68'-0"	18'-9 1/8"	0	83'-0"	0.06	0.10	0.08	0.24	.04	0.28	3 3/8"	119 Spaces @ 12' - 119'-0"

S.D.L. = superimposed dead load, includes weight of railing and parapet.
 V.C.C. = vertical curve correction

PROJECT ENGINEER R.L. Parker
 IN CHARGE OF E. Finkel
 DESIGNED BY E. Finkel
 DESIGN CHECKED BY J. Darcy
 DETAILED BY J.F. Darcy
 DETAIL CHECKED BY J.F. Darcy

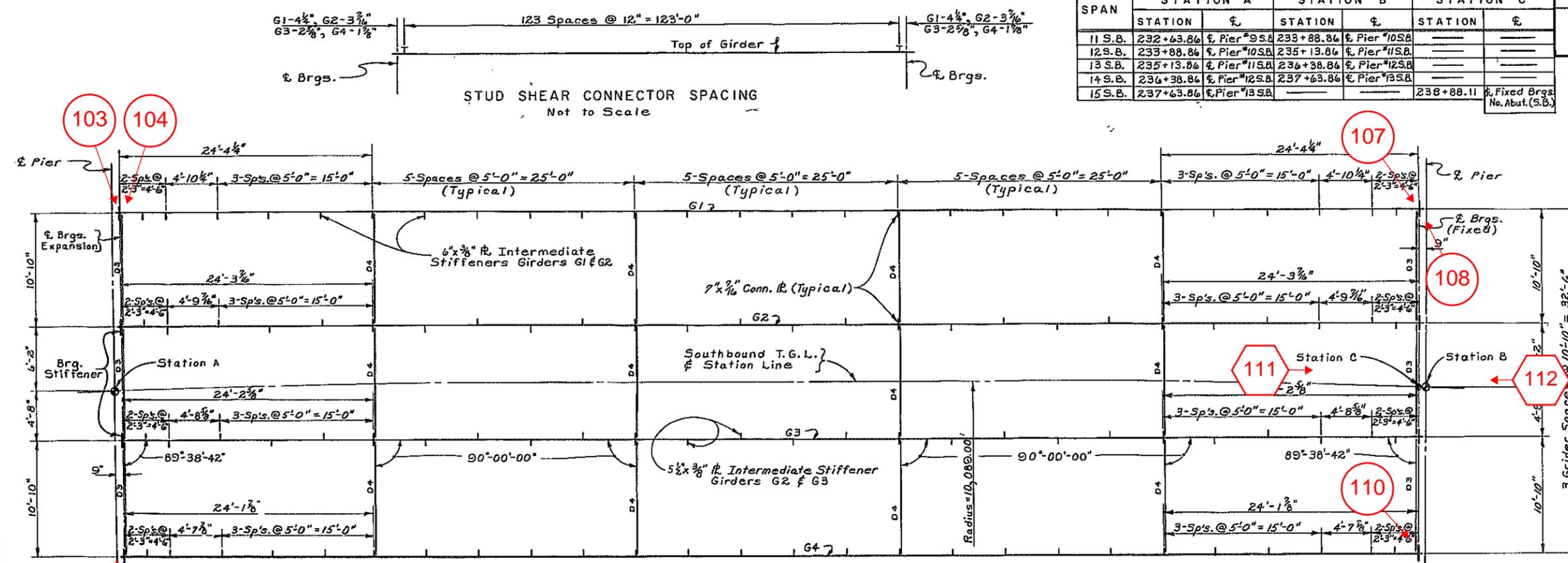
BRIDGE NO. 2
 INTERSTATE ROUTE 481 OVER DEWITT YARDS
 SPAN IO
 SOUTHBOUND BIN 1093571
 PHOTO LOCATION PLAN

SPAN	STATION "A"		STATION "B"		STATION "C"	
	STATION	℄	STATION	℄	STATION	℄
11 S.B.	232+63.86	℄ Pier #9 S.B.	233+88.86	℄ Pier #10 S.B.		
12 S.B.	233+88.86	℄ Pier #10 S.B.	235+13.86	℄ Pier #11 S.B.		
13 S.B.	235+13.86	℄ Pier #11 S.B.	236+38.86	℄ Pier #12 S.B.		
14 S.B.	236+38.86	℄ Pier #12 S.B.	237+63.86	℄ Pier #13 S.B.		
15 S.B.	237+63.86	℄ Pier #13 S.B.		238+88.11	℄ Fixed Brgs. No. Abut. (S.B.)	

FED. RD. REG. NO.	STATE	FEDERAL AID PROJECT NO.	SHEET NO.	TOTAL SHEETS
1	NEW YORK	I-690-3(2B) I-481-2(116)	206	309

INTERSTATE ROUTE CONNECTION 570
SUFFERNUT INTERCHANGE (PHASE 2)
ONONDAGA COUNTY

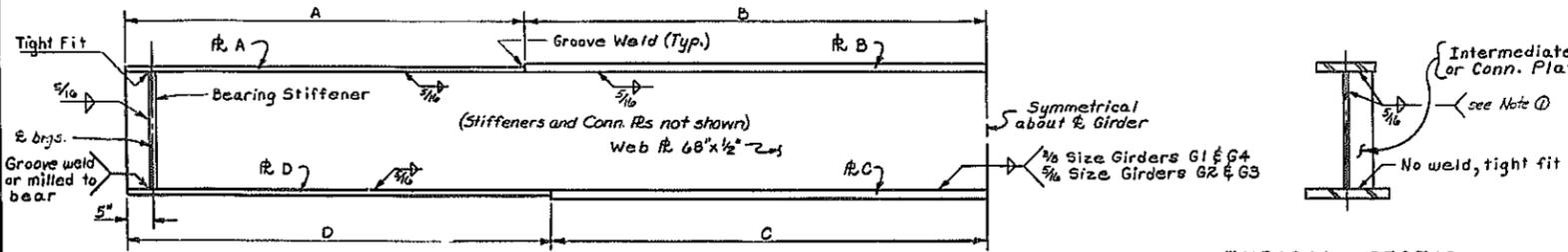
- LEGEND**
- # SUPERSTRUCTURE PHOTOS
 - # SUBSTRUCTURE PHOTOS



STEEL FRAMING PLAN (SPANS 11, 12, 13, 14 & 15 SOUTHBOUND)
Scale : 3/16" = 1'-0"



Noted: Flanges shall be A.S.T.M. designation A441 Steel.
Stiffeners, connection plates, and diaphragms shall be A.S.T.M. designation A36 Steel.



TYPICAL SECTION
Not to Scale

Note: Where connection plates are used in pairs, the fillet weld at top of stiffener is optional. See Note "Sup 4" in the Superstructure Notes.

GIRDER ELEVATION
Not to Scale

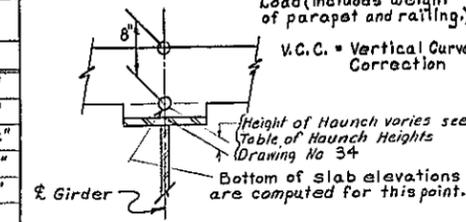
GIRDER	SPAN C. to C. BRGS.	FLANGE SIZES				FLANGE DIMENSIONS				Brq. Stiffener 2-℄s. required each end of Girder	Intermediate Stiffener Size
		RA	RB	RC	RD	A	B	C	D		
G1	123'-8 1/2"	16" x 3/4"	16" x 1 1/2"	23" x 2"	23" x 1"	28'-9 1/4"	33'-6"	40'-6"	21'-9 1/4"	9" x 7/8"	6" x 3/8"
G2	123'-6 1/16"	15" x 3/4"	13" x 1 1/2"	22" x 1 1/2"	22" x 1"	28'-8 3/8"	33'-6"	33'-6"	28'-8 3/8"	8" x 3/4"	5 1/2" x 3/8"
G3	123'-5 5/16"	15" x 3/4"	13" x 1 1/2"	22" x 1 1/2"	22" x 1"	28'-7 7/8"	33'-6"	33'-6"	28'-7 7/8"	8" x 3/4"	5 1/2" x 3/8"
G4	123'-3 1/16"	16" x 3/4"	16" x 1 1/2"	23" x 2"	23" x 1"	28'-6 7/8"	33'-6"	40'-6"	21'-6 7/8"	9" x 7/8"	6" x 3/8"

GIRDER	SPAN	℄ So. Brq.	℄ Span	℄ No. Brq.
G1	11	441.24	440.03	438.56
G2	11	441.12	439.30	438.33
G3	11	440.96	439.58	438.11
G4	11	440.74	439.35	437.88

SPAN	GIRDER	STEEL SLAB				CAMBER	
		STEEL	SLAB	S.D.L.	TOTAL	V.C.C.	TOTAL
Spans 11, 12 & 13 (S.B.)	G1 and G4	.07	.12	.08	.27	.04	.31
	G2 and G3	.08	.24	.03	.35	.04	.39
Span 14 (S.B.)	G1 and G4	.07	.12	.08	.27	-.05	.22
	G2 and G3	.08	.24	.03	.35	-.05	.30
Span 15 (S.B.)	G1 and G4	.07	.12	.08	.27	-.07	.20
	G2 and G3	.08	.24	.03	.35	-.07	.28

LOCATION	AZIMUTHS
℄ Pier #10 (S.B.)	262°-32'-11"
℄ Pier #11 (S.B.)	263°-14'-46"
℄ Pier #12 (S.B.)	263°-57'-22"
℄ Pier #13 (S.B.)	264°-39'-57"
℄ Pier #14 (S.B.)	265°-22'-33"
℄ Fixed Brqs. @ North Abut. (S.B.)	266°-05'-08"
℄ Girders, Span 11 (S.B.)	352°-53'-28"
℄ Girders, Span 12 (S.B.)	353°-36'-04"
℄ Girders, Span 13 (S.B.)	354°-18'-40"
℄ Girders, Span 14 (S.B.)	355°-01'-15"
℄ Girders, Span 15 (S.B.)	355°-43'-51"

SPAN	GIRDER	STEEL SLAB				CAMBER	
		STEEL	SLAB	S.D.L.	TOTAL	V.C.C.	TOTAL
Spans 11, 12 & 13 (S.B.)	G1 and G4	.07	.12	.08	.27	.04	.31
	G2 and G3	.08	.24	.03	.35	.04	.39
Span 14 (S.B.)	G1 and G4	.07	.12	.08	.27	-.05	.22
	G2 and G3	.08	.24	.03	.35	-.05	.30
Span 15 (S.B.)	G1 and G4	.07	.12	.08	.27	-.07	.20
	G2 and G3	.08	.24	.03	.35	-.07	.28



PROJECT ENGINEER R. Parker
IN CHARGE OF V. B. Sherman
DESIGNED BY N. A. Toppase
DESIGN CHECKED BY S. Rowe
DETAILED BY V. C. Thompson
DETAIL CHECKED BY J. J. Hill

BRIDGE NO. 2
INTERSTATE ROUTE 481 OVER DEMITT YARDS
SPANS 11 SOUTHBOUND BIN 1093571 PHOTO LOCATION PLAN

TABLE OF STATIONS						
SPAN	STATION "A"		STATION "B"		STATION "C"	
	STATION	℄	STATION	℄	STATION	℄
11 S.B.	232+63.86	℄ Pier #9 S.B.	233+88.86	℄ Pier #10 S.B.		
12 S.B.	233+88.86	℄ Pier #10 S.B.	235+13.86	℄ Pier #11 S.B.		
13 S.B.	235+13.86	℄ Pier #11 S.B.	236+38.86	℄ Pier #12 S.B.		
14 S.B.	236+38.86	℄ Pier #12 S.B.	237+63.86	℄ Pier #13 S.B.		
15 S.B.	237+63.86	℄ Pier #13 S.B.		238+88.11	℄ Fixed Brgs. No. Abut. (S.B.)	

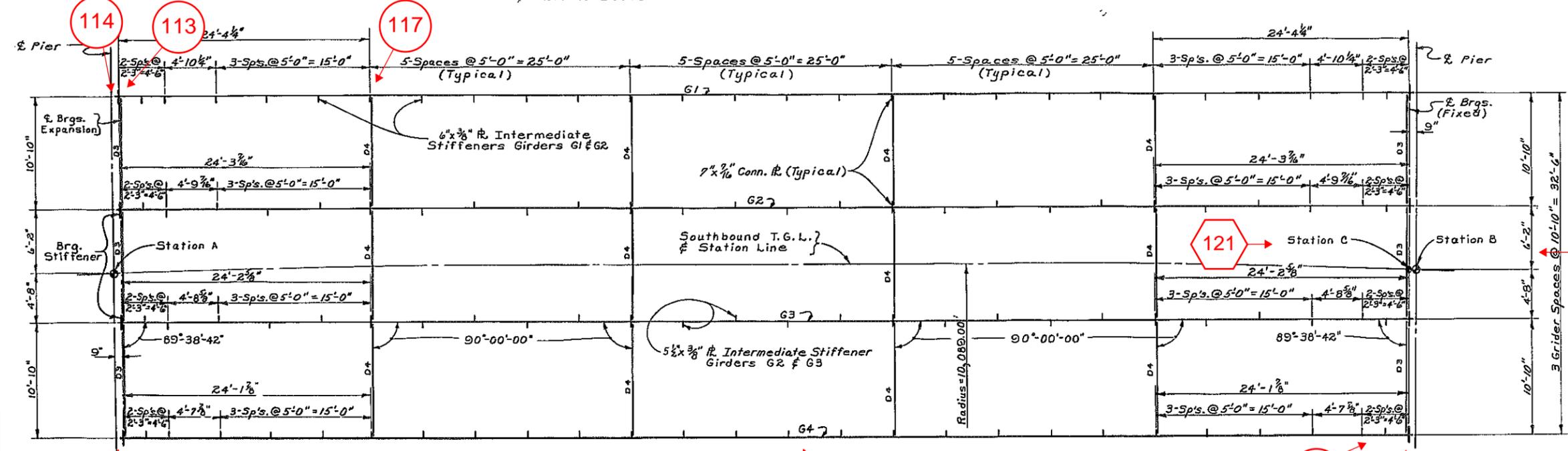
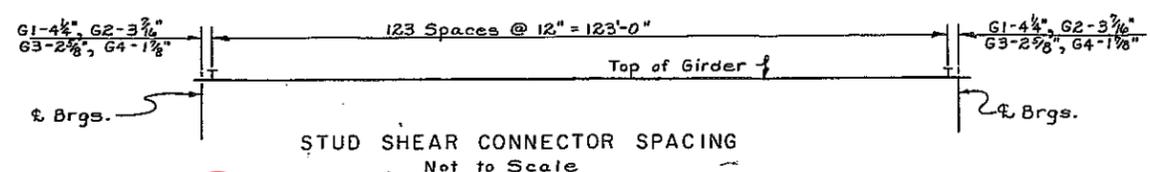
FED. RD. PROJ. NO.	STATE	FEDERAL AID PROJECT NO.	SHEET NO.	TOTAL SHEETS
1	NEW YORK	1-690-3(2B) 1-481-2(116)	206	309

INTERSTATE ROUTE CONNECTION 570
SUFFERBUT INTERCHANGE (PHASE 2)
ONONDAGA COUNTY

LEGEND

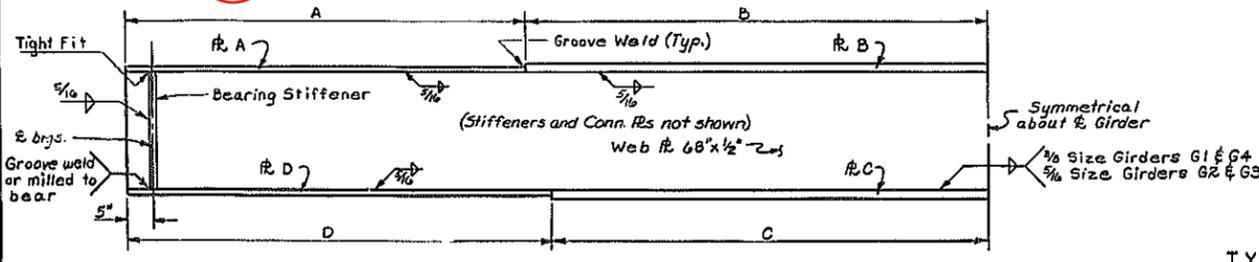
SUPERSTRUCTURE PHOTOS

SUBSTRUCTURE PHOTOS



Not a transverse section for Diaphragms.

Web & Flanges shall be A.S.T.M. designation A441 Steel.
Stiffeners, connection plates, and diaphragms shall be A.S.T.M. designation A36 Steel.



GIRDER ELEVATION
Not to Scale

THEO. BOTTOM OF SLAB ELEV. (Southbound)

GIRDER	SPAN	℄ So. Brg.	℄ Span	℄ No. Brg.
G1	11	441.24	440.03	438.56
G2	11	441.12	439.30	438.33
G3	11	440.96	439.58	438.11
G4	11	440.74	439.35	437.88

SPAN	GIRDER	DEFLECTIONS (ft.)				CAMBER	
		STEEL	SLAB	S.D.L.	TOTAL	V.C.C.	TOTAL
Spans 11, 12 & 13 (S.B.)	G1 and G4	.07	.12	.08	.27	.04	.31
	G2 and G3	.08	.24	.03	.35	.04	.39
Span 14 (S.B.)	G1 and G4	.07	.12	.08	.27	-.05	.22
	G2 and G3	.08	.24	.03	.35	-.05	.30
Span 15 (S.B.)	G1 and G4	.07	.12	.08	.27	-.07	.20
	G2 and G3	.08	.24	.03	.35	-.07	.28

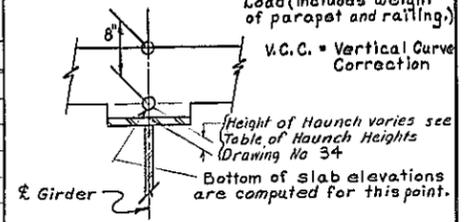
TABLE OF AZIMUTHS

LOCATION	AZIMUTHS
℄ Pier #10 (S.B.)	262°-32'-11"
℄ Pier #11 (S.B.)	263°-14'-46"
℄ Pier #12 (S.B.)	263°-57'-22"
℄ Pier #13 (S.B.)	264°-39'-57"
℄ Pier #14 (S.B.)	265°-22'-33"
℄ Fixed Brgs. @ North Abut. (S.B.)	266°-05'-08"
℄ Girders, Span 11 (S.B.)	352°-53'-28"
℄ Girders, Span 12 (S.B.)	353°-36'-04"
℄ Girders, Span 13 (S.B.)	354°-18'-40"
℄ Girders, Span 14 (S.B.)	355°-01'-15"
℄ Girders, Span 15 (S.B.)	355°-43'-51"

SOUTHBOUND GIRDER TABLE - SPANS 11 thru 15

GIRDER	SPAN C. to C. BRGS.	FLANGE SIZES				FLANGE DIMENSIONS				Brg. Stiffener 2-℄s. required each end of Girder	Intermediate Stiffener Size
		RA	RB	RC	RD	A	B	C	D		
G1	123'-8 1/2"	16" x 3/4"	16" x 1 1/2"	23" x 2"	23" x 1"	28'-9 1/4"	33'-6"	40'-6"	21'-9 1/4"	9" x 7/8"	6" x 3/8"
G2	123'-6 15/16"	15" x 3/4"	15" x 1 1/2"	22" x 1 1/2"	22" x 1"	28'-8 3/8"	33'-6"	39'-6"	28'-8 3/8"	8" x 3/4"	5 1/2" x 3/8"
G3	123'-5 5/16"	15" x 3/4"	15" x 1 1/2"	22" x 1 1/2"	22" x 1"	28'-7 7/8"	33'-6"	39'-6"	28'-7 7/8"	8" x 3/4"	5 1/2" x 3/8"
G4	123'-3 1/16"	16" x 3/4"	16" x 1 1/2"	23" x 2"	23" x 1"	28'-6 7/8"	33'-6"	40'-6"	21'-6 7/8"	9" x 7/8"	6" x 3/8"

PROJECT ENGINEER R. Parker
IN CHARGE OF V. B. Sherman
DESIGNED BY N. A. Shepperd
DESIGN CHECKED BY S. Rowe
DETAILED BY V. C. Thompson
DETAIL CHECKED BY J. J. Hill



BRIDGE NO. 2
INTERSTATE ROUTE 481 OVER DEWITT YARDS

SPANS 12 SOUTHBOUND BIN 1093571 PHOTO LOCATION PLAN

TABLE OF STATIONS

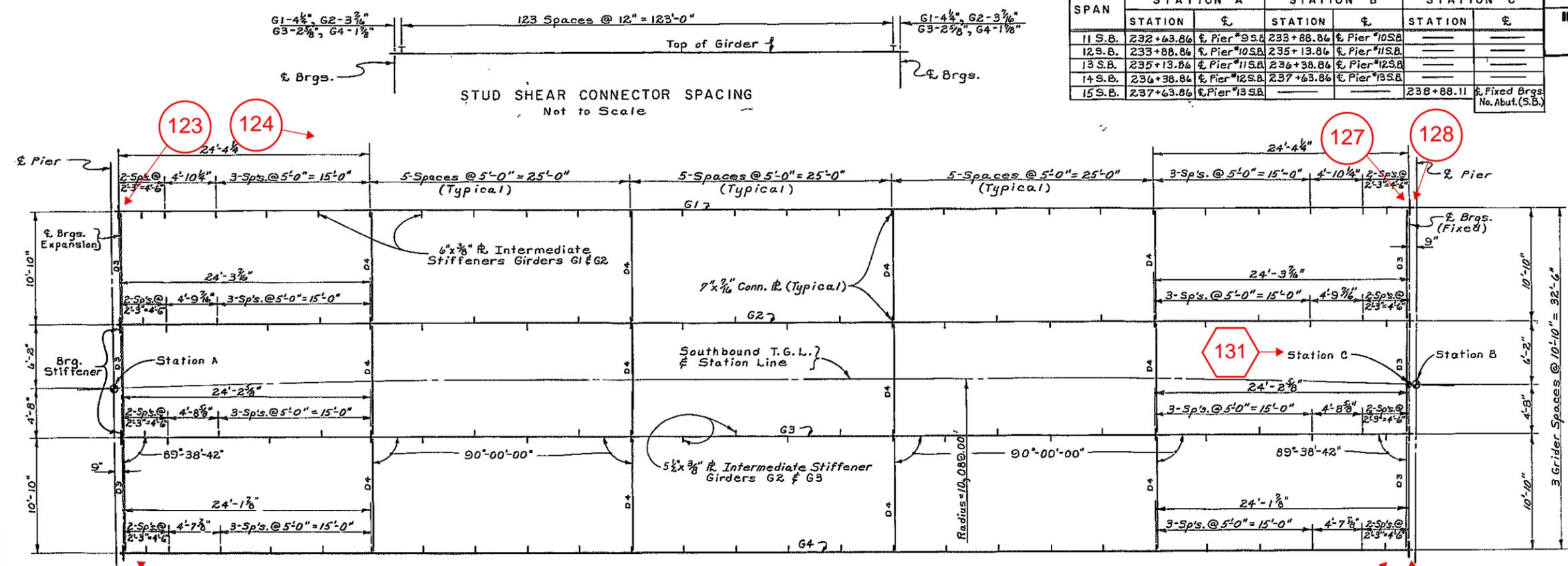
SPAN	STATION "A"		STATION "B"		STATION "C"	
	STATION	℄	STATION	℄	STATION	℄
11 S.B.	232+63.86	℄ Pier #9 S.B.	233+88.86	℄ Pier #10 S.B.		
12 S.B.	233+88.86	℄ Pier #10 S.B.	235+13.86	℄ Pier #11 S.B.		
13 S.B.	235+13.86	℄ Pier #11 S.B.	236+38.86	℄ Pier #12 S.B.		
14 S.B.	236+38.86	℄ Pier #12 S.B.	237+63.86	℄ Pier #13 S.B.		
15 S.B.	237+63.86	℄ Pier #13 S.B.		238+88.11	℄ Fixed Brgs. No. Abut. (S.B.)	

FED. RD. REG. NO.	STATE	FEDERAL AID PROJECT NO.	SHEET NO.	TOTAL SHEETS
1	NEW YORK	1-690-3(2B) 1-481-2(116)	206	309

INTERSTATE ROUTE CONNECTION 570
SUFFERBUT INTERCHANGE (PHASE 2)
ONEIDA COUNTY

LEGEND

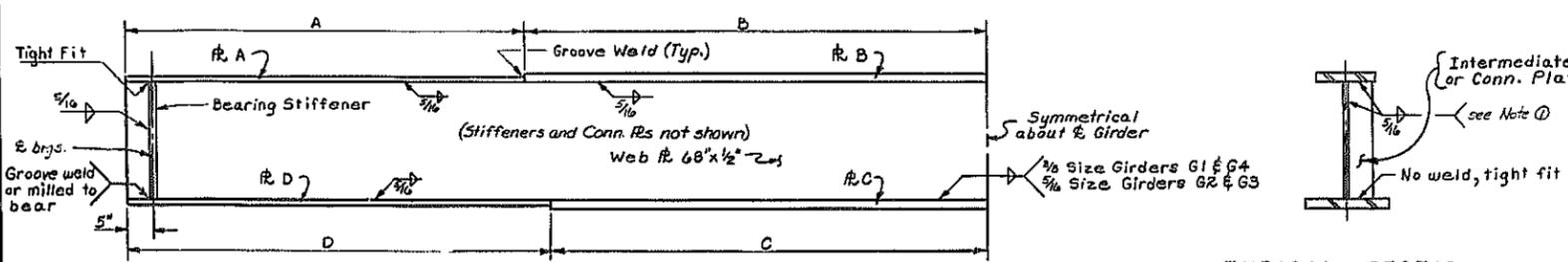
- # SUPERSTRUCTURE PHOTOS
- # SUBSTRUCTURE PHOTOS



STEEL FRAMING PLAN (SPANS 11, 12, 13, 14 & 15 SOUTHBOUND)
Scale : 3/16" = 1'-0"

Note: Weir shall be A.S.T.M. designation A441 Steel.
Stiffeners, connection plates, and diaphragms shall be A.S.T.M. designation A36 Steel.

Note: See Transverse Diaphragms.



TYPICAL SECTION
Not to Scale

Note: Where connection plates are used in pairs, the fillet weld at top of stiffener is optional. See Note "Sup 4" in the Superstructure Notes.

GIRDER ELEVATION
Not to Scale

SOUTHBOUND GIRDER TABLE - SPANS 11 thru 15

GIRDER	SPAN C. to C. BRGS.	FLANGE SIZES				FLANGE DIMENSIONS				Brg. Stiffener 2-℄s. required each end of Girder	Intermediate Stiffener Size
		RA	RB	RC	RD	A	B	C	D		
G1	123'-8 1/2"	16" x 3/4"	16" x 1 1/2"	23" x 2"	23" x 1"	28'-9 1/4"	33'-6"	40'-6"	21'-9 1/4"	9" x 7/8"	6" x 3/8"
G2	123'-6 15/16"	15" x 3/4"	13" x 1 1/2"	22" x 1 1/2"	22" x 1"	28'-8 3/8"	33'-6"	33'-6"	28'-8 3/8"	8" x 3/4"	5 1/2" x 3/8"
G3	123'-5 9/16"	15" x 3/4"	13" x 1 1/2"	22" x 1 1/2"	22" x 1"	28'-7 7/8"	33'-6"	33'-6"	28'-7 7/8"	8" x 3/4"	5 1/2" x 3/8"
G4	123'-3 1/16"	16" x 3/4"	16" x 1 1/2"	23" x 2"	23" x 1"	28'-6 7/8"	33'-6"	40'-6"	21'-6 7/8"	9" x 7/8"	6" x 3/8"

THEO. BOTTOM OF SLAB ELEV. (Southbound)

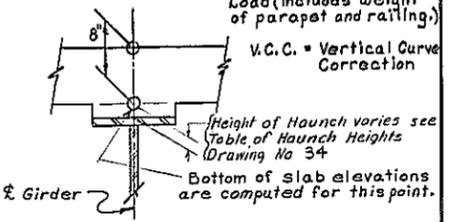
GIRDER	SPAN	℄ So. Brg.	℄ Span	℄ No. Brg.
G1	11	441.24	440.03	438.56
G2	11	441.12	439.30	438.33
G3	11	440.96	439.58	438.11
G4	11	440.74	439.35	437.88

DEFLECTIONS (ft.)

SPAN	GIRDER	STEEL SLAB S/DL				CAMBER	
		STEEL	SLAB	S/DL	TOTAL	V.C.C.	TOTAL
Spans 11, 12 & 13 (S.B.)	G1 and G4	.07	.12	.08	.27	.04	.31
	G2 and G3	.08	.24	.03	.35	.04	.39
Span 14 (S.B.)	G1 and G4	.07	.12	.08	.27	-.05	.22
	G2 and G3	.08	.24	.03	.35	-.05	.30
Span 15 (S.B.)	G1 and G4	.07	.12	.08	.27	-.07	.20
	G2 and G3	.08	.24	.03	.35	-.07	.28

TABLE OF AZIMUTHS

LOCATION	AZIMUTHS
℄ Pier #10 (S.B.)	262°-32'-11"
℄ Pier #11 (S.B.)	263°-14'-46"
℄ Pier #12 (S.B.)	263°-57'-22"
℄ Pier #13 (S.B.)	264°-39'-57"
℄ Pier #14 (S.B.)	265°-22'-33"
℄ Fixed Brgs. @ North Abut. (S.B.)	266°-05'-08"
℄ Girders, Span 11 (S.B.)	352°-53'-28"
℄ Girders, Span 12 (S.B.)	353°-36'-04"
℄ Girders, Span 13 (S.B.)	354°-18'-40"
℄ Girders, Span 14 (S.B.)	355°-01'-15"
℄ Girders, Span 15 (S.B.)	355°-43'-51"



PROJECT ENGINEER R. Parker
IN CHARGE OF V. B. Sherman
DESIGNED BY N. A. Spitzer
DESIGN CHECKED BY S. Rowe
DETAILED BY V. C. Thompson
DETAIL CHECKED BY J. J. Hill

BRIDGE NO. 2
INTERSTATE ROUTE 481 OVER DEWITT YARDS
SPANS 13 SOUTHBOUND BIN 1093571 PHOTO LOCATION PLAN

SPAN	STATION "A"		STATION "B"		STATION "C"	
	STATION	℄	STATION	℄	STATION	℄
11 S.B.	232+63.86	℄ Pier #9 S.B.	233+88.86	℄ Pier #10 S.B.		
12 S.B.	233+88.86	℄ Pier #10 S.B.	235+13.86	℄ Pier #11 S.B.		
13 S.B.	235+13.86	℄ Pier #11 S.B.	236+38.86	℄ Pier #12 S.B.		
14 S.B.	236+38.86	℄ Pier #12 S.B.	237+63.86	℄ Pier #13 S.B.		
15 S.B.	237+63.86	℄ Pier #13 S.B.		238+88.11	℄ Fixed Brgs. No. Abut. (S.B.)	

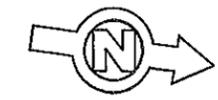
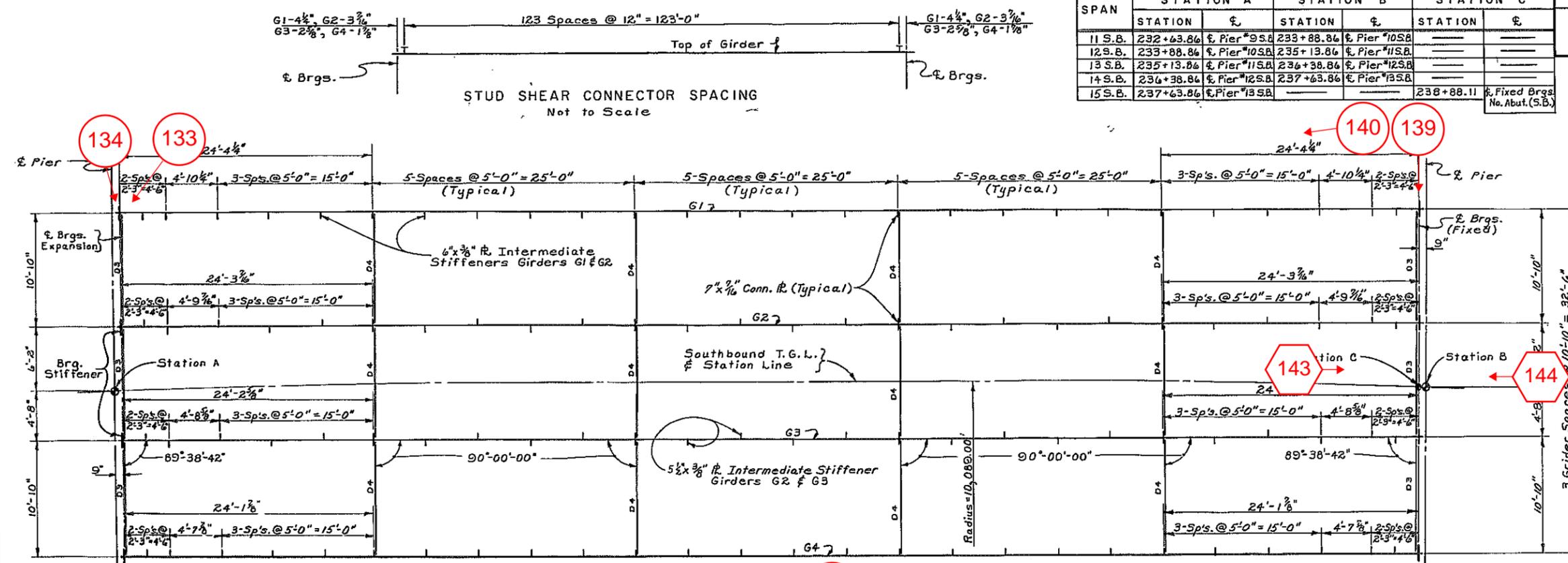
FED. RD. PROJ. NO.	STATE	FEDERAL AID PROJECT NO.	SHEET NO.	TOTAL SHEETS
1	NEW YORK	1-690-3(2B) 1-481-2(116)	206	309

INTERSTATE ROUTE CONNECTION 570
SUFFERNUT INTERCHANGE (PHASE 2)
ONONDAGA COUNTY

LEGEND

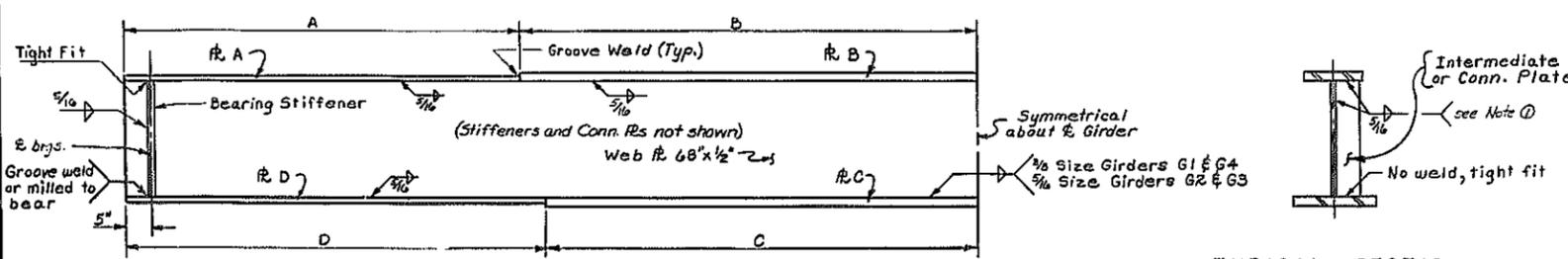
SUPERSTRUCTURE PHOTOS

SUBSTRUCTURE PHOTOS



STEEL FRAMING PLAN (Spans 11, 12, 13, 14 & 15 SOUTHBOUND)
Scale: 1/4" = 1'-0"

Note: All steel shall be A.S.T.M. designation A441 Steel.
Stiffeners, connection plates, and diaphragms shall be A.S.T.M. designation A36 Steel.



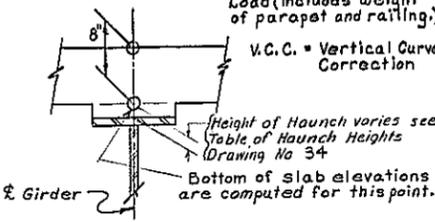
TYPICAL SECTION
Not to Scale

Note: Where connection plates are used in pairs, the fillet weld at top of stiffener is optional. See Note "Sup 4" in the Superstructure Notes.

GIRDER	SPAN	℄ So. Brg.	℄ Span	℄ No. Brg.
G1	11	441.24	440.03	438.56
G2	11	441.12	439.30	438.33
G3	11	440.96	439.58	438.11
G4	11	440.74	439.35	437.88

SPAN	GIRDER	DEFLECTIONS (ft.)				CAMBER	
		STEEL	SLAB	S.D.L.	TOTAL	V.C.C.	TOTAL
Spans 11, 12 & 13 (S.B.)	G1 and G4	.07	.12	.06	.27	.04	.31
	G2 and G3	.08	.24	.03	.35	.04	.39
Span 14 (S.B.)	G1 and G4	.07	.12	.06	.27	-.05	.22
	G2 and G3	.08	.24	.03	.35	-.05	.30
Span 15 (S.B.)	G1 and G4	.07	.12	.06	.27	-.07	.20
	G2 and G3	.08	.24	.03	.35	-.07	.28

LOCATION	AZIMUTHS
℄ Pier #10 (S.B.)	262°-32'-11"
℄ Pier #11 (S.B.)	263°-14'-46"
℄ Pier #12 (S.B.)	263°-57'-22"
℄ Pier #13 (S.B.)	264°-39'-57"
℄ Pier #14 (S.B.)	265°-22'-33"
℄ Fixed Brgs. @ North Abut. (S.B.)	266°-05'-08"
℄ Girders, Span 11 (S.B.)	352°-53'-28"
℄ Girders, Span 12 (S.B.)	353°-36'-04"
℄ Girders, Span 13 (S.B.)	354°-18'-40"
℄ Girders, Span 14 (S.B.)	355°-01'-15"
℄ Girders, Span 15 (S.B.)	355°-43'-51"



GIRDER	SPAN C. to C. BRGS.	FLANGE SIZES				FLANGE DIMENSIONS				Brg. Stiffener 2-℄s. required each end of Girder	Intermediate Stiffener Size
		RA	RB	RC	RD	A	B	C	D		
G1	123'-8 1/2"	16" x 3/4"	16" x 1 1/2"	23" x 2"	23" x 1"	28'-9 1/4"	33'-6"	40'-6"	21'-9 1/4"	9" x 7/8"	6" x 3/8"
G2	123'-6 15/16"	15" x 3/4"	15" x 1 1/2"	22" x 1 1/2"	22" x 1"	28'-8 3/8"	33'-6"	33'-6"	28'-8 3/8"	8" x 3/4"	5 1/2" x 3/8"
G3	123'-5 9/16"	15" x 3/4"	15" x 1 1/2"	22" x 1 1/2"	22" x 1"	28'-7 7/8"	33'-6"	33'-6"	28'-7 7/8"	8" x 3/4"	5 1/2" x 3/8"
G4	123'-3 1/16"	16" x 3/4"	16" x 1 1/2"	23" x 2"	23" x 1"	28'-6 7/8"	33'-6"	40'-6"	21'-6 7/8"	9" x 7/8"	6" x 3/8"

PROJECT ENGINEER: R. Parker
 IN CHARGE OF: V. B. Sherman
 DESIGNED BY: N. A. Thompson
 DESIGN CHECKED BY: S. Rowe
 DETAILED BY: V. C. Thompson
 DETAIL CHECKED BY: J. L. Hill

BRIDGE NO. 2
 INTERSTATE ROUTE 481 OVER DEWITT YARDS
SPANS 14 SOUTHBOUND BIN 1093571 PHOTO LOCATION PLAN

TABLE OF STATIONS

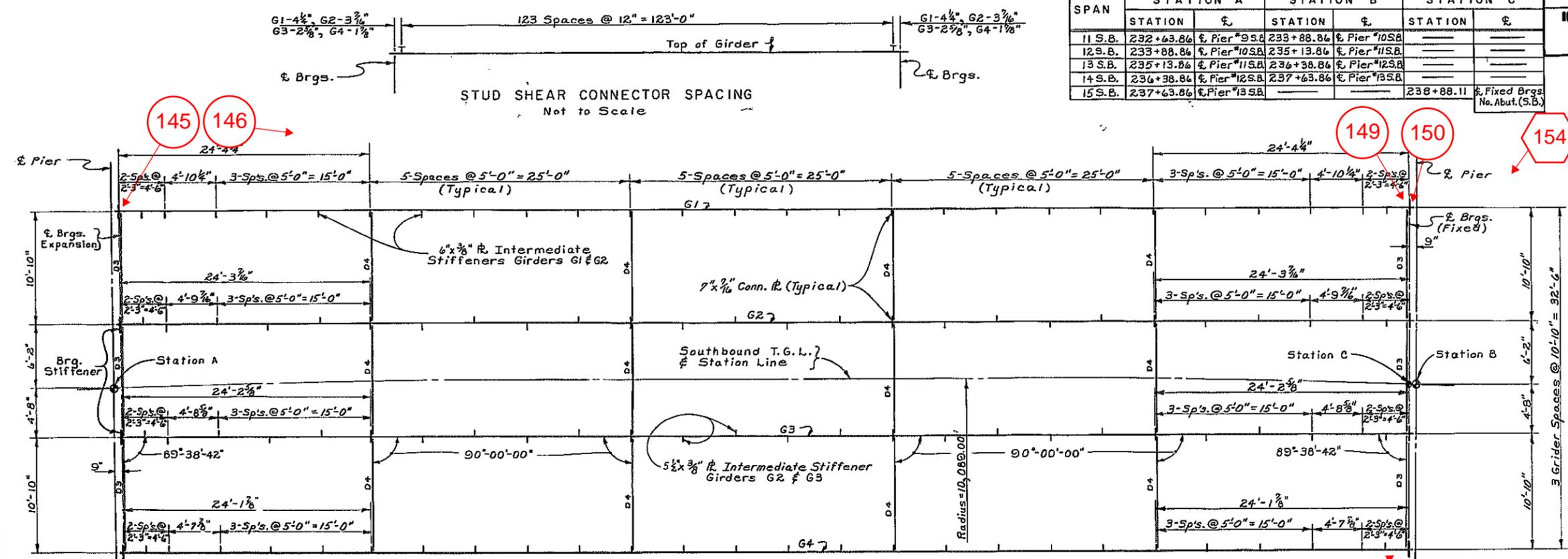
SPAN	STATION "A"		STATION "B"		STATION "C"	
	STATION	℄	STATION	℄	STATION	℄
11 S.B.	232+63.86	℄ Pier #9 S.B.	233+88.86	℄ Pier #10 S.B.		
12 S.B.	233+88.86	℄ Pier #10 S.B.	235+13.86	℄ Pier #11 S.B.		
13 S.B.	235+13.86	℄ Pier #11 S.B.	236+38.86	℄ Pier #12 S.B.		
14 S.B.	236+38.86	℄ Pier #12 S.B.	237+63.86	℄ Pier #13 S.B.		
15 S.B.	237+63.86	℄ Pier #13 S.B.		238+88.11	℄ Fixed Brgs. No. Abut. (S.B.)	

FED. RD. AID PROJECT NO.	STATE	FEDERAL AID PROJECT NO.	SHEET NO.	TOTAL SHEETS
1-481-2(116)	NEW YORK	1-690-3(28)	206	309

INTERSTATE ROUTE CONNECTION 570
SUFFERNUT INTERCHANGE (PHASE 2)
ONDAGA COUNTY

LEGEND

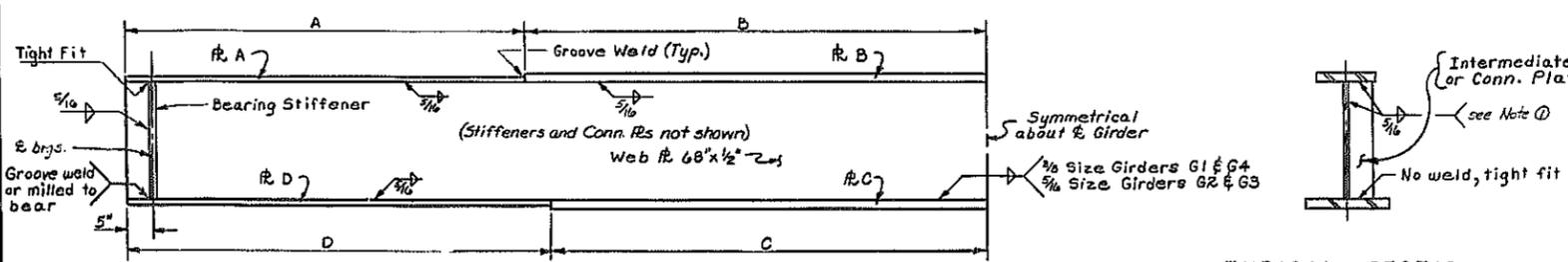
- # SUPERSTRUCTURE PHOTOS
- # SUBSTRUCTURE PHOTOS



STEEL FRAMING PLAN (SPANS 11, 12, 13, 14 & 15 SOUTHBOUND)
Scale : 3/16" = 1'-0"

Note: See Transverse Section for Diaphragms.

Web and flanges shall be A.S.T.M. designation A441 Steel.
Stiffeners, Connect. Plates, and diaphragms shall be A.S.T.M. designation A572 Steel.



TYPICAL SECTION
Not to Scale

Note: Where connection plates are used in pairs, the fillet weld at top of stiffener is optional. See Note "Sup 4" in the Superstructure Notes.

GIRDER ELEVATION
Not to Scale

SOUTHBOUND GIRDER TABLE - SPANS 11 thru 15

GIRDER	SPAN C. to C. BRGS.	FLANGE SIZES				FLANGE DIMENSIONS				Brq. Stiffener 2-℄s. required each end of Girder	Intermediate Stiffener Size
		RA	RB	RC	RD	A	B	C	D		
G1	123'-8 1/2"	16" x 3/4"	16" x 1 1/2"	23" x 2"	23" x 1"	28'-9 1/4"	33'-6"	40'-6"	21'-9 1/4"	9" x 7/8"	6" x 3/8"
G2	123'-6 1/16"	15" x 3/4"	13" x 1 1/2"	22" x 1 1/2"	22" x 1"	28'-8 3/8"	33'-6"	33'-6"	28'-8 3/8"	8" x 3/4"	5 1/2" x 3/8"
G3	123'-5 5/16"	15" x 3/4"	13" x 1 1/2"	22" x 1 1/2"	22" x 1"	28'-7 7/8"	33'-6"	33'-6"	28'-7 7/8"	8" x 3/4"	5 1/2" x 3/8"
G4	123'-3 1/16"	16" x 3/4"	16" x 1 1/2"	23" x 2"	23" x 1"	28'-6 7/8"	33'-6"	40'-6"	21'-6 7/8"	9" x 7/8"	6" x 3/8"

THEO. BOTTOM OF SLAB ELEV. (Southbound)

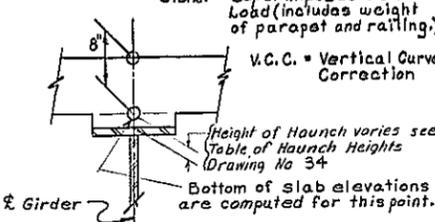
GIRDER	SPAN	℄ So. Brq.	℄ Span	℄ No. Brq.
G1	11	441.24	440.03	438.56
G2	11	441.12	439.30	438.33
G3	11	440.96	439.58	438.11
G4	11	440.74	439.35	437.88

TABLE OF AZIMUTHS

LOCATION	AZIMUTHS
℄ Pier #10 (S.B.)	262°-32'-11"
℄ Pier #11 (S.B.)	263°-14'-46"
℄ Pier #12 (S.B.)	263°-57'-22"
℄ Pier #13 (S.B.)	264°-39'-57"
℄ Pier #14 (S.B.)	265°-22'-33"
℄ Fixed Brqs. @ North Abut. (S.B.)	266°-05'-08"
℄ Girders, Span 11 (S.B.)	352°-53'-28"
℄ Girders, Span 12 (S.B.)	353°-36'-04"
℄ Girders, Span 13 (S.B.)	354°-18'-40"
℄ Girders, Span 14 (S.B.)	355°-01'-15"
℄ Girders, Span 15 (S.B.)	355°-43'-51"

DEFLECTIONS (ft.)

SPAN	GIRDER	STEEL SLAB			TOTAL	CAMBER	
		STEEL	SLAB	SDL		V.C.C.	TOTAL
Spans 11, 12 & 13 (S.B.)	G1 and G4	.07	.12	.08	.27	.04	.31
	G2 and G3	.08	.24	.03	.35	.04	.39
Span 14 (S.B.)	G1 and G4	.07	.12	.08	.27	-.05	.22
	G2 and G3	.08	.24	.03	.35	-.05	.30
Span 15 (S.B.)	G1 and G4	.07	.12	.08	.27	-.07	.20
	G2 and G3	.08	.24	.03	.35	-.07	.28



PROJECT ENGINEER R. Parker
IN CHARGE OF V. B. Sherman
DESIGNED BY N. A. Toppase
DESIGN CHECKED BY S. Rowe
DETAILED BY V. C. Thompson
DETAIL CHECKED BY J. J. Hill

BRIDGE NO. 2
INTERSTATE ROUTE 481 OVER DEWITT YARDS
SPANS 15 SOUTHBOUND BIN 1093571
PHOTO LOCATION PLAN



C&S Engineers, Inc.
499 Col. Eileen Collins Blvd.
Syracuse, New York 13212
Phone: (315) 455-2000
Fax: (315) 455-9667
www.cscos.com

I-481 SB over CSX BIN 1093571



DESCRIPTION: BEGIN ABUTMENT

PHOTO 1



DESCRIPTION: BEGIN ABUTMENT – LEFT WINGWALL
MAP CRACKING ALONG ENTIRE FACE

PHOTO 2



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I-481 SB over CSX BIN 1093571



DESCRIPTION: SPAN 1: GIRDER 1 (BEGIN)

PHOTO 3



DESCRIPTION: SPAN 1: GIRDER 4 (BEGIN)

PHOTO 4



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Sheet 3 of 77

I-481 SB over CSX BIN 1093571



DESCRIPTION: SPAN 1: GIRDER 1 (END) – FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 5



DESCRIPTION: SPAN 1: GIRDER 1 (END) – FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 6



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Sheet 4 of 77

I-481 SB over CSX BIN 1093571



DESCRIPTION: PIER NO. 1 – BEGIN FACE

PHOTO 7



DESCRIPTION: PIER NO. 1 – END FACE

PHOTO 8



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Sheet 5 of 77

I-481 SB over CSX BIN 1093571



DESCRIPTION: SPAN 2: GIRDER 1 (BEGIN) – FASCIA

PHOTO 9



DESCRIPTION: SPAN 2: GIRDER 3 (BEGIN) -
DIAPHRAGM AND WEB OVERHANG
DETERIORATION

PHOTO 10



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Sheet 6 of 77

I-481 SB over CSX BIN 1093571



DESCRIPTION: SPAN 2: GIRDER 4 (BEGIN) – EXTERIOR
WEB AND STIFFENER DETERIORATION

PHOTO 11



DESCRIPTION: SPAN 2: GIRDER 4 – FASCIA
FLANGE DETERIORATION

PHOTO 12



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Sheet 7 of 77

I-481 SB over CSX BIN 1093571



DESCRIPTION: SPAN 2: GIRDER 1 (END) – FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 13



DESCRIPTION: SPAN 2: GIRDER 4 (END) – FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 14



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Sheet 8 of 77

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DESCRIPTION: PIER NO. 2 – BEGIN FACE

PHOTO 15



DESCRIPTION: PIER NO. 2 – END FACE

PHOTO 16



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DESCRIPTION: SPAN 3: GIRDER 1 (BEGIN) – FASCIA
WEB OVERHANG AND STIFFENER
DETERIORATION

PHOTO 17



DESCRIPTION: SPAN 3: GIRDER 1 – FASCIA
FLANGE DETERIORATION

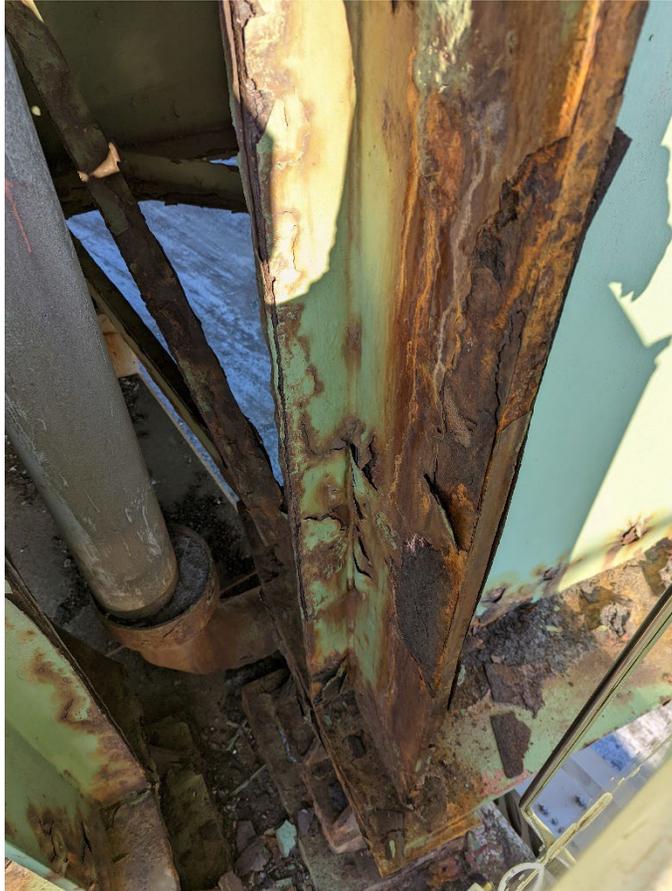
PHOTO 18



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Sheet 10 of 77

I-481 SB over CSX BIN 1093571



DESCRIPTION: SPAN 3: GIRDER 4 (BEGIN) - FASCIA
END OF WEB AND STIFFENER
DETERIORATION

PHOTO 19



DESCRIPTION: SPAN 3: GIRDER 4 - FASCIA
FLANGE DETERIORATION

PHOTO 20



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Sheet 11 of 77

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DESCRIPTION: SPAN 3: GIRDER 1 (END) - FASCIA
HOLE IN WEB

PHOTO 21



DESCRIPTION: SPAN 3: GIRDER 1 (END) - INTERIOR
HOLES IN END DIAPHRAGM BRACING

PHOTO 22



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Sheet 12 of 77

I-481 SB over CSX BIN 1093571



DESCRIPTION: SPAN 3: GIRDER 4 (END) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 23



DESCRIPTION: SPAN 3: GIRDER 4 – FASCIA
FLANGE DETERIORATION

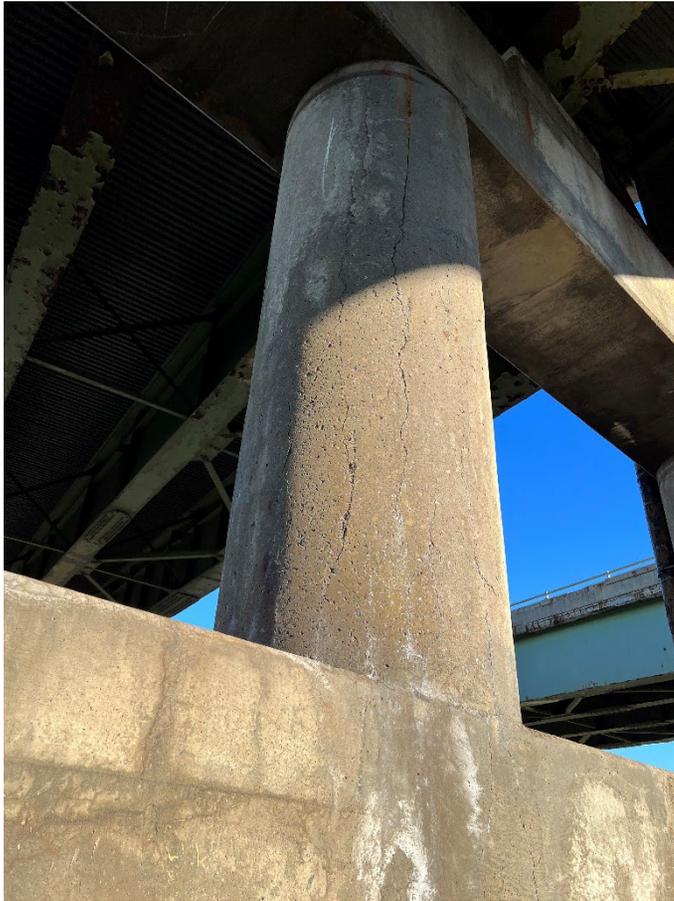
PHOTO 24



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Sheet 13 of 77

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DESCRIPTION: PIER NO. 3 – BEGIN FACE

PHOTO 25



DESCRIPTION: PIER NO. 3 – END FACE

PHOTO 26



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Sheet 14 of 77

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DESCRIPTION: SPAN 4: GIRDER 1 (BEGIN) - FASCIA
HOLE IN WEB

PHOTO 27



DESCRIPTION: SPAN 4: GIRDER 1 – FASCIA
WEB AND FLANGE DETERIORATION

PHOTO 28



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Sheet 15 of 77

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DESCRIPTION: SPAN 4: GIRDER 2 (BEGIN) - INTERIOR
WEB CRIPPLE

PHOTO 29



DESCRIPTION: SPAN 4: GIRDER BAY 2 (BEGIN)
END DIAPHRAGM DETERIORATION

PHOTO 30



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Sheet 16 of 77

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DESCRIPTION: SPAN 4: GIRDER 4 (BEGIN) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 31



DESCRIPTION: SPAN 4: GIRDER 4 (MID) - FASCIA
SPLICE AND FLANGE DETERIORATION

PHOTO 32



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Sheet 17 of 77

I-481 SB over CSX BIN 1093571



DESCRIPTION: SPAN 4: GIRDER 1 (FLTR) - FASCIA
FLANGE DETERIORATION AT MIDSPAN

PHOTO 33



DESCRIPTION: SPAN 4: GIRDER 4 (FLTR) - FASCIA
FLANGE DETERIORATION AT MIDSPAN

PHOTO 34



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Sheet 18 of 77

I-481 SB over CSX BIN 1093571



DESCRIPTION: SPAN 4: GIRDER 1 (END) - FASCIA
FLANGE DETERIORATION AT MIDSPAN

PHOTO 35



DESCRIPTION: SPAN 4: GIRDER 2 (END) - INTERIOR
WEB AND STIFFENER DETERIORATION

PHOTO 36



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Sheet 19 of 77

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DESCRIPTION: PIER NO. 4 – BEGIN FACE

PHOTO 37



DESCRIPTION: PIER NO. 4 – END FACE

PHOTO 38



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DESCRIPTION: SPAN 5: GIRDER 1 (BEGIN) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 39



DESCRIPTION: SPAN 5: GIRDER 1 (BEGIN) – INTERIOR
INTERIOR WEB AND STIFFENER
DETERIORATION

PHOTO 40



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Sheet 21 of 77

I-481 SB over CSX BIN 1093571



DESCRIPTION: SPAN 5: GIRDER 1 – FASCIA
FLANGE DETERIORATION

PHOTO 41



DESCRIPTION: SPAN 5: GIRDER 4 (MID) - FASCIA
WEB AND FLANGE DETERIORATION

PHOTO 42



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DESCRIPTION: SPAN 5: GIRDER 1 (END) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 43



DESCRIPTION: SPAN 5: GIRDER 1 – FASCIA
FLANGE DETERIORATION

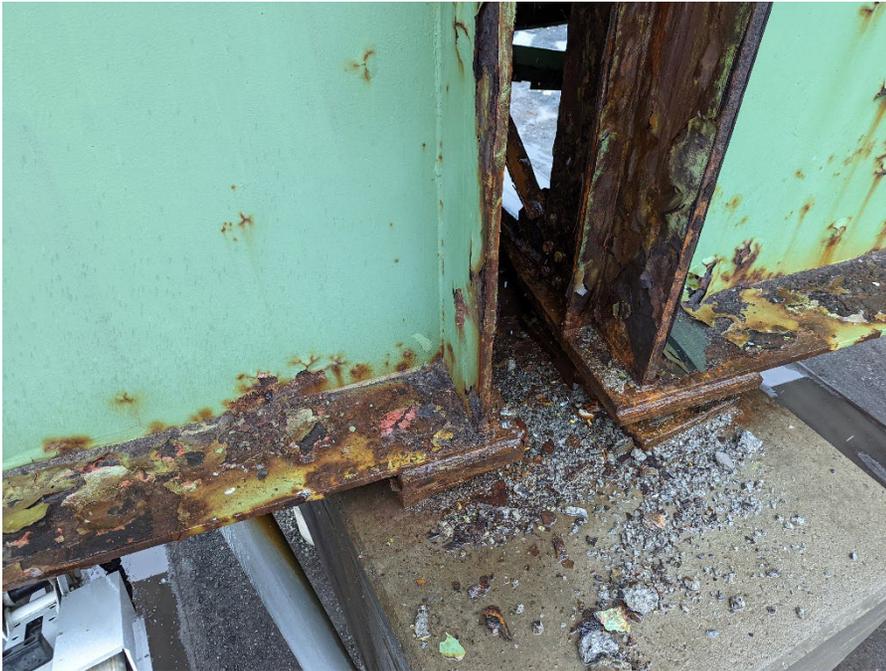
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Sheet 23 of 77

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DESCRIPTION: SPAN 5: GIRDER 4 (END) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 45



DESCRIPTION: SPAN 5: GIRDER 4 – FASCIA
FLANGE DETERIORATION

PHOTO 46



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Sheet 24 of 77

I-481 SB over CSX BIN 1093571



DESCRIPTION: PIER NO. 5 – BEGIN FACE

PHOTO 47



DESCRIPTION: PIER NO. 5 – END FACE

PHOTO 48



DESCRIPTION: SPAN 6: GIRDER 1 (BEGIN) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 49



DESCRIPTION: SPAN 6: GIRDER 1 (BEGIN) - FASCIA
FLANGE DETERIORATION

PHOTO 50



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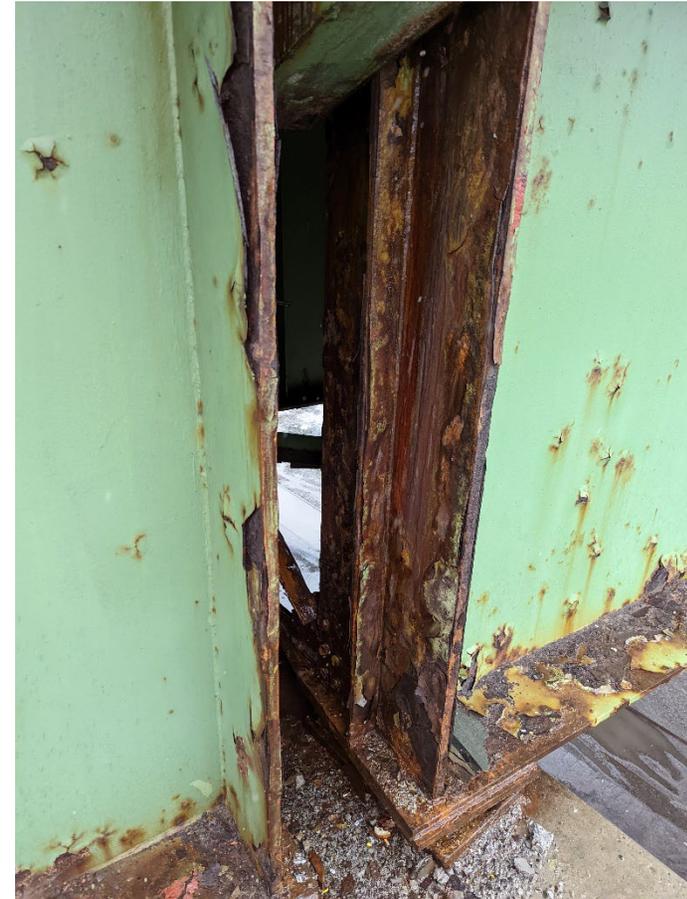
Sheet 26 of 77

I-481 SB over CSX BIN 1093571



DESCRIPTION: SPAN 6: GIRDER BAY 1 (BEGIN) –
LOOKING SOUTHWEST
END DIAPHRAGM DETERIORATION

PHOTO 51



DESCRIPTION: SPAN 6: GIRDER 4 (BEGIN) – FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 52



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Sheet 27 of 77

I-481 SB over CSX BIN 1093571



DESCRIPTION: SPAN 6: GIRDER 1 - FASCIA
FLANGE DETERIORATION

PHOTO 53



DESCRIPTION: SPAN 6: GIRDER 1 (END) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 54



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DESCRIPTION: SPAN 6: GIRDER 4 (END)
WEB AND STIFFENER DETERIORATION

PHOTO 55



DESCRIPTION: SPAN 6: GIRDER 4 FASCIA
WEB AND FLANGE DETERIORATION

PHOTO 56



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DESCRIPTION: PIER NO. 6 – BEGIN FACE

PHOTO 57



DESCRIPTION: PIER NO. 6 – END FACE

PHOTO 58



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DESCRIPTION: SPAN 7: GIRDER 1 (BEGIN) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 59



DESCRIPTION: SPAN 7: GIRDER BAY 2 (BEGIN)
END DIAPHRAGM BUCKLED

PHOTO 60



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Sheet 31 of 77

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DESCRIPTION: SPAN 7: GIRDER 4 (BEGIN) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 61



DESCRIPTION: SPAN 7: GIRDER 4 – FASCIA
FLANGE DETERIORATION

PHOTO 62



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DESCRIPTION: SPAN 7: GIRDER 1 (END) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 63



DESCRIPTION: SPAN 7: GIRDER 1 (END) - INTERIOR
WEB AND STIFFENER DETERIORATION

PHOTO 64



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DESCRIPTION: SPAN 7: GIRDER 4 (END) – FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 65



DESCRIPTION: SPAN 7: GIRDER 4 – FASCIA
FLANGE DETERIORATION

PHOTO 66



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DESCRIPTION: PIER NO. 7 – BEGIN FACE

PHOTO 67



DESCRIPTION: PIER NO. 7 – END FACE

PHOTO 68



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Sheet 35 of 77

I-481 SB over CSX BIN 1093571



DESCRIPTION: SPAN 8: GIRDER 1 (BEGIN) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 69



DESCRIPTION: SPAN 8: GIRDER 1 – FASCIA
FLANGE DETERIORATION

PHOTO 70



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I-481 SB over CSX BIN 1093571



DESCRIPTION: SPAN 8: GIRDER 4 (BEGIN) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 71



DESCRIPTION: SPAN 8: GIRDER 4 – FASCIA
FLANGE DETERIORATION

PHOTO 72



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I-481 SB over CSX BIN 1093571



DESCRIPTION: SPAN 8: GIRDER 1 (END) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 73



DESCRIPTION: SPAN 8: GIRDER 1 – FASCIA
FLANGE DETERIORATION

PHOTO 74



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DESCRIPTION: SPAN 8: GIRDER 4 (END) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 75



DESCRIPTION: SPAN 7: GIRDER 4 – FASCIA
FLANGE DETERIORATION

PHOTO 76



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Sheet 39 of 77

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DESCRIPTION: PIER NO. 8 – BEGIN FACE

PHOTO 77



DESCRIPTION: PIER NO. 8 – END FACE

PHOTO 78



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DESCRIPTION: SPAN 9: GIRDER 1 (BEGIN) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 79



DESCRIPTION: SPAN 9: GIRDER 1 – FASCIA
FLANGE DETERIORATION

PHOTO 80



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DESCRIPTION: SPAN 9: GIRDER 4 (BEGIN) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 81



DESCRIPTION: SPAN 9: GIRDER 4 – FASCIA
FLANGE DETERIORATION

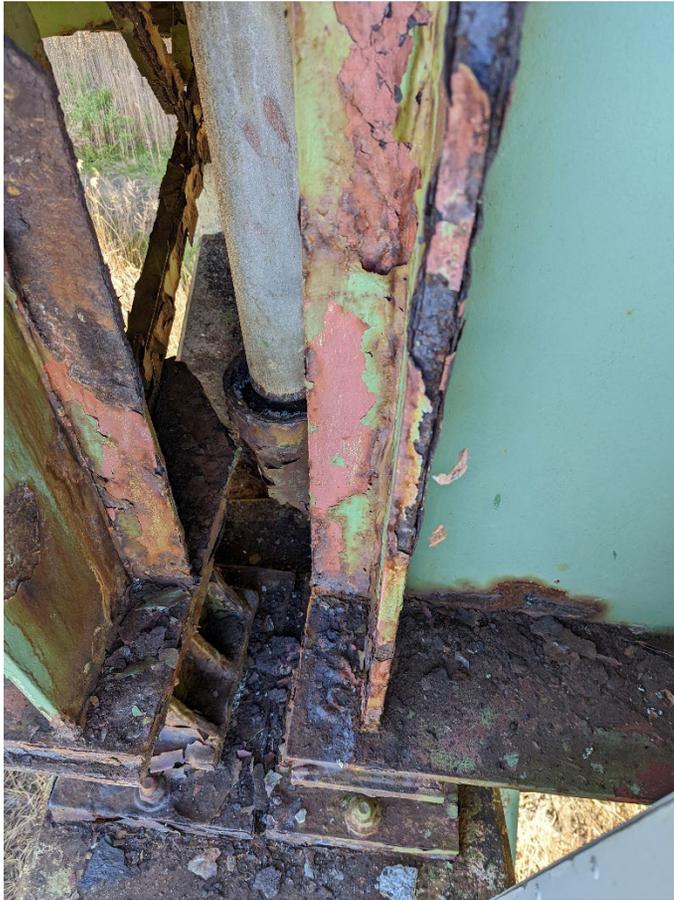
PHOTO 82



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DESCRIPTION: SPAN 9: GIRDER 1 (END) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 83



DESCRIPTION: SPAN 9: GIRDER 1 – FASCIA
FLANGE DETERIORATION

PHOTO 84



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DESCRIPTION: SPAN 9: GIRDER 4 (END) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 85



DESCRIPTION: SPAN 9: GIRDER 4 – FASCIA
FLANGE DETERIORATION

PHOTO 86



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DESCRIPTION: PIER NO. 9 – BEGIN FACE

PHOTO 87



DESCRIPTION: PIER NO. 9 – END FACE

PHOTO 88



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Sheet 45 of 77

I-481 SB over CSX BIN 1093571



DESCRIPTION: SPAN 10: GIRDER 1 (BEGIN) - FASCIA
HOLE IN WEB

PHOTO 89



DESCRIPTION: SPAN 10: GIRDER 1 (BEGIN) - INTERIOR
HOLE IN WEB

PHOTO 90



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Sheet 46 of 77

I-481 SB over CSX BIN 1093571



DESCRIPTION: SPAN 10: GIRDER 4 (BEGIN) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 91



DESCRIPTION: SPAN 10: GIRDER 4 (BEGIN) - FASCIA
DELAMINATED STEEL ON STIFFENER

PHOTO 92



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Sheet 47 of 77

I-481 SB over CSX BIN 1093571



DESCRIPTION: SPAN 10: GIRDER 1 (MIDSPAN) – FASCIA
FLANGE DETERIORATION

PHOTO 93



DESCRIPTION: SPAN 10: GIRDER 1 (MIDSPAN) – FASCIA
FLANGE DETERIORATION

PHOTO 94



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Sheet 48 of 77

I-481 SB over CSX BIN 1093571



DESCRIPTION: SPAN 10: GIRDER 4 (MIDSPAN) – FASCIA
FLANGE AND WEB DETERIORATION

PHOTO 95



DESCRIPTION: SPAN 10: GIRDER 4 (MIDSPAN) – FASCIA
FLANGE AND WEB DETERIORATION

PHOTO 96



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Sheet 49 of 77

I-481 SB over CSX BIN 1093571



DESCRIPTION: SPAN 10: GIRDER 1 (END) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 97



DESCRIPTION: SPAN 10: GIRDER 1 (END) – INTERIOR
HOLE IN WEB OVERHANG

PHOTO 98



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Sheet 50 of 77

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DESCRIPTION: SPAN 10: GIRDER 4 (END) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 99



DESCRIPTION: SPAN 10: GIRDER 4 (END) - FASCIA
WEB OVERHANG DETERIORATION

PHOTO 100



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Sheet 51 of 77

I-481 SB over CSX BIN 1093571



DESCRIPTION: PIER NO. 10 – BEGIN FACE

PHOTO 101



DESCRIPTION: PIER NO. 10 – END FACE

PHOTO 102



DESCRIPTION: SPAN 11: GIRDER 1 (BEGIN) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 103



DESCRIPTION: SPAN 11: GIRDER 1 (BEGIN) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 104



DESCRIPTION: SPAN 11: GIRDER 4 (BEGIN) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 105



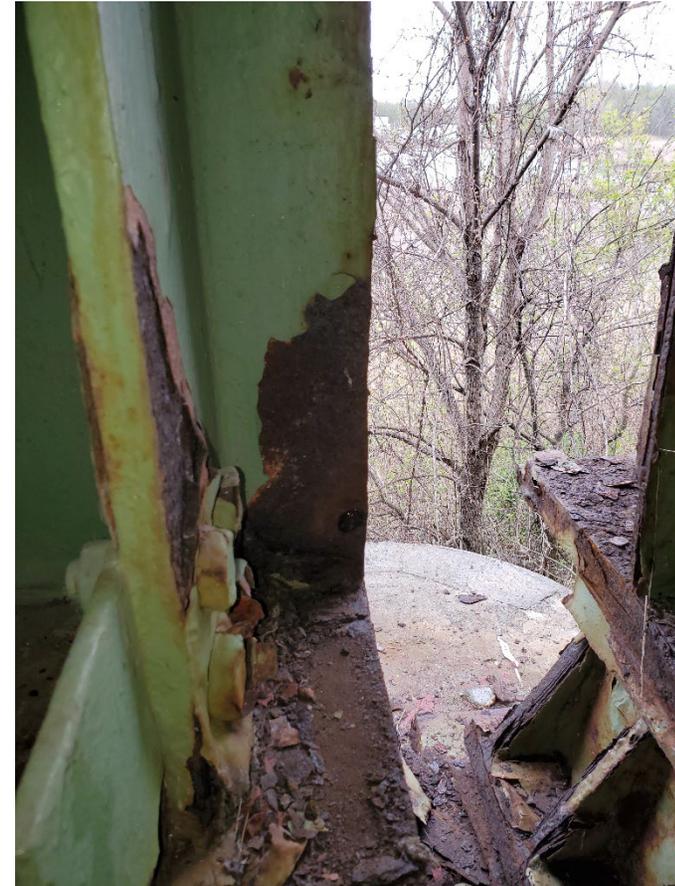
DESCRIPTION: SPAN 11: GIRDER 4 (BEGIN) - FASCIA
STIFFENER DETERIORATION

PHOTO 106



DESCRIPTION: SPAN 11: GIRDER 1 (END) - FASCIA
WEB DETERIORATION

PHOTO 107



DESCRIPTION: SPAN 11: GIRDER 1 (END) - INTERIOR
WEB AND STIFFENER DETERIORATION

PHOTO 108



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Sheet 55 of 77

I-481 SB over CSX BIN 1093571



DESCRIPTION: SPAN 11: GIRDER 4 (END) - FASCIA
WEB DETERIORATION

PHOTO 109



DESCRIPTION: SPAN 11: GIRDER 4 (END) - INTERIOR
WEB AND STIFFENER DETERIORATION

PHOTO 110



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DESCRIPTION: PIER NO. 11 – BEGIN FACE

PHOTO 111



DESCRIPTION: PIER NO. 11 – END FACE

PHOTO 112



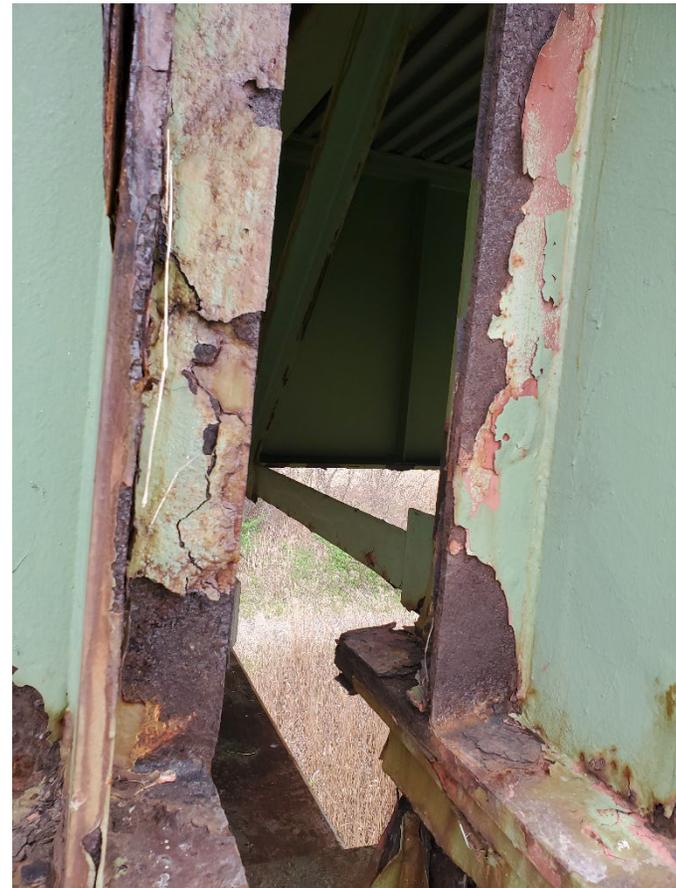
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DESCRIPTION: SPAN 12: GIRDER 1 (BEGIN) - FASCIA
WEB DETERIORATION

PHOTO 113



DESCRIPTION: SPAN 12: GIRDER 1 (BEGIN) - FASCIA
WEB DETERIORATION

PHOTO 114



DESCRIPTION: SPAN 12: GIRDER 4 (BEGIN) - FASCIA
FLANGE AND WEB DETERIORATION

PHOTO 115



DESCRIPTION: SPAN 12: GIRDER 4 (BEGIN) - FASCIA
STIFFENER DETERIORATION

PHOTO 116



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Sheet 5 of 77
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DESCRIPTION: SPAN 12: GIRDER 1 (FLTR) - FASCIA
FLANGE AND WEB DETERIORATION

PHOTO 117



DESCRIPTION: SPAN 12: GIRDER 4 (MIDSPAN) - FASCIA
FLANGE AND WEB DETERIORATION

PHOTO 118



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DESCRIPTION: SPAN 12: GIRDER 4 (END) - FASCIA
FLANGE AND STIFFENER
DETERIORATION

PHOTO 119



DESCRIPTION: SPAN 12: GIRDER 4 (END) - FASCIA
STIFFENER DETERIORATION

PHOTO 120



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DESCRIPTION: PIER NO. 12 – BEGIN FACE

PHOTO 121



DESCRIPTION: PIER NO. 12 – END FACE

PHOTO 122



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Sheet 62 of 77

I-481 SB over CSX BIN 1093571



DESCRIPTION: SPAN 13: GIRDER 1 (BEGIN) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 123



DESCRIPTION: SPAN 13: GIRDER 1 (BEGIN) - FASCIA
FLANGE DETERIORATION

PHOTO 124



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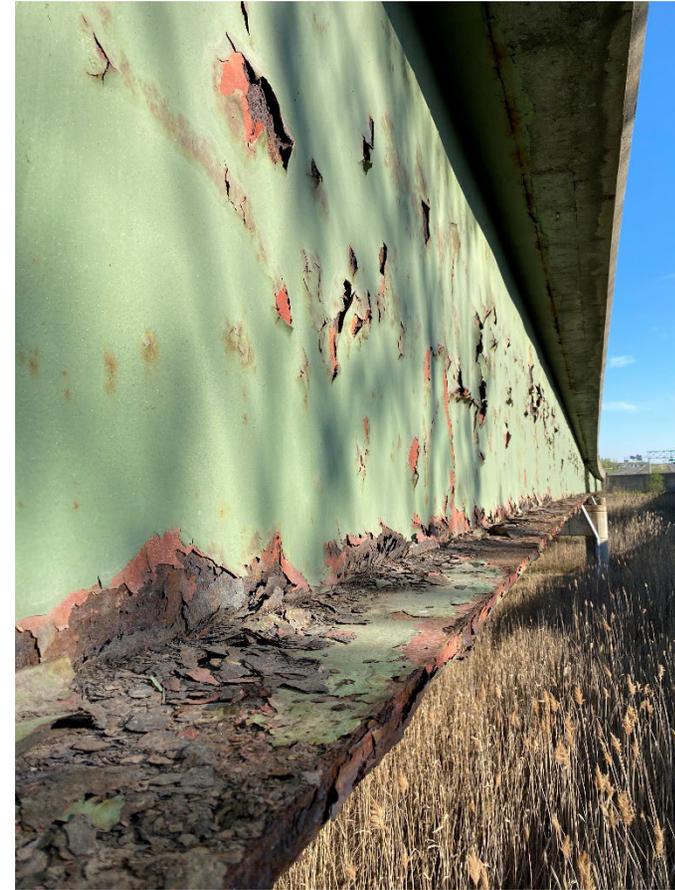
Sheet 63 of 77

I-481 SB over CSX BIN 1093571



DESCRIPTION: SPAN 13: GIRDER 1 (BEGIN) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 125



DESCRIPTION: SPAN 13: GIRDER 1 (BEGIN) - FASCIA
FLANGE DETERIORATION

PHOTO 126



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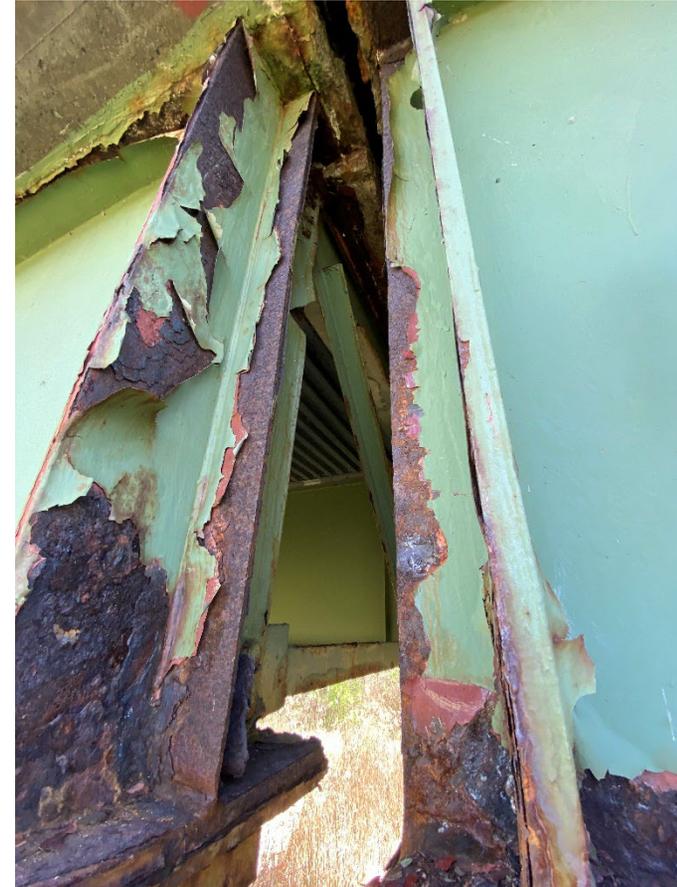
Sheet 64 of 77

I-481 SB over CSX BIN 1093571



DESCRIPTION: SPAN 13: GIRDER 1 (END) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 127



DESCRIPTION: SPAN 13: GIRDER 1 (END) - FASCIA
WEB DETERIORATION

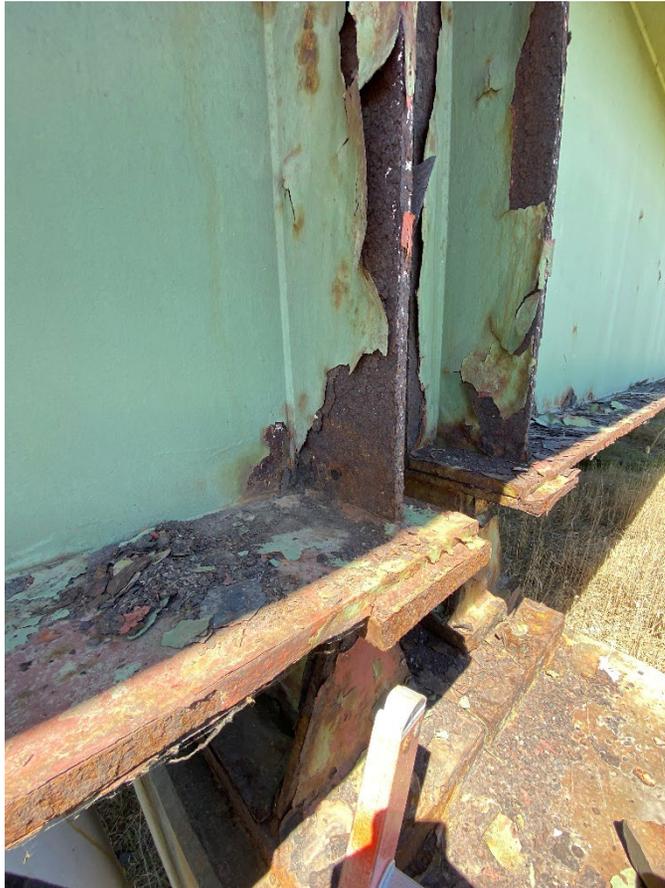
PHOTO 128



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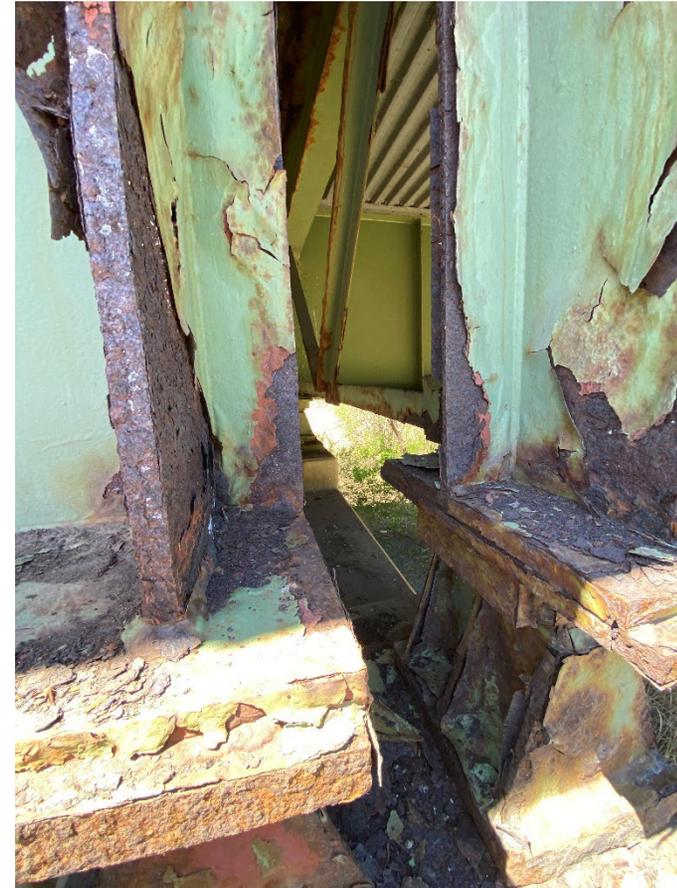
Sheet 65 of 77

I-481 SB over CSX BIN 1093571



DESCRIPTION: SPAN 13: GIRDER 4 (END) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 129



DESCRIPTION: SPAN 13: GIRDER 4 (END) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 130



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Sheet 66 of 77

I-481 SB over CSX BIN 1093571



DESCRIPTION: PIER NO. 13 – BEGIN FACE

PHOTO 131



DESCRIPTION: PIER NO. 13 – END FACE

PHOTO 132



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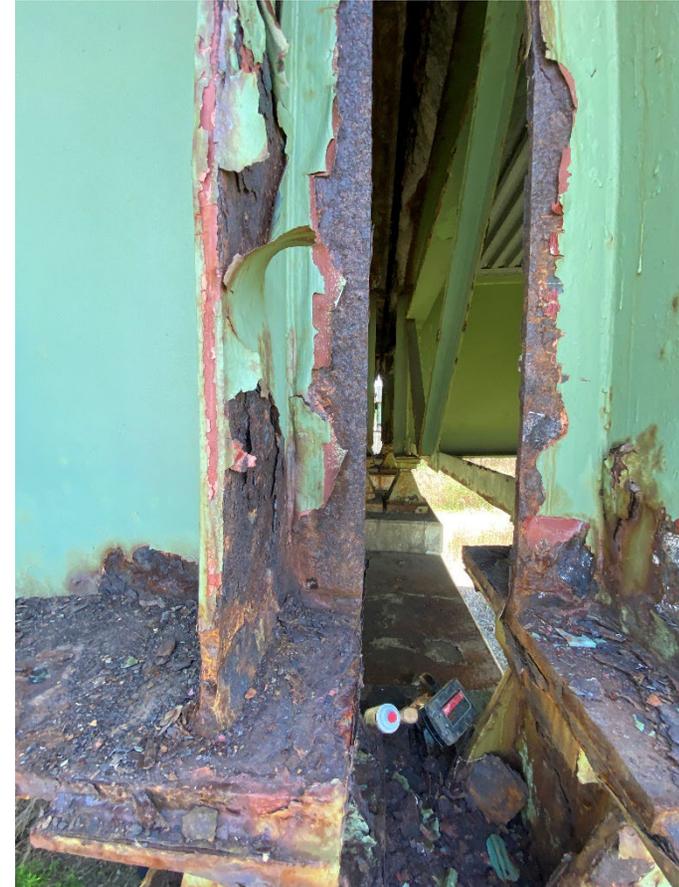
Sheet 67 of 77

I-481 SB over CSX BIN 1093571



DESCRIPTION: SPAN 14: GIRDER 1 (BEGIN) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 133



DESCRIPTION: SPAN 14: GIRDER 1 (BEGIN) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 134



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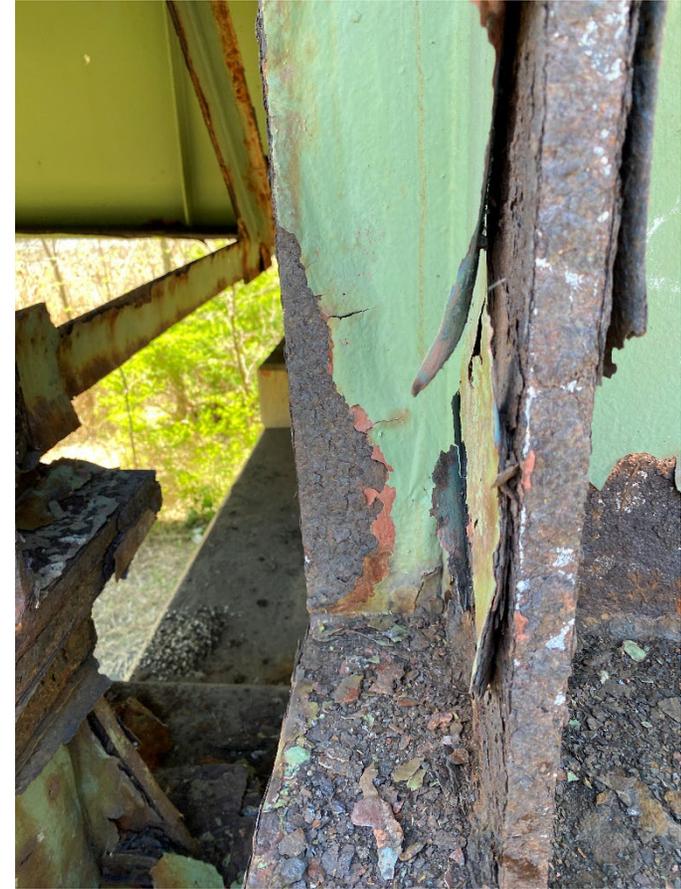
Sheet 68 of 77

I-481 SB over CSX BIN 1093571



DESCRIPTION: SPAN 14: GIRDER 4 (BEGIN) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 135



DESCRIPTION: SPAN 14: GIRDER 4 (BEGIN) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 136



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DESCRIPTION: SPAN 14: GIRDER 4 (MID) - FASCIA
FLANGE DETERIORATION

PHOTO 137



DESCRIPTION: SPAN 14: GIRDER 4 (MID) - FASCIA
FLANGE DETERIORATION

PHOTO 138



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Sheet 70 of 77

I-481 SB over CSX BIN 1093571



DESCRIPTION: SPAN 14: GIRDER 1 (END) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 139



DESCRIPTION: SPAN 14: GIRDER 1 (END) - FASCIA
FLANGE DETERIORATION

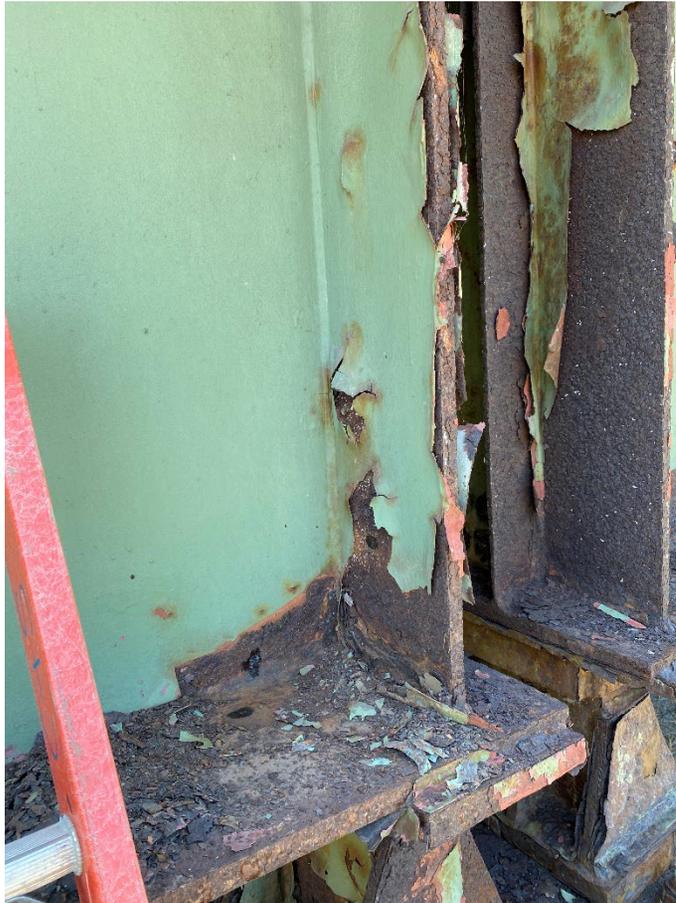
PHOTO 140



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Sheet 71 of 77

I-481 SB over CSX BIN 1093571



DESCRIPTION: SPAN 14: GIRDER 4 (END) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 141



DESCRIPTION: SPAN 14: GIRDER 4 (END) - FASCIA
WEB DETERIORATION

PHOTO 142



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DESCRIPTION: PIER NO. 14 – BEGIN FACE

PHOTO 143



DESCRIPTION: PIER NO. 14 – END FACE

PHOTO 144



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Sheet 73 of 77

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DESCRIPTION: SPAN 15: GIRDER 1 (BEGIN) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 145



DESCRIPTION: SPAN 15: GIRDER 1 (BEGIN) - FASCIA
FLANGE DETERIORATION

PHOTO 146



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Sheet 74 of 77

I-481 SB over CSX BIN 1093571



DESCRIPTION: SPAN 15: GIRDER 4 (BEGIN) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 147



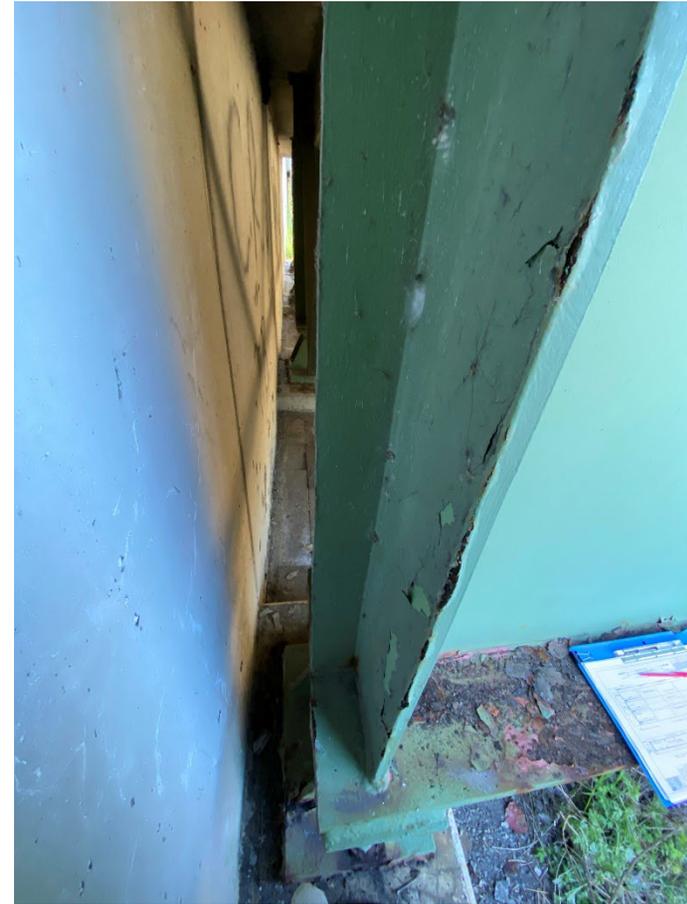
DESCRIPTION: SPAN 15: GIRDER 4 (BEGIN) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 148



DESCRIPTION: SPAN 15: GIRDER 1 (END) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 149



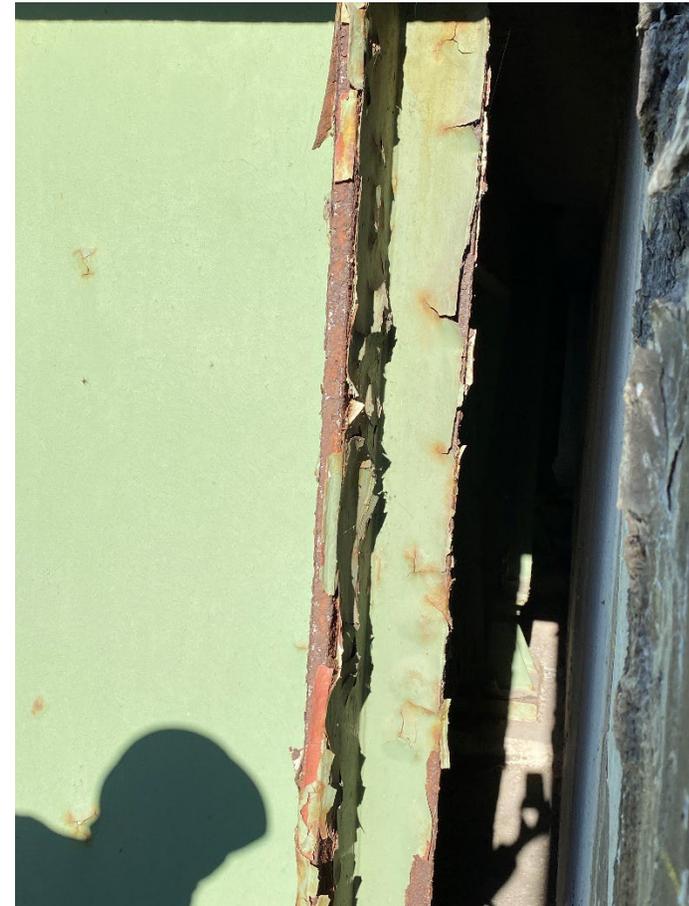
DESCRIPTION: SPAN 15: GIRDER 1 (END) - FASCIA
WEB AND STIFFENER

PHOTO 150



DESCRIPTION: SPAN 15: GIRDER 4 (END) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 151



DESCRIPTION: SPAN 15: GIRDER 4 (END) - FASCIA
WEB AND STIFFENER DETERIORATION

PHOTO 152



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Sheet 77 of 77

I-481 SB over CSX BIN 1093571



DESCRIPTION: END ABUTMENT
CONCRETE SPALLING AND EXPOSED
REBAR

PHOTO 153



DESCRIPTION: END ABUTMENT – LEFT WINGWALL
ISOLATED MAP CRACKING ALONG FACE

PHOTO 154